

## Final Report Dickson Precinct Traffic and Parking Study

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#### SMEC COMPANY DETAILS

SMEC Australia Pty. Ltd.									
Level 1	, 243 Northbourne Avenue, Lyneham ACT 2602								
Tel:	(02) 6234 1958								
Fax:	(02) 6234 1966								

Email: @smec.com

#### www.smec.com

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# Dickson Precinct Traffic and Parking Study – Final Report

For: ESDD JULY 25, 2012

## EXECUTIVE SUMMARY

A master plan for the Dickson Group Centre was developed in 2011 by ACT Planning and Land Authority (ACTPLA, now Environment and Sustainable Development Directorate (ESDD)). This master plan defines the structure of development in the Dickson precinct over the next 30 years. SMEC has been engaged to undertake an assessment of the master plan and make recommendations regarding transport, including non-motorised transport, parking and traffic, for the current situation, the short term time frame and the long term implementation of the master plan.

A number of transport studies have been conducted in Dickson recently, especially regarding parking, and these studies have been reviewed and the outcomes used where appropriate for this study.

## **ES.1 Existing Conditions**

The following investigations into the current conditions in Dickson were undertaken:

- Examination of pedestrian and cycle network connectivity and condition
- Survey of pedestrian and cycle volumes
- Survey of parking supply and utilisation
- Survey of traffic volumes
- Micro-simulation modelling
- Intersection analysis

All surveys of demand (cyclist, pedestrian, parking and traffic) were carried out for four peak periods, namely:

- Weekday AM peak (7:00AM-9:00AM)
- Weekday PM peak (4:00PM-6:00PM)
- Weekend midday peak (11:00AM-1:00PM)
- Weekend evening peak (6:00PM-8:00PM)

A site visit was conducted to examine the provision of pedestrian and cyclist infrastructure, as well as its condition and possible safety risks. It was found that, while the provision and condition of the infrastructure tend to be good, there are a few areas that require upgrades.

The pedestrian and cyclist volumes at 24 locations across Dickson were surveyed to allow identification of areas of high demand and possible prioritising of recommendations based on volumes.

A parking supply and utilisation survey was carried out for the centre. 23 parking areas were identified and surveyed for the four peak periods. It was found that there is significant spare capacity across the centre, with utilisation peaking at 63%. However, demand during different peaks is focused on certain parking areas and the centre may benefit from better signage and pedestrian connectivity to encourage better use of car parking areas that are slightly further from the land use they are serving. Gated and publicly accessible car parks were analysed separately and it was found that 65% of the parking is publicly accessible. The publicly accessible parking had a maximum utilisation of 64%, which occurred in the weekday PM and weekend midday peak periods. It is noted that 391 parking spaces were not surveyed (329 in the Dame Pattie Menzies House structured car park and up to 62 on-street parking spaces on Antill Street).

Traffic turning volume surveys were conducted for 21 intersections across the precinct for the four peak periods. These surveys showed that traffic volumes tend to be higher on external roads, including Northbourne Avenue, Antill Street and Cowper Street, than inside the centre. These roads had significantly higher traffic volumes on weekdays than weekends. However, the traffic volumes inside the centre tended to be higher during the weekend peaks than the weekday peaks.

The hierarchy of the road network in the study area was assessed, based on the traffic volumes. It was found that most roads carried traffic volumes appropriate to their hierarchy. However, Antill Street, Cowper Street and Challis Street are currently carrying more traffic than is recommended for their respective hierarchies.

Finally, the 21 intersections were analysed for the four surveyed peaks using SIDRA Intersection. The results of this analysis showed that there are a small number of intersections currently experiencing traffic congestion and high levels of delay for vehicles travelling through them.

## ES.2 Assessment of Master Plan

The master plan goals were assessed for two future scenarios:

- Short term, including the development of supermarkets on Blocks 19 and 21
- Long term where the entire master plan is assumed to be implemented

In the short term, only the parking requirements during construction of the supermarkets were assessed as the traffic impacts of the completed supermarket developments were assessed in 2011 by Brown Consulting. It was found that there was insufficient spare parking capacity to cater for the lost capacity while construction is underway if both blocks are developed at the same time. In addition, there is unlikely to be sufficient spare capacity in existing car parks to cater for the lost capacity while Block 21 is being developed, if construction cannot be staged and part of the car park kept open during construction. Indicative levels of development of the two sites were found by Brown Consulting to require replacement of the existing 326 parking spaces and provision of an additional 359 parking spaces. Any development on this site should comply with the requirements of the *ACT Parking and Vehicular Access General Code*.

In the long term, a number of recommendations were made to meet the pedestrian connectivity goals. These included new crossing points and upgrades to existing paths. Some of the goals of the master plan regarding pedestrian and cyclist facilities were addressed in the short term recommendations.

The level of detail available regarding future developments in the Dickson master plan is relatively low so a detailed parking assessment was not able to be carried out. However, indicative parking requirements were developed based on potential land use supplied by ESDD. The calculations carried out indicate that the parking demand is likely to increase to approximately 6,000 in 2031 from 2,500 in 2012. All of this parking will need to be provided inside future developments, probably as basement levels.

The potential indicative road hierarchy, based on predicted daily traffic volumes was also investigated. It was found that with the increased development in Dickson, a number of roads would be carrying substantially higher traffic volumes than is recommended for their hierarchy. These roads include:

- Cowper Street
- Challis Street
- Cape Street (including the extension)

- Badham Street
- Dickson Place

The predicted traffic volumes on these streets mean that on-street parking may not be appropriate. In addition, access and egress to and from developments may be impacted by the high volumes.

The final assessment of the long term scenarios was intersection analysis, both with and without the implementation of the master plan. The same 21 intersections that were analysed for the current situation were analysed for the long term scenarios. In addition, the new intersection related to the extension of Cape Street to Northbourne Avenue in the west and Dickson Place in the east were analysed for the master plan scenario.

It was found that there are a number of intersections that are expected to perform at Level of Service F, which indicates an unacceptable level of delay for drivers, in the long term. Many of these intersections showed similar performance in scenarios both with and without master plan implementation. A number of potential upgrades were recommended to address the performance issues.

## ES.3 Cost Estimates

Cost estimates for the recommended upgrades and modifications were developed for the current situation and the long term master plan scenario. These costs are shown in the table below and include 40% contingency and GST.

Time Frame	Estimated Cost (inc GST)
Short term recommendations	\$650,850
Long term recommendations	\$1,113,600
Total	\$1,764,450

These recommended actions will address all current issues identified and allow the implementation of the developments proposed in the master plan.

## **ES.4 Recommendations**

The current operation of the transport network in Dickson was assessed and found to be generally good. However, addressing the following improvements should be prioritised in the short term:

- Pedestrian and cyclist infrastructure and safety:
  - Provide a pedestrian crossing on Challis Street near its intersection with Morphett Street
  - Provide a pedestrian crossing on Challis Street near the Telstra Building
  - Provide a pedestrian crossing on Antill Street near its intersection with Pigot Street (short term only)
  - Provide a pedestrian crossing on Dickson Shops Road close to the intersection with Cowper Street

- Monitor the safety of the pedestrian crossing on Challis Street north of Daramalan College and intervene with a raised pedestrian crossing if required
- Widen the 1.2m concrete paths around Daramalan College to 2.0 metres wide
- Provide better lighting on the path extension from Badham Street to the shared path to the south of the precinct to improve security
- Ensure pedestrian ramps along Challis Street have appropriate steepness for wheelchair access.
- Construct a new pedestrian/cyclist path connection from north of Rosevear Place to shared path to the south and swimming pool
- Car parking operations:
  - Implement better signage to inform users about the location of parking areas that are currently underutilised, especially the pool car park, the surface car park south of Dickson Place and the car park underneath the Dickson Tradies Club
- Road network and intersections
  - Signalise the intersection of Morphett Street with the southbound carriageway of Northbourne Avenue. The northbound carriageway would remain as it is.

These recommendations are expected to improve the transport operations and safety in Dickson in the short term.

A number of recommendations have been made to allow implementation of the master plan and these should be implemented as required. These recommendations include:

- Pedestrian and cyclist infrastructure and safety:
  - Provide north/south external links into Dickson by signalising the intersection of Antill Street and Badham Street
  - Improve pedestrian safety at the intersection of Morphett Street and Challis Street (preferably by signalisation)
  - Improve pedestrian safety at the intersection of Morphett Street and Cowper Street (preferably by signalisation)
- Car parking operations:
  - Implement an area wide parking strategy to efficiently plan parking for future developments
- Road network and intersections:
  - Signalise the intersection of Antill Street and Challis Street
  - Signalise the intersection of Challis Street and Cape Street
  - Signalise the intersection of Challis Street and Morphett Street
  - Signalise the intersection of Antill Street and Badham Street (also recommended to improve pedestrian access to Dickson from the suburbs to the north)
  - Signalise the intersection of Morphett Street and Cowper Street (also recommended to improve pedestrian safety around Daramalan College)

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## **1 INTRODUCTION**

## 1.1 Background

Dickson Group Centre is one of the larger and more active group centres in Canberra and a master plan for the centre has been recently developed (May 2011). This master plan redefines building heights and the layout of the group centre in the longer term, which then allows for increased development in the area.

There have been concerns that the increased development and changes to the layout will have a negative impact on transport and parking in the area. The aim of this project is to assess the impacts of the proposed master plan and to develop options to address these impacts. In addition, the current public transport proposals for Dickson and the Northbourne Avenue corridor will be integrated with the future options for Dickson.

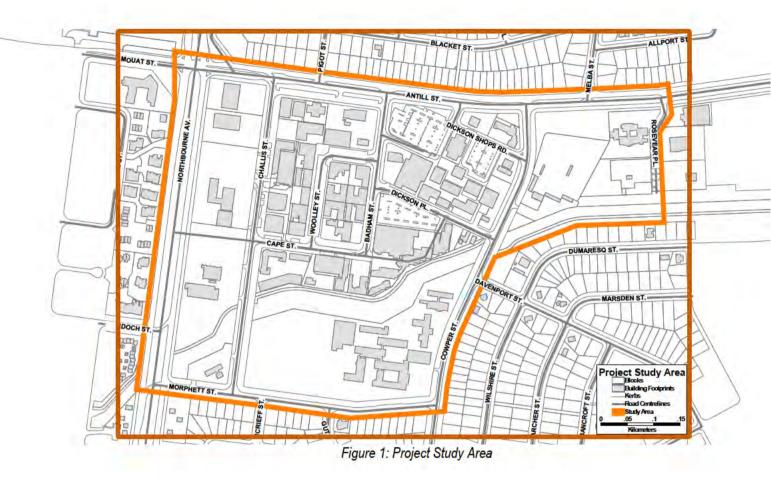
## 1.2 Objectives

The following are the main objectives of this study:

- Integrate transport into the Dickson Master Plan and consider the current public transport planning projects underway in the area
- Develop integrated parking and transport network provisions for the next 30 years (assuming that the Dickson Master Plan is implemented)
- Investigate the effects of the master plan developments on transport in and around the Dickson Group Centre and develop solutions if required
- Determine if and where road improvements will be required to address the increased development in the group centre
- Determine if changes need to be made to the Dickson Master Plan to address potential parking and transport impacts

## 1.3 Study Area

The study area for this project is shown in *Figure 1*. The study area includes the group centre and is bounded by Antill Street, Northbourne Avenue, Morphett Street, Cowper Street and Rosevear Place.



## **2 REVIEW OF RELEVANT BACKGROUND REPORTS**

SMEC has reviewed previous reports relating to traffic and parking in Dickson. These reports and a brief description of each are listed below:

- Dickson Master Plan, ACTPLA, May 2011
- Dickson Group Centre Temporary Parking Areas, Brown Consulting, May 2011
- Development Traffic Assessment Report for Block 19 and 21, Dickson Shops, Dickson, ACT, Brown Consulting, June 2011
- Dickson Group Centre Parking Utilisation Study, Brown Consulting, August 2011
- Dickson Temporary Car Parks, SMEC, August 2011

The Dickson master plan, developed by ACTPLA (now ESDD) is the primary background document for this study. The master plan examines the current situation and makes long term recommendations for the development of the precinct. The Dickson master plan also defines a number of sub-precincts in Dickson as shown in *Figure* **2**. These precinct names will be used in this study to refer to the relevant areas.

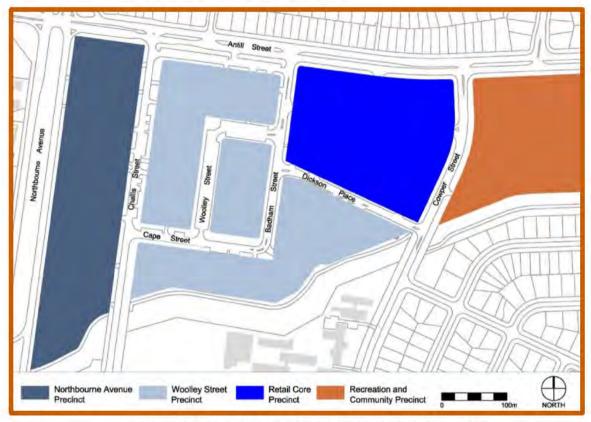


Figure 2: Dickson Precincts (Source: Dickson Master Plan, p19)

In May 2011, Brown Consulting undertook a brief examination of the Dickson area to determine if there were suitable areas able to be used for temporary car parking while redevelopment of existing car parks was underway. They found that there were a number of areas available and recommended further investigations into the following sites:

- Formal on-street parking on Antill Street
- An extension of the existing aquatic centre car park to the east
- A new parking area to the north of the aquatic centre
- Temporary parking on the old ACTAB site

These recommendations were examined in more detail in a later study by SMEC.

Brown Consulting conducted a traffic assessment report for the proposed developments on Block 19 and Block 21, which are currently open air car parks. This assessment found that the current car parks operate at, or slightly over, capacity during the main shopping peak, which is during the middle of the day on a Saturday. The conclusions of the study stated that the development is not expected to have a significant effect on traffic operations in the Dickson Group Centre. In addition, the development will cause a slight shortfall in parking provision, which is expected to lead to overspill of parking demand into adjacent areas.

In August 2011, a parking utilisation study for the Dickson Precinct was undertaken by Brown Consulting. This study found that there was significant spare parking capacity in the Dickson centre at all times, with utilisation rates of 51%-74% on weekdays and 40%-51% on weekends.

Also in August 2011, SMEC undertook concept design of four of the temporary parking areas suggested by Brown Consulting in May 2011. The concept design process included an investigation into existing drainage, vegetation and services. The designs produced included details of car park surface treatments, drainage, access, landscaping, lighting, impacts on vegetation and pedestrian connectivity. In addition, preliminary cost estimates for each of the four car parks were estimated.

## **3 EXISTING CONDITIONS**

A number of investigations on the existing conditions in and around the Dickson Group Centre were conducted, which include the following:

- Examination of pedestrian and cycle network connectivity and condition
- Survey of pedestrian and cycle volumes
- Survey of parking supply and utilisation
- Survey of traffic volumes
- Micro-simulation modelling
- Intersection analysis

Currently, Dickson has a mixture of land use including office, commercial and entertainment. As these land uses have different peak times in terms of traffic generation, the surveys and analyses were conducted over four peak periods to gain a thorough understanding of the existing transport and parking conditions. The four peak periods specified by the client were:

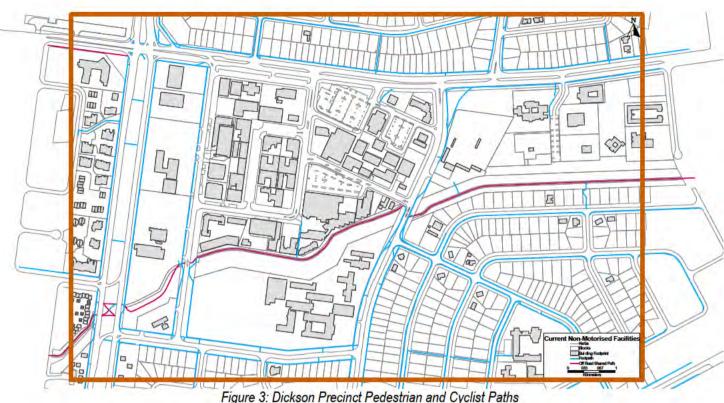
- Weekday AM peak (7:00AM-10:00AM)
- Weekday PM peak (4:00PM-6:00PM)
- Weekend Mid-day peak (11:00AM-1:00PM)
- Weekend Evening peak (6:00PM-8:00PM)

The following sections provide detailed discussions of the investigations outlined above.

#### 3.1 Pedestrian and Cycle Network

The pedestrian and cycle network assessment was based on a site inspection of Dickson Precinct and a desktop assessment of the pedestrian and cycle survey results.

*Figure 3* shows the existing cycle and pedestrian path network within Dickson, based on GIS information obtained from TAMSD. A site inspection was carried out on 19 March 2012, which focused on identifying potential infrastructure and safety issues for pedestrians and cyclists.



rigure 5. Dickson'r recinci r euestrian and Cyclist r

#### 3.1.1 Pedestrian and Cyclist Safety Assessment

#### Pedestrian Safety

The precinct is generally safe for pedestrians to move around with pedestrian crossings or signalised crossings on most pedestrian desire lines. However, there are some areas where pedestrian safety at road crossings should be considered:

- Safer access across Challis Street at the intersection with Morphett Street would improve safety for students accessing Daramalan College by walking along Morphett Street
- A second pedestrian crossing of Challis Street near the Telstra Building would benefit office building occupants accessing the Dickson business area (construction of a crossing in this location began after the site inspection was carried out)
- A formal pedestrian crossing on Antill Street near the Pigot Street intersection would provide better access from the subdivision area north of Dickson (in the short term only as signals will be provided at Challis Street in the long term)

The footpaths around Daramalan College are 1.2m wide concrete paths, 2.0m wide paths should be considered to allow groups to access them. In addition, they are narrow for a mix of pedestrian and bicycling use.

There is anecdotal evidence that the pedestrian crossing on Challis Street, just north of Daramalan College, is a regular accident site. No specific issues at this location were noted during the site visit. If there are persistent safety issues at this crossing, it is recommended that a raised pedestrian crossing be installed. This will increase the visibility of the crossing point and also force drivers to slow down as they approach.

#### **Off-Road Bicycle Safety**

There is a shared path running in a generally east-west direction to the south of Dickson that is used regularly by cyclists and pedestrians. There are three access points into Dickson from this path:

- Along Challis Street footpath
- Along Cowper Street footpath
- Footpath connection to Badham Street

Some paths on Challis Street, Badham Street, Cowper Street, Dickson Place and Antill Street are narrow and not suitable for shared usage by pedestrians and cyclists. The paved areas within the shopping areas have 90-degree blind corners and are not suitable for cycling.

#### **On-Road Bicycling**

There are no formal on-road cycle lanes marked within the Dickson Group Centre except for Northbourne Avenue. The internal streets have a lot of turning vehicles accessing intersections and car parking areas as well as reversing out of 90-degree on-street parking bays. The area is not considered to be a safe environment for on-road cycling under the current traffic control scheme.

#### **Bus Services**

The Dickson Group Centre has bus stops around the perimeter on Northbourne Avenue, Antill Street and Cowper Street. The maximum walking distance between the bus stops and employment/shops is approximately 600m across the precinct. An internal bus interchange would reduce that distance to less than 400m.

#### Lighting

All streets have street lighting and most public car parking areas have adequate lighting. The pedestrian areas through the shopping area have pedestrian style lighting. The regional shared path has been provided with lighting recently. The only pedestrian area to rely on ambient light is the path extension from Badham Street to the shared path on the south side of the precinct. This path is constrained by fences either side and has no passive security opportunity and provides a moderate security risk for users late at night.

#### Infrastructure Quality

The internal paths and crossings are in good condition with evidence of maintenance. Some concrete paths, both inside the precinct and on the perimeter are narrow for two way pedestrian/cycle use. Future path widening of these paths will improve the amenity.

#### 3.1.2 Pedestrian and Cyclist Infrastructure Assessment

#### Path Widths

Concrete path widths outside the central retail area are generally 1.2m wide. Pedestrian traffic in some areas, particularly around Daramalan College would benefit from 2.0m wide paths to allow simultaneous bicycle and pedestrian access.

#### Morphett Street - Challis Street Intersection

Students cross Challis Street at the intersection when walking along the Morphett Street northern path. There is risk of conflict between turning vehicles and pedestrians. A controlled crossing should be considered.

#### **Challis Street**

During the AM peak, traffic generally formed platoons allowing adult pedestrians breaks in traffic to cross safely without traffic control.

Students accessing Daramalan College were observed to use the pedestrian crossing adjacent to the storm drain and access the college through a side gate.

Pedestrian movements appear to be low in the morning.

Movements may increase between the office blocks and food outlets during lunch break. A pedestrian crossing near the Telstra Building may be warranted.

Two steep pedestrian ramps were noted north of Dame Pattie Menzies House that are too steep for wheel chair access. These ramps are located at each end of the row of shops between Dame Pattie Menzies House and Antill Street. There is a footpath that runs adjacent to the ramps but this is often blocked by cars parking with their wheels against the kerb and their noses over the footpath.

#### **Dickson Place**

There is a pedestrian crossing from the shopping plaza area to the car park that requires pedestrians to share the area with circulating vehicles. The footpath adjacent to Dickson Place south has trees blocking access.

There is a paved walkway at the south-eastern end of the shopping plaza (approximately 40 metres west of the intersection with Cowper Street) that would provide a safer crossing if a pedestrian crossing was provided. However, a crossing at this location may have a negative impact on the operation of the intersection of Dickson Place and Cowper Street, which already provides a signalised pedestrian crossing, due to potential queuing caused by the crossing if pedestrian volumes are high.

#### **Rosevear Place**

There is no direct access from the area south of the storm drain, including the shared path, to the businesses in Rosevear Place.

There is a pedestrian path at the northern end of Rosevear Place. Pedestrians share the road with vehicles accessing car parks at the southern half of the road.

There is no formal path between the southern end of Rosevear Place and the swimming pool complex.

#### **Cowper Street**

There are four formal crossing points provided along Cowper Street, two are signalised and two are pedestrian crossings.

Dickson Shops Road has no pedestrian crossing close to the intersection with Cowper Street.

#### **Antill Street**

There are signalised pedestrian crossings at Cowper Street and another near the Dickson Library providing access to the north-east. The next signalised intersection is at Northbourne Avenue.

Pedestrian access across Antill Street from Pigot Street and to Challis Street is uncontrolled

#### Northbourne Avenue

There are pedestrian crossings at the Antill Street signalised intersection and a signalised pedestrian crossing at the shared path crossing near Morphett Street.

There are two uncontrolled mid-block crossing points with concrete paths in the median.

#### General Observations

Pedestrian access between Woolley Street, Badham Street and the car parks and plaza area are good with formal pedestrian crossings on desire lines. The condition of paths is generally good in this area.

#### 3.1.3 Pedestrian and Cyclist Volumes

*Figure 4* shows the locations where pedestrian and cyclist count surveys were conducted. Note that the locations are mostly the same as the intersection count survey locations discussed later in this report, except for some pedestrian crossings. As discussed earlier, four peaks were identified by the client and the surveys were conducted for those peaks.

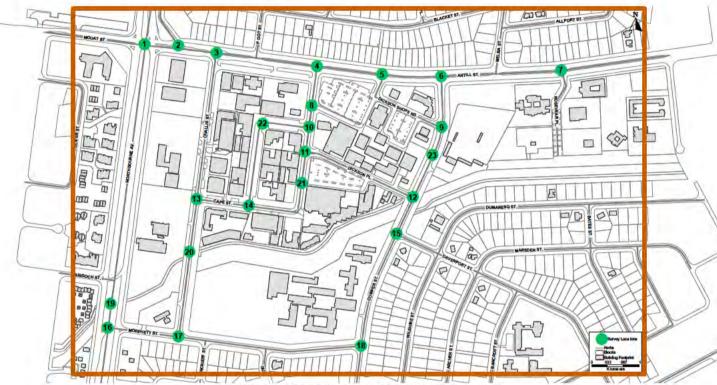


Figure 4: Pedestrian and Cyclist Survey Locations

*Figure 5* to *Figure 8* show summaries of the pedestrian and cyclist survey for each peak period. The blue bars represent the total pedestrian movements through a location while the green bars represent the total cyclist movements through the location.

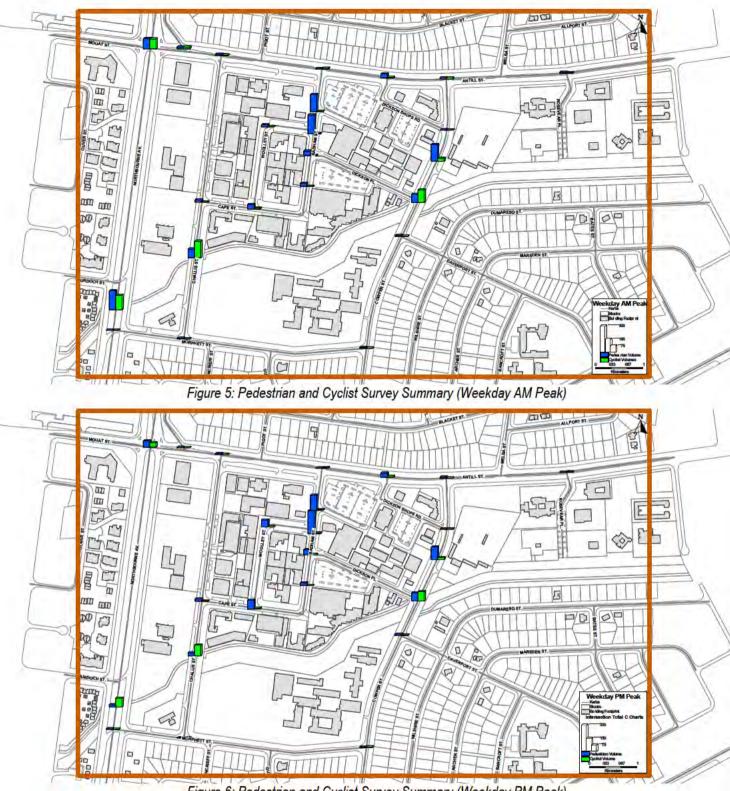


Figure 6: Pedestrian and Cyclist Survey Summary (Weekday PM Peak)



Table 1 shows a summary of the volumes presented in the preceding figures.

Table 1: Summar	y of Pedestrian and	Cyclist Survey	(2 Hour Peak Period)
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Location (Intersection)		day AM		day PM	1222.2	end AM	Weekend PM		
	Peds	Cycles	Peds	Cycles	Peds	Cycles	Peds	Cycles	
1	117	113	70	48	7	27	2	3	

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Location	Week	day AM	Week	day PM	Week	end AM	Weekend PM		
(Intersection)	Peds	Cycles	Peds	Cycles	Peds	Cycles	Peds	Cycles	
2	21	22	4	2	1	4	10	0	
3	17	8	13	2	7	0	12	2	
4	9	11	4	6	1	0	3	2	
5	46	10	46	8	37	13	17	3	
6	17	17	13	4	8	6	8	4	
7	13	7	3	7	6	4	8	1	
8	193	3	171	6	171	8	240	2	
9	3	0	4	2	2	0	5	0	
10	213	3	259	8	295	13	334	7	
11	52	3	58	3	139	15	149	5	
12	93	141	91	104	45	43	42	19	
13	25	0	26	0	15	0	96	0	
14	49	10	102	10	72	2	305	1	
15	11	6	16	7	16	0	13	3	
16	0	0	6	4	5	0	2	0	
17	11	1	0	3	5	6	5	0	
18	9	5	4	7	7	5	8	4	
19	215	164	25	99	18	50	13	27	
20	101	191	33	121	46	46	14	25	
21	35	0	29	0	35	3	74	4	
22	23	2	76	2	93	2	265	3	
23	190	34	143	21	110	38	28	6	
Total	1,463	751	1,196	474	1,141	285	1,653	121	

These figures and table show that Badham Street and the northern section of Cowper Street are utilised heavily by pedestrians during all four peak periods of the survey, except for Cowper Street in the weekend evening peak. Cape Street and Woolley Street are also heavily used by pedestrians during all peaks with the exception of the weekday AM peak period.

The shared path north of Daramalan College carries a significant number of cyclists and pedestrians at the crossings with Cowper Street, Challis Street, and Northbourne Avenue, mainly during the weekday peaks. The shared path tends to have more cyclists than pedestrians during both weekday peak periods and is likely to be used primarily by commuters. During the weekend peak periods, this path is utilised considerably less than during the weekday peak periods.

## 3.2 Parking Assessment

The parking assessment was based on a parking utilisation survey which was conducted during the same four peak periods specified for the pedestrian and cyclists count surveys. *Figure* **9** shows the location of the parking areas that were surveyed. It is noted that the surveys do not include the structured car park to the north of Dame Pattie Menzies House (on Challis Street) or the on-street parking areas on Antill Street. Previous studies in Dickson indicate that the capacity of the on-street parking area on Antill Street could hold up to 62 cars if it was properly marked. In its current unmarked state, it has a capacity of 44 cars. Utilisation of this area peaks at 64% during the week and 27% on weekends. The structured car park on Challis Street has a capacity of 329 vehicles, including 4 disabled spaces (data provided by ESDD). No previous utilisation data is available for this car park.

The weather during the survey periods was mainly fine and there was no rainfall recorded on the weekday surveyed. There was heavy rain recorded after 7:00pm on the weekend day surveyed. This may have had some impact on the evening parking utilisation survey.

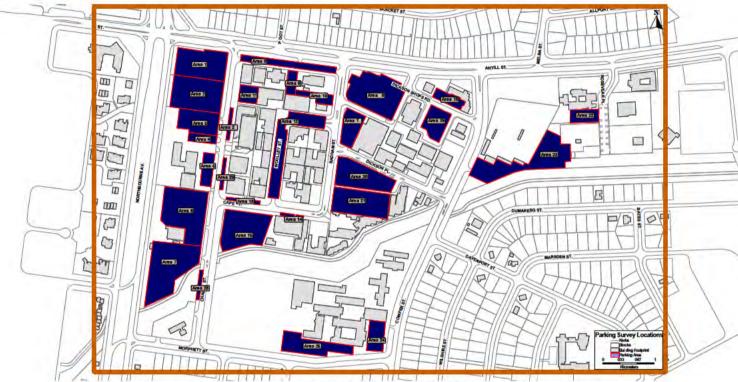


Figure 9: Parking Survey Areas

*Figure* **10** shows the proportion of parking allocations and parking restrictions in each area. It can be seen that Dickson Precinct has a significant number of long term parking spaces, mainly associated with the Northbourne Avenue Precinct, followed by short term parking. Parking spaces are well distributed throughout the precinct, with the majority of the short term parking located closer to the commercial areas.

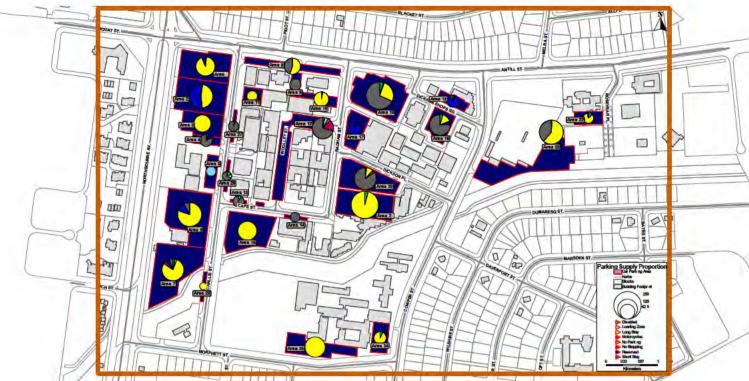


Figure 10: Current Parking Supply at each Location Based on Restrictions Figure **11** to Figure **14** provide a summary of the parking utilisation survey.

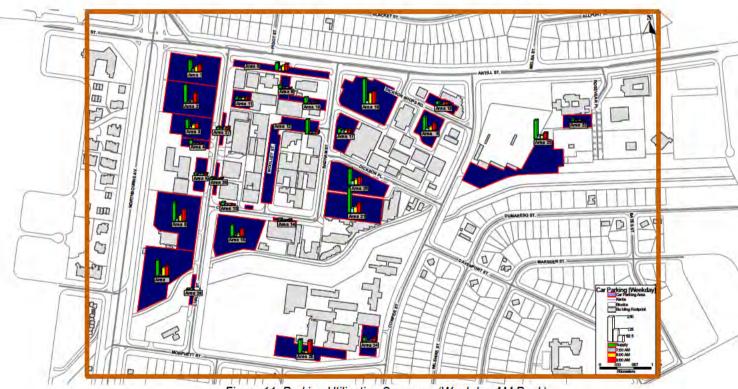


Figure 11: Parking Utilisation Summary (Weekday AM Peak)

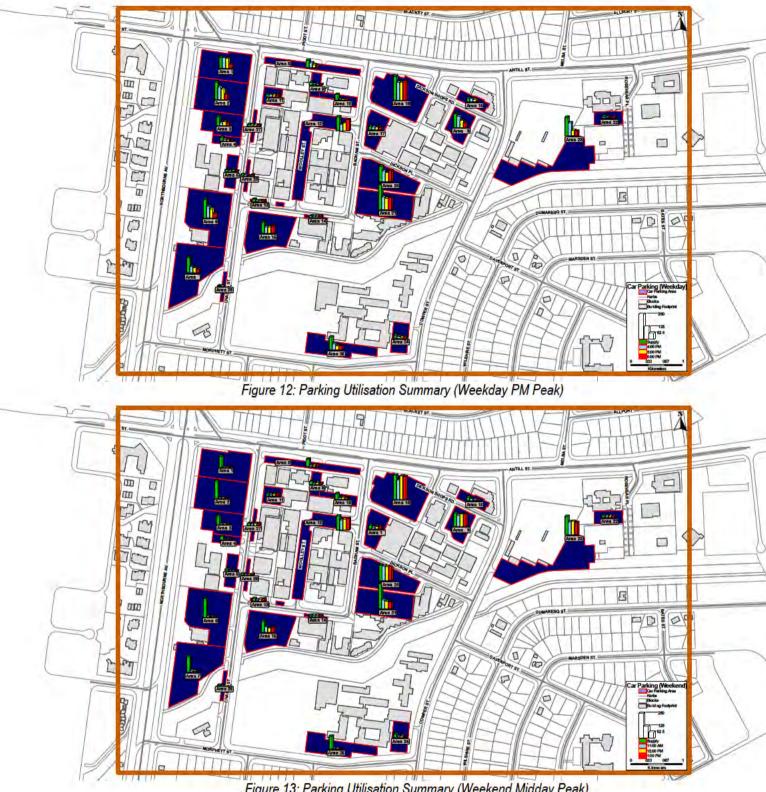


Figure 13: Parking Utilisation Summary (Weekend Midday Peak)



Figure 14: Parking Utilisation Summary (Weekend Evening Peak)

The figures shown above indicate areas where there is expected to be high demand for parking and which time period that demand occurs in. *Table* **2** shows the actual supply and demand for each of the parking areas in each of the time periods surveyed.

Location		Thursday, 16 February 2012							Weekend, 18 February 2012					
(Parking Area)	Supply	7:00	8:00	9:00	16:00	17:00	18:00	11:00	12:00	13:00	18:00	19:00	20:00	
1	111	15	47	69	107	100	43	2	1	1	0	0	1	
2	176	11	15	95	129	107	53	6	6	3	6	4	6	
3	87	6	37	49	27	37	27	4	3	3	1	0	10	
4	35	0	0	11	16	- 11	0	0	0	0	1	2	10	
5	20	0	10	12	14	12	8	0	0	0	5	12	7	
6	183	20	55	114	116	94	48	12	5	3	8	9	30	
7	151	12	79	95	63	48	33	13	9	4	3	4	9	
8	74	21	46	64	49	30	15	- 14	18	19	12	33	57	
9	30	0	2	3	14	7	9	12	18	13	24	29	29	
10	58	0	3	6	8	11	15	14	21	20	36	40	41	
11	25	7	21	24	24	22	20	18	21	23	20	24	25	
12	141	13	11	25	63	78	110	103	109	115	117	121	121	
13	27	1	2	0	12	14	17	14	19	21	26	27	26	
14	18	3	2	8	16	17	17	17	17	17	17	17	17	
15	110	7	23	72	65	65	72	54	42	47	29	32	44	
16	242	23	102	116	182	180	191	195	233	228	159	193	177	
17	38	21	25	28	32	27	23	19	22	24	17	15	31	

Table 2: Summary of Parking Utilisation Survey for the Dickson Precinct

Dickson Precinct Traffic and Parking Study 3002303 | Revision No. 2 | 25 July 2012

Location	Ormatic		Thur	sday, 16	Februa	ry 2012		Weekend, 18 February 2012					
(Parking Area)	Supply	7:00	8:00	9:00	16:00	17:00	18:00	11:00	12:00	13:00	18:00	19:00	20:00
18	36	0	13	23	32	24	2	20	11	5	5	1	1
19	135	21	36	72	112	69	44	113	123	124	26	32	27
20	154	35	52	74	107	91	114	147	141	147	80	141	149
21	244	46	57	93	127	115	117	91	74	66	79	82	117
22	22	2	6	10	12	8	15	19	17	18	22	0	0
23	183	27	31	58	129	55	46	130	134	124	31	7	2
24	37	1	11	32	20	16	19	11	15	9	11	10	4
25	129	9	17	120	64	41	39	3	5	2	2	2	3
26	15	0	2	8	11	4	11	3	3	10	9	12	13
27	21	3	15	21	19	17	13	17	21	19	19	21	21
28	7	3	7	7	4	1	0	0	0	0	0	0	0
Total	2,509	307	727	1,309	1,574	1,301	1,121	1,051	1,088	1,065	765	870	978
Total	2,309	12%	29%	52%	63%	52%	45%	42%	43%	42%	30%	35%	39%

*Table* **2** shows that the total utilisation of parking in Dickson is relatively low, peaking at 63% at 4:00 PM on a weekday. However, the parking demand is not evenly distributed and there are a number of parking areas operating close to capacity, which are indicated in red in the table. A number of these parking areas are small (Areas 9, 11, 13, 14, 22, 27 and 28) and excess demand for these areas is likely to be met in adjacent sites. However, large areas such as Areas 1, 16, 19, and 20 that are operating close to capacity indicates that there are likely to be large numbers of vehicles circulating and searching for space.

A number of car parks in Dickson are access controlled and are not accessible to the public. These controlled car parks include Areas 1, 2, 3, 6, 7, 24 and 25. While 24 and 25 are not controlled, they are used by Daramalan College and are too far from the Dickson Centre to be used by people travelling to Dickson. Area 3 has its boom gates removed after workday business hours and on weekends and becomes publicly accessible. However, it is not heavily utilised in the weekend peaks.

As mentioned earlier, the DPMH structured car park and Antill Street on-street car parking areas were not surveyed. These areas would add 391 spaces to the supply in Dickson but the utilisation of these areas is not known.

*Table* **3** shows a summary of the supply and utilisation for private and publicly accessible parking areas in Dickson.

Тура	Supply	Thursday, 16 February 2012					Weekend, 18 February 2012						
туре	Type Supply	7:00	8:00	9:00	16:00	17:00	18:00	11:00	12:00	13:00	18:00	19:00	20:00
Private	874	74	261	574	526	443	262	51	44	25	31	29	63
Utilisation		8%	30%	66%	60%	51%	30%	6%	5%	3%	4%	3%	7%
Public	1,635	233	466	735	1,048	858	859	1,000	1,044	1,040	734	841	915

Table 3: Summary of Public and Private Parking for the Dickson Precinct

Dickson Precinct Traffic and Parking Study 3002303 | Revision No. 2 | 25 July 2012

Turno	Type Supply		Thursday, 16 February 2012				Weekend, 18 February 2012						
туре		7:00	8:00	9:00	16:00	17:00	18:00	11:00	12:00	13:00	18:00	19:00	20:00
Utilisation		14%	29%	45%	64%	52%	53%	61%	64%	64%	45%	51%	56%

This table shows that the private parking demand peaks at 66% at 9:00am on a weekday. This peak may change if the utilisation of the offices in the Northbourne Precinct changes. The current utilisation is not known. The DPMH structured car park would add 329 spaces to the private supply, presumably with a similar utilisation as the other private car parks in the area.

On the weekend, the private parking is almost completely empty as these car parks serve offices and Daramalan College, which are not typically used on the weekend. The publicly accessible parking demand peaks at 64%. This demand is reached during both the weekday PM and the weekend midday peak periods.

From *Table* **2** and the earlier figures, two disparate groups with demand for parking were identified, namely:

- Commuters and students
- Shoppers and diners

These two groups are discussed in more detail in the following sections.

#### 3.2.1 Commuter and Student Parking

The parking survey shows that the parking areas in the Northbourne Avenue precinct, between Challis Street and Northbourne Avenue, (Areas 1-7 with exception of Area 5) and Daramalan College (Areas 24 and 25) appear to be mainly used by commuters and students. These parking areas share a similar pattern of parking utilisation for each peak period of the parking survey.

During the AM peak period these parking areas have their highest utilisation, increasing between 7:00 AM and 9:00 AM. During the PM peak period they are progressively emptied as commuters and students go home. During both weekend peak periods these parking areas have near zero utilisation.

It is noted that most of the parking areas west of Challis Street are gated, and are currently underutilised. The parking survey shows that the untimed parking spaces in Areas 1 to 7 are only 62% utilised by 9:00 AM on weekdays. This is the highest utilisation rate recorded for these parking areas during any of the four peak periods.

The non-gated untimed parking spaces near Challis Street (Areas 8, 11 and 28) have near 100% utilisation by 9:00 AM on weekdays. Moreover, these parking areas empty during the PM peak period in the same pattern as the gated parking areas west of Challis Street, and it is assumed that these parking areas are primarily used by commuters that do not have access to the gated parking areas.

It is also noted that the paid parking Area 15 had only 65% utilisation by 9:00 AM on a weekday, which was its highest utilisation in any surveyed peak. Thus, it appears that there is no immediate need for additional parking for commuters along the western side of Dickson. However, the relatively low utilisation of the gated parking areas and the high utilisation of the non-gated car parks suggest that better management of the parking in this area may be of some benefit.

The parking survey conducted did not allow for the identification of which land use drivers from each car park were accessing. If it is the case that workers in buildings with gated car parks are using non-gated car parks, they should be encouraged to use the gated parking areas. This will ease utilisation of the untimed parking areas along Challis Street where time restrictions could then be applied to provide more opportunities for customer access to the adjacent commercial centre.

#### 3.2.2 Shopping and Dining Parking

The parking areas in the Woolley Street Precinct and Retail Core Precinct tend to be heavily utilised during the PM peak, and are also heavily used during both weekend peak periods.

Demand is distributed across these two precincts fairly evenly during the weekend midday peak period with some spare capacity in Areas 8, 15 and 21. There is also some spare capacity in Area 23.

During the weekend evening peak period, demand is centred more on Woolley Street itself. There is now some spare capacity in Areas 19 and 23. Area 15 remains underutilised in this time period as well.

## 3.3 Traffic Assessment

#### 3.3.1 Traffic Count Surveys

*Figure* **15** shows the locations of the surveyed intersections as part of this study. A total of 18 intersections were surveyed, three of which (Intersections 1, 6 and 12) are signalised, while the rest are priority controlled. Vehicles were classified as either 'Light' or 'Heavy'.

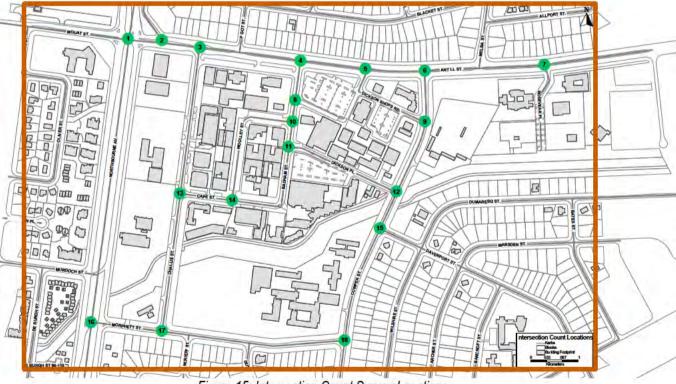


Figure 15: Intersection Count Survey Locations

*Figure 16* to *Figure 19* show summaries of the traffic survey. The volumes shown at each location are the total volumes passing through that site for the relevant two-hour peak period.





Table 4 shows a summary of the results presented in the previous figures.

Table 4: Summary	y of Light and Heavy	Vehicle Survey (2 Hour Peak Period)	
			_

Location	Weekday AM		Weekday PM		Weeke	nd AM	Weekend PM	
(Intersection)	Light	Heavy	Light	Heavy	Light	Heavy	Light	Heavy
1	4,690	244	5,402	138	4,351	85	2,708	26

Location	Weekd	ay AM	Weekd	lay PM	M Weekend AM		Weeke	nd PM
(Intersection)	Light	Heavy	Light	Heavy	Light	Heavy	Light	Heavy
2	2,161	61	2,125	36	1,695	13	1,428	9
3	2,332	75	2,135	38	1,716	15	1,440	9
4	2,035	63	1,983	37	1,677	12	1,385	8
5	1,639	54	1,347	36	1,110	11	836	4
6	2,252	66	1,811	43	1,458	15	916	7
7	1,646	35	1,135	14	944	6	595	2
8	786	19	1,113	7	1,013	3	1,081	5
9	1,181	36	1,338	25	1,066	12	553	7
10	728	19	1,052	7	868	3	1,168	6
11	748	23	1,006	6	809	3	1,001	4
12	1,334	43	1,262	26	1,068	13	706	6
13	1,027	18	1,237	3	672	4	881	1
14	423	11	922	1	658	3	963	2
15	1,338	39	1,198	17	983	10	675	3
16	3,603	188	4,022	107	3,042	59	2,034	20
17	1,248	28	1,218	4	670	5	695	1
18	1,388	47	1,112	25	931	7	655	3
Total	30,559	1,069	31,418	570	24,731	279	19,720	123

These figures and table show high traffic volumes on Northbourne Avenue and Mouat Street/Antill Street in all peak periods. These roads, together with the roads surrounding the core of the Dickson precinct (i.e. Cowper Street and Morphett Street), carry the largest amount of traffic during weekday peak periods. The western and northern boundaries of the Dickson precinct experience their highest volumes during the PM peak, whilst the southern and eastern boundaries of the Dickson precinct experience the Dickson precinct experience their highest volumes during the AM peak, possible due to the location of Daramalan College.

During the weekend peaks, the internal roads of Dickson Precinct (e.g. Badham Street and Dickson Place) carry more traffic during the evening peak than the midday peak period, as shown in *Figure* **18** and *Figure* **19**. The opposite trend is seen on the external roads during the two weekend peaks.

Northbourne Avenue carries most of the heavy vehicles with more than 150 heavy vehicles during the AM peak period but this is only a small proportion of the total traffic volume.

#### 3.3.2 Micro-simulation Modelling Calibration Results

Micro-simulation models for the four peak periods were developed for the assessment of the Do Nothing and master plan scenarios. Below is a summary of the process and the calibration results.

Origin – Destination (OD) matrix estimation was performed using Paramics Estimator V6, with input from intersection turn counts conducted as part of this study. Estimator generates an OD matrix by iteratively adjusting a previous OD matrix to provide a trip pattern that more closely matches the supplied count data.

The accuracy of OD matrix estimation is gauged by the GEH statistic, which compares the modelled (M) volumes to the counted (C) volumes. It has the effect that as C increases, the allowable difference as a proportion of C decreases. The GEH statistic is given by the equation:

$$GEH = \sqrt{\frac{2(M-C)^2}{M+C}}$$

Where:

*M* : traffic volume estimated by the model

*C* : actual (real-world) traffic volume

Where possible, the GEH for 85% of the estimated volumes should be less than 5, and ideally no estimated volumes should have a GEH greater than 10. The resulting GEH statistics from the calibration of the base network model for each peak period are shown in *Table* **5**. In the micro-simulation modelling tasks done for this study, the estimated matrices were optimised in terms of its GEH as far as practical. Whilst there are a few GEH values above 10, they have been assessed individually and are considered acceptable.

Peak Period	Average GEH	GEH<5	5<=GEH<10	10<=GEH
Weekday AM Peak	1.81	182 (93%)	11 (6%)	2 (1%)
Weekday PM Peak	2.04	177 (91%)	17 (9%)	1 (1%)
Weekend MD Peak	2.11	177 (91%)	14 (7%)	4 (2%)
Weekend EV Peak	2.29	174 (89%)	16 (8%)	5 (3%)

Table 5: Summary of Paramics Estimation Results

The seed values that determine the release pattern of vehicles in the model were selected for each peak period based on the lowest GEH value for turning movements.

Table 6: Seed Values Selected for Each Peak Period

Peak Period	Seed Value
Weekday AM Peak	7771
Weekday PM Peak	2849
Weekend MD Peak	5321
Weekend EV Peak	28

#### 3.3.3 Road Network Hierarchy Assessment

The road network in the study area, along with the hierarchy, is shown in Figure 20.



Figure 20: Study Area Road Network and Hierarchy (Source: TAMSD)

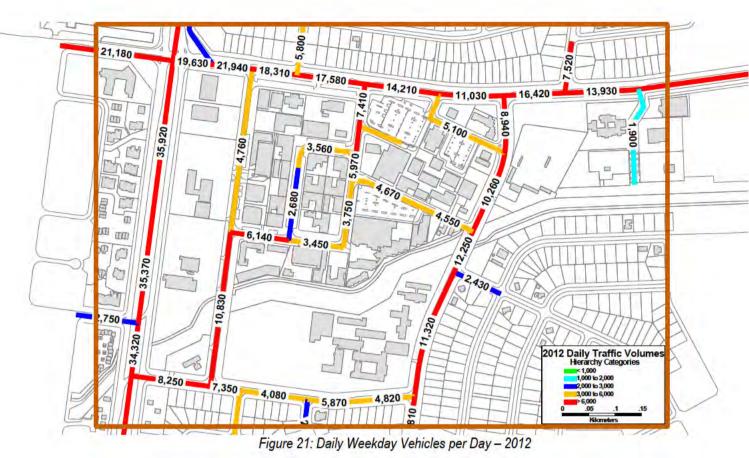
One of the criteria used by the ACT Residential Subdivision Code to define the road hierarchy is the average daily traffic in vehicles per day (vpd). This classification has been reproduced in *Table* **7**.

Road Classification	Indicative Traffic Volume (vpd			
Local Access	0-1000			
Local Access C	1001-2000			
Minor Collector	1000-3000			
Major collector	3000-6000			

Table 7 (	Classification	of Roads in	Hierarchy
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Source: ACT Residential Subdivision Code

*Figure 21* provides an indication of daily weekday traffic volumes in 2012. These volumes were calculated by taking the AM and PM peak volumes from the micro-simulation modelling outputs and assuming a peak factor of 10% to determine the daily flow.



*Figure 21* shows that, in 2012, the volumes on most roads are not appropriate for the classified road hierarchy. In particular, Antill Street, Cowper Street and Challis Street appear to have volumes that are not appropriate for their classification.

#### 3.3.4 Intersection Analysis

Intersection analysis was conducted using SIDRA Intersection. Intersection performance measures such as Level of Service (from delay and degree of saturation) and queue length at the specified key intersections within the study area provided a quantitative basis on the performance of these major junctions in the four peak periods analysed. The Highway Capacity Manual (HCM) criteria for the evaluation of intersection Level of Service (LoS) are given in Table 8.

Level of Service	Signalised/Roundabout	Stop/Give Way	Colour
А	D < 10s	D < 10s	
В	10s ≤ D < 20s	10s ≤ D < 15s	
С	20s ≤ D < 35s	15s ≤ D < 25s	
D	35s ≤ D < 55s	25s ≤ D < 35s	
Ē	55s ≤ D < 80s	35s ≤ D < 50s	
F	D ≥ 80s	D ≥ 50s	

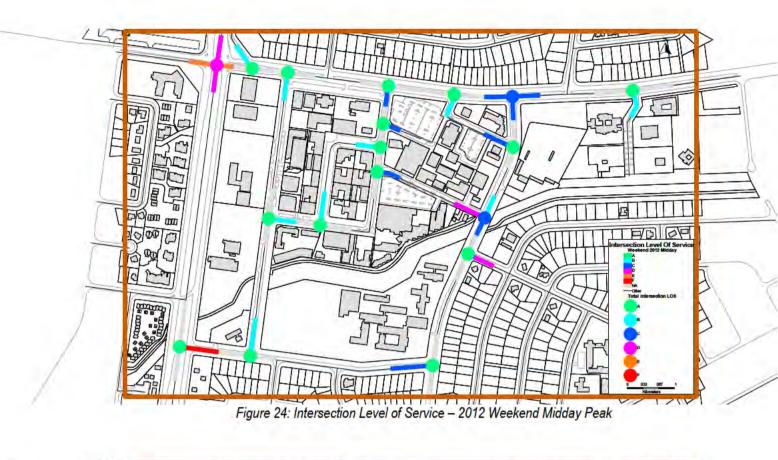
Source: Highway Capacity Manual 2000, Exhibit 16-2 (p.16-2) and 17-2 (p.17-2)

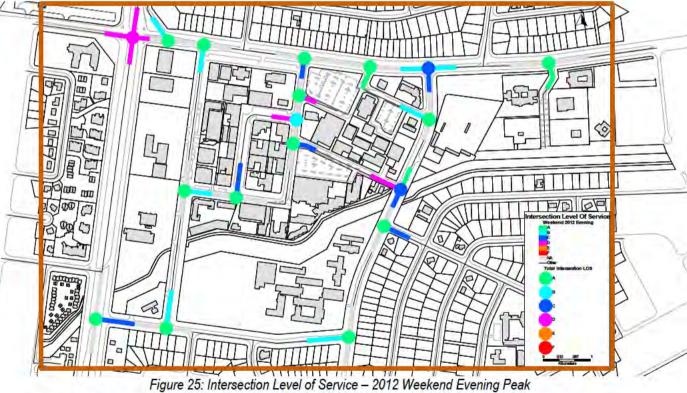
The following 21 intersections were analysed:

- Northbourne Avenue Antill Street / Mouat Street
- Northbourne Avenue Service Road Antill Street
- Antill Street Challis Street
- Antill Street Badham Street
- Antill Street Dickson Shops Access Road
- Antill Street Cowper Street
- Antill Street Rosevear Place
- Badham Street Dickson Shop Access Road
- Cowper Street Dickson Shop Access Road
- Badham Street Woolley Street
- Badham Street Dickson Place
- Cowper Street Dickson Place
- Challis Street Cape Street
- Cape Street Woolley Street
- Cowper Street Davenport Street
- Northbourne Avenue Morphett Street
- Morphett Street Challis Street
- Cowper Street Morphett Street

Consistent with the survey periods, these intersections were modelled for the weekday AM, weekday PM, weekend midday and weekend evening peak periods. *Figure* **22** through *Figure* **25** show a graphical representation of the modelled performance of each intersection for each peak period in 2012. Detailed intersection analysis results for the current situation are shown in Appendix A.







The results of the intersection analysis show that there are some areas of concern in the current network, especially in the weekday AM and PM peaks. In the weekday AM peak, the intersection of Northbourne Avenue with Mouat Street and Antill Street is expected to operate at LoS F, with both Mouat Street and Antill Street operating at LoS F. In addition, the following roads are expected to operate at LoS F:

Morphett Street, at its intersection with Northbourne Avenue

- Morphett Street, at its intersection with Cowper Street
- Davenport Street, at its intersection with Cowper Street

In the weekday PM peak, the following locations are expected to operate at LoS F:

- Intersection of Northbourne Avenue with Mouat Street and Antill Street, on Northbourne Avenue (northbound), Mouat Street and Antill Street approaches
- Morphett Street, at its intersection with Cowper Street
- Davenport Street, at its intersection with Cowper Street

During the weekend midday peak, the only area operating at LoS F is Morphett Street, at its intersection with Northbourne Avenue.

There are no intersections or approaches to intersections operating at LoS F in the weekend evening peak period.

# 3.4 Summary of Existing Issues and Potential Solutions

*Figure* **26** shows the recommended changes in the Dickson Precinct to address the identified issues in the current situation.

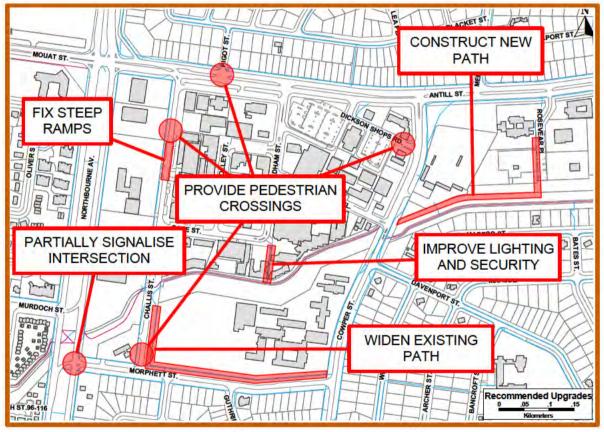


Figure 26: Recommended Upgrades in 2012

These recommendations are discussed in more detail in the following sections.

#### 3.4.1 Pedestrian and Cyclist Facilities

The following changes to the existing pedestrian and cyclist facilities, based on the site inspection, are recommended:

 Provide a pedestrian crossing on Challis Street near its intersection with Morphett Street

- Provide a pedestrian crossing on Challis Street near the Telstra Building
- Provide a pedestrian crossing on Antill Street near its intersection with Pigot Street in the short term
- Provide a pedestrian crossing on Dickson Shops Road close to the intersection with Cowper Street
- Widen the 1.2m concrete paths around Daramalan College to 2.0m wide
- Provide better lighting on the path extension from Badham Street to the shared path to the south of the precinct to improve security
- Ensure pedestrian ramps along Challis Street have appropriate steepness for wheelchair access.
- Construct a new pedestrian/cyclist path connection from north of Rosevear Place to shared path to the south and swimming pool

### 3.4.2 Car Parking

The analysis of the car parking supply and utilisation has shown that there appears to be sufficient car parking capacity in Dickson for current operations. However, the car parking is not evenly utilised across the centre. It is recommended that appropriate signage be implemented to direct drivers to car parks that are shown to be underutilised in this study. The large car parks that are currently underutilised are:

- Dickson pool car park (this is likely to be variable with weather and season)
- Surface car park on the southern side of Dickson Place
- Underground car park underneath Dickson Tradies Club

Better signage, along with the upgrades to the pedestrian facilities presented above, should allow more efficient usage of current parking facilities.

#### 3.4.3 Road Network and Intersections

The following changes to the existing road network and intersections are recommended to address current performance issues:

Partial signalisation of the intersection of Northbourne Avenue and Morphett Street

This signalisation would only affect the eastern (southbound) carriageway of Northbourne Avenue. The signals would need to be linked to the existing signalised pedestrian crossing immediately to the north to ensure efficient operation.

These signals would also allow safe crossing of Morphett Street for pedestrians and offroad cyclists travelling north or south along the eastern side of Northbourne Avenue.

While the intersection analysis results indicate that the intersection of Northbourne Avenue with Antill Street and Mouat Street is currently operating at Level of Service F in both the AM and PM peak periods, it is recommended that the *Gungahlin to City Transitway Feasibility Study* (currently in progress) investigate options to improve the performance at this location. That study is expected to recommend significant changes to Northbourne Avenue to address transit operations and is better placed to make recommendations about this intersection.

# 4 ASSESSMENT OF MASTER PLAN

The proposed master plan developments have been discussed here in two stages. The first is the short term plan to develop Blocks 19 and 21 into two new supermarkets with supporting specialty retail shops. The predicted traffic impact of this development has already been assessed by Brown Consulting in 2011 and the outcomes of that assessment are reviewed here. An in depth analysis, which would essentially duplicate work already undertaken, has not been carried out.

The second stage of the assessment is the long term master plan options, assumed to be implemented by 2031. A more detailed analysis of the long term has been carried out, including traffic modelling and an indicative forecast of future parking requirements.

# 4.1 Analysis of Short Term Plan

Brown Consulting has already conducted an in depth analysis of the impact of an indicative level of development for the proposed supermarket sites, which found that the developments were not expected to have a significant impact on traffic operations in the area.

Based on the indicative level of development assumed by Brown Consulting and the ACT Parking and Vehicular Access General Code parking provision requirements, an extra 326 parking spaces would be required. Additionally, replacement of the existing spaces removed by the development would be required. The following section discusses a brief review of the parking analysis shown in Development Traffic Assessment Report for Block 19 and 21, Dickson Shops, Dickson, ACT (Brown Consulting, June 2011) based on more recent parking utilisation surveys carried out for this project.

### 4.1.1 Short Term Parking Requirements During Construction

The Dickson master plan indicates that parking Areas 16 and 19 are expected to be redeveloped with supermarkets and supporting specialty shops. These parking areas are located in Block 21 and Block 19, respectively.

*Table* **9** shows the existing utilisation of the parking Areas 16 and 19 (Blocks 21 and 19). It should be noted that parking areas in the Retail Core Precinct are currently near capacity during the weekday PM peak and weekend midday peak periods. Car parking areas operating at very high utilisation levels typically have a large number of cars circulating repeatedly searching for spaces, which could potentially lead to traffic congestion issues.

Location	n Supply		Thursday, 16 February 2012							Weekend, 18 February 2012					
Location		7:00	8:00	9:00	16:00	17:00	18:00	11:00	12:00	13:00	18:00	19:00	20:00		
Area 16 (Block 21)	242	23	102	116	182	180	191	195	233	228	159	193	177		
Area 19 (Block 19)	135	21	36	72	112	69	44	113	123	124	26	32	27		
Total	977	44	138	188	294	249	235	308	356	352	185	225	204		
Total	377	12%	37%	50%	78%	66%	62%	82%	94%	93%	49%	60%	54%		

#### Table 9 Summary of Parking Utilisation Survey for Parking Areas 16 and 19

*Table* **9** shows that Block 21 has a total of 242 parking spaces and block 19 has a total of 135 parking spaces (including taxi zone and motorcycle parking areas). This table also shows that the peak parking demand of Areas 16 and 19 is 356 vehicles and occurs at 12:00 PM on weekends, which is the same as the adjacent parking areas in the Retail Core Precinct.

Temporary removal of either of these parking areas will put significant parking pressure on adjacent parking areas and the surrounding road network as consumers search for parking spaces, so this demand should be met through the use of adjacent parking areas and temporary car parks.

It is expected that some of the parking demand from Areas 16 and 19 will be absorbed by the adjacent parking areas during the construction period. After construction is complete. the developments are expected to replace the existing parking supply and provide whatever additional capacity is required by the new development. The ACT Parking and Vehicular Access General Code requires that, in Commercial 'C' areas in Group Centres like Dickson, short stay parking may be provided on-site or within 200 metres of a development, while long stay parking may be provided on-site or within 400 metres of a development. The dispensation to allow provision of parking off-site depends on there being spare capacity in existing publicly provided parking spaces, whether on-street or offstreet. Therefore, the most likely existing parking areas to be utilised are Areas 8, 9, 10, 12, 17 (McDonalds 30 minute parking), 18, 20 and 23. While Area 12 is in a location that makes it possible to be utilised, its on-street nature is likely to make it less attractive to supermarket shoppers who often use trolleys to transport goods to their cars. Table 10 shows the number of unused parking spaces in nearby parking areas. It is also noted that demand for parking in Area 23 is likely to be seasonal. The parking surveys were conducted in February and the area is approximately 65% utilised during the weekend midday peak. During winter, when the Aquatic Centre is closed, there is likely to be less demand in this area.

Parking		Thurs	sday, 1	6 Februa	ary 2012			Week	end, 18 I	February	y 2012	
Area	7:00	8:00	9:00	16:00	17:00	18:00	11:00	12:00	13:00	18:00	19:00	20:00
Area 8	53	28	10	25	44	59	60	56	55	62	41	17
Area 9	30	28	27	16	23	21	18	12	17	6	1	1
Area 10	58	55	52	50	47	43	44	37	38	22	18	17
Area 12	128	130	116	78	63	31	38	32	26	24	20	20
Area 17	17	13	10	6	11	15	19	16	14	21	23	7
Area 18	36	23	13	4	12	34	16	25	31	31	35	35
Area 20	119	102	80	47	63	40	7	13	7	74	13	5
Area 23	156	152	125	54	128	137	53	49	59	152	176	181
Total Available Spaces	597	531	433	280	391	380	255	240	247	392	327	283

Table 10: Available Parking Space in Areas near Blocks 19 and 21

Based on the available parking spaces around Blocks 19 and 21, it appears that Block 19 (which contains 135 parking spaces) can be redeveloped without the need to provide additional parking during construction. However, appropriate signage must be used to redirect users to the parking areas they should use while construction is underway.

If the development of Block 21 (which contains 242 parking spaces) can be staged, additional temporary parking spaces may not be required. If staging is not possible for Block 21, it is recommended to provide temporary parking spaces as close to Area 16 as is feasible.

It is noted that there will be an increase in circulating traffic caused by drivers looking for free spaces in highly utilised parking areas. To reduce this impact it is recommended to install temporary signs directing drivers to alternative parking areas (particularly for Area 23). It is expected that this work will be conducted as part of the temporary traffic management plan.

# 4.2 Analysis of Long Term Master Plan

The impacts of the master plan developments on transport operations in Dickson were assessed, regarding:

- Pedestrian and cycle network
- Car parking requirements
- Road hierarchy
- Intersection level of service

These investigations are discussed further in the following sections.

### 4.2.1 Pedestrian and Cycle Network

*Figure 5* to *Figure 8* show that Dickson appears to have significant east-west pedestrian and cyclist movements. However, the main north-south movements occur on Northbourne Avenue only. This is likely to be due to lack of appropriate pedestrian crossings on Antill Street and Morphett Street. It is noted that the one signalised crossing at the intersection of Antill Street and Dickson Shops Road carries relatively less pedestrian and cyclists compared to the other crossing facilities surrounding Dickson. For the long term scenario it is recommended to increase the north-south permeability of the precinct by improving the crossing facilities that provide access to the centre of Dickson. This can be achieved by signalising the Antill Street – Badham Street intersection, and improving access from Badham and Cape Streets to the shared path north of Daramalan College. In addition, pedestrian access from the south can be improved by signalising the intersection of Morphett Street and Challis Street and continuing the paved path on the western end of Daramalan College.

The master plan proposes several external connections to the surrounding area as shown in *Figure* **27**. These external connections are consistent with the recommendations made above. Similarly, the master plan proposes new internal links and improvements to the existing links as shown in *Figure* **28**. The new east-west internal links will allow better access to Challis Street, which will be very important if the planned major bus station is located along this street.

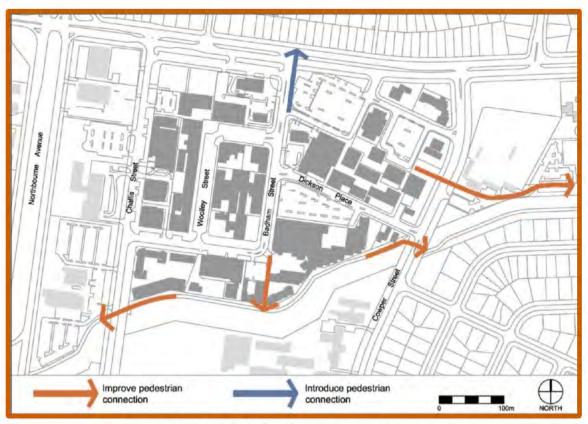


Figure 27: External Pedestrian Access (Source: Dickson master plan)

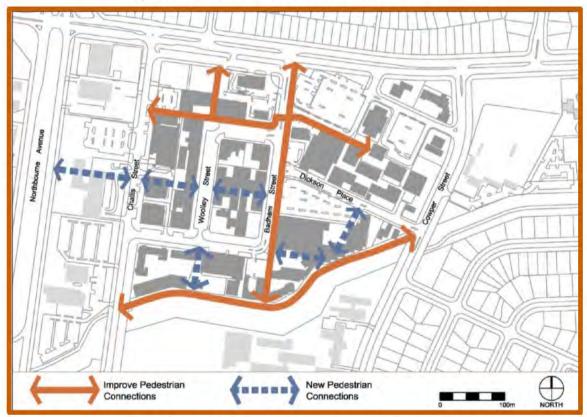


Figure 28: Internal Pedestrian Access (Source: Dickson master plan)

## 4.2.2 Long Term Parking Requirements

The master plan does not include enough details regarding the proposed land use to conduct an accurate assessment of the number of parking spaces required in the future.

However, a basic assessment has been carried out based on the indicative changes in land use shown in *Table* **11**.

Land Use	2016	2021	2031 (Without Master Plan)	2031 (With Master Plan)
Population	101	250	280	1,890
Employment	2,880	3,306	3,329	4,590
Retail Space (m <sup>2</sup> )	31,945	34,130	36,000	40,800

Table 11: Indicative Future Land Use in Dickson

Using current parking provision rates and making assumptions about residential unit occupancy and employment rates per square metre of commercial development, the Dickson Group Centre is expected to require approximately the number of parking spaces shown in *Table* **12**.

Land Use	2012	2016	2021	2031 (Without Master Plan)	2031 (With Master Plan)
Population	-	90	220	250	1,670
Employment	-	1,440	1,650	1,660	2,300
Retail Space (m <sup>2</sup> )	-	1,600	1,710	1,800	2,040
Total	2,509	3,130	3,580	3,710	6,010

Table 12: Indicative Future Parking Requirements in Dickson

Based on rates from the ACT Parking and Vehicular Access General Code and assumptions about residential unit occupancy and employment rates.

This is an increase to nearly 2.5 times the current supply if the master plan is implemented. It is assumed that the parking associated with population (residential) and employment will be contained within individual developments and will not be publicly accessible. The parking associated with retail development should be accessible to the general public.

The large increase in vehicles entering and exiting the Dickson Precinct to use this parking may have a significant impact on the performance of the road network and intersections in and around Dickson.

It is also noted that the 2031 scenario without the master plan (i.e. the do nothing scenario) includes some development and population, employment and retail space are all expected to increase. The master plan allows for higher levels of development, especially in residential and employment numbers.

### 4.2.3 Road Network Hierarchy Assessment

*Figure* **29** and *Figure* **30** show that the indicative road hierarchies (based only on traffic volumes) in 2031, both with and without master plan scenarios, are very similar.



Figure 30: Predicted Vehicles per Day - 2031 With Master Plan

Figure 29 shows that even in the do nothing scenario, there is expected to be an increase in development and thus, an increase in traffic.

Cape Street Extension, east of Badham Street, is expected to carry approximately 6,800 vehicles per day. West of Challis Street, Cape Street Extension is expected to carry

7,800 vehicles per day, which places it in the arterial category, based on traffic volumes. However, the daily traffic volumes shown here are indicative and the code allows for some flexibility in road hierarchy planning. It is recommended that Cowper Street and Antill Street be upgraded to arterial roads while Cape Street and Badham Street should become major collectors. Challis Street and Morphett Street are likely to remain as major collectors.

*Figure 31* shows the expected differences in daily traffic volumes in 2031 if the master plan is implemented.

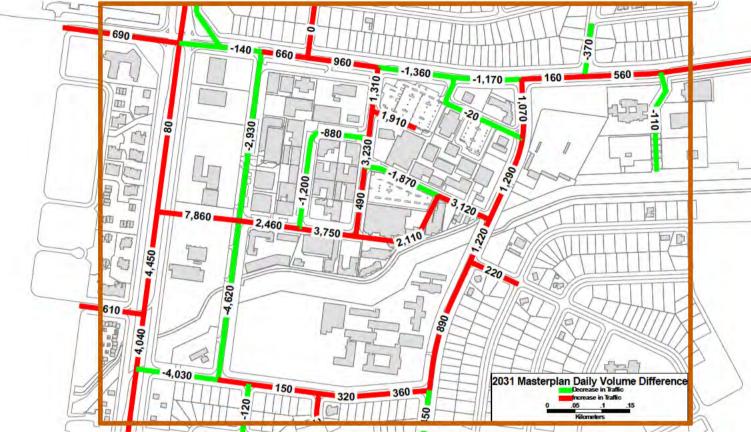


Figure 31: Daily Traffic Differences in 2031 after Master Plan Implementation

The major reductions in traffic are on Challis Street and Morphett Street. This is mainly due to the Cape Street extension to Northbourne Avenue. Similarly, there is a reduction on Antill Street between Cowper Street and Badham Street which appears to be due to the extension of the eastern end of Cape Street to Dickson Place. Small increases in traffic are noted on Mouat Street, Antill Street (east of Cowper Street) and Murdoch Street.

While access arrangements appear to be appropriate for the proposed hierarchy, as more traffic travels on Dickson Place, Badham Street and Cape Street, the access arrangements to developments on these streets may need to be reconsidered. Of particular note are the:

- Woolworths loading dock at the western end of Dickson Place
- 90 degree parking on Cape Street

These facilities may not be appropriate for the relatively high volumes of traffic expected on these roads.

### 4.2.4 Intersection Analysis

In addition to the 21 intersection that were analysed in Section 3.3.4, the following three intersections, related to the extension of Cape Street at both the eastern and western ends, were analysed for the master plan scenario:

- Northbourne Avenue Cape Street
- Cape Street Badham Street
- Dickson Place Cape Street

Again, these intersections were modelled for the weekday AM, PM and Weekend mid-day and evening peak periods using volumes taken from the micro-simulation modelling. *Figure 32* through *Figure 35* show a graphical representation of the expected performance of each intersection for each of the four peak periods in 2031, assuming that the master plan developments do not go ahead. Detailed intersection analysis results for the future scenarios are shown in Appendix A.

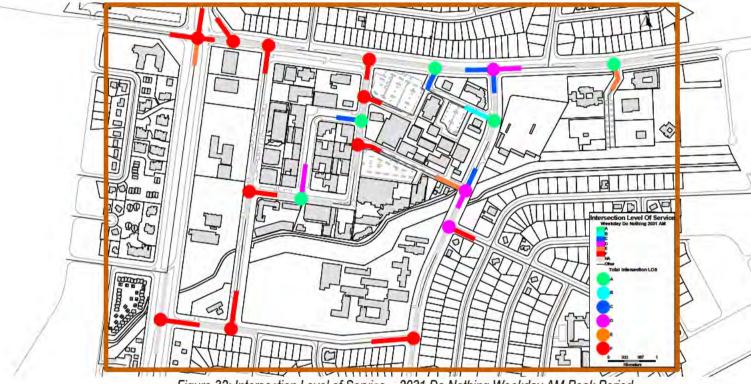


Figure 32: Intersection Level of Service - 2031 Do Nothing Weekday AM Peak Period



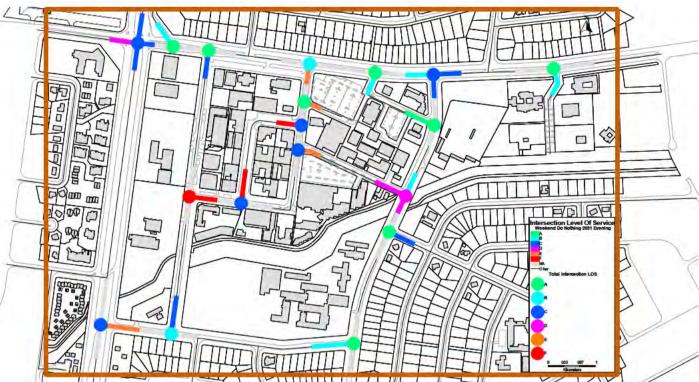


Figure 35: Intersection Level of Service – 2031 Do Nothing Weekend Evening Peak

From the preceding figures, it can be seen that there are a number of intersections and links that are expected to operate at Level of Service F without the master plan developments. The intersections include:

- Northbourne Avenue Antill Street / Mouat Street (AM and PM peaks)
- Northbourne Avenue Morphett Street (AM peak)
- Antill Street Northbourne Avenue Service Road (AM peak)
- Antill Street Challis Street (AM and PM peaks)
- Challis Street Cape Street (AM, PM, weekend midday and weekend evening peaks)
- Challis Street Morphett Street (AM peak)
- Antill Street Badham Street (AM, PM and weekend midday peaks)
- Badham Street Dickson Shops Road (AM and weekend midday peaks)
- Badham Street Dickson Place (AM and weekend midday peaks)
- Cowper Street Morphett Street (AM peak)

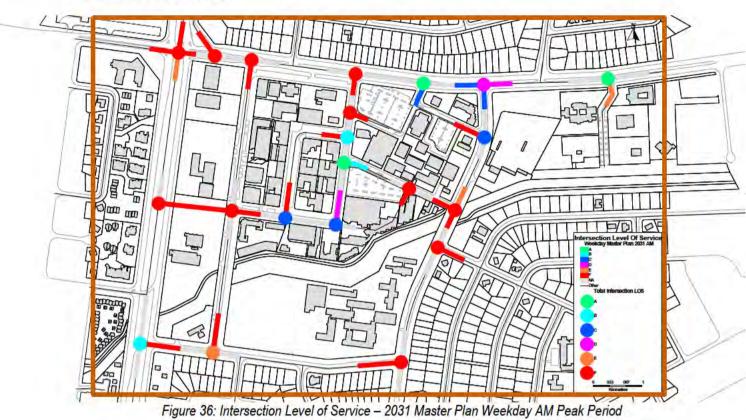
In addition to these intersections, the following roads are expected to operate at Level of Service F:

- Davenport Street, at its intersection with Cowper Street (AM and PM peaks)
- Morphett Street, at its intersection with Northbourne Avenue (PM and weekend midday peaks)
- Woolley Street, at its intersection with Badham Street (weekend midday and weekend evening peaks)
- Woolley Street, at its intersection with Cape Street (weekend evening peak)

The operation of these intersections worsens in the future for two reasons. The first is that, even in the do nothing scenario, there is expected to be development in Dickson, as discussed in Section 4.2.2. The second reason is that the traffic growth has been taken

from a strategic model that includes all of Canberra. As development occurs in other parts of Canberra, traffic along major roads across Canberra is expected to increase.

*Figure* **36** through *Figure* **39** show a graphical representation of the expected performance of each intersection for each of the four peak periods in 2031, assuming that the master plan developments go ahead. Detailed intersection analysis results for the future scenarios are shown in Appendix A.





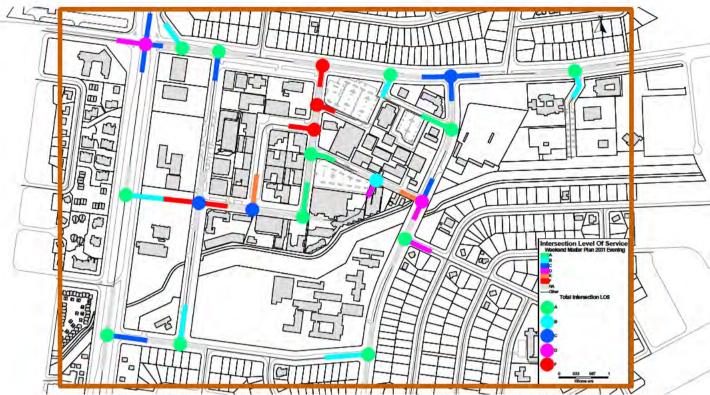


Figure 39: Intersection Level of Service - 2031 Master Plan Weekend Evening Peak Period

From the preceding figures, it can be seen that there are a number of intersections and links that are expected to operate at Level of Service F if the master plan developments are implemented. The intersections include:

- Northbourne Avenue Antill Street / Mouat Street (AM and PM peaks)
- Antill Street Northbourne Avenue Service Road (AM peak)
- Antill Street Challis Street (AM and PM peaks)
- Challis Street Cape Street (AM, PM and weekend evening peaks)
- Antill Street Badham Street (AM, PM, weekend midday and weekend evening peaks)
- Badham Street Dickson Shops Road (AM, weekend midday and weekend evening peaks)
- Badham Street Woolley Street (PM, weekend midday and weekend evening peaks)
- Cowper Street Dickson Place (AM and weekend midday peaks)
- Cowper Street Davenport Street (AM peak)
- Cowper Street Morphett Street (AM peak)

In addition to these intersections, the following roads are expected to operate at Level of Service F in 2031 if the master plan is implemented:

- Morphett Street, at its intersection with Northbourne Avenue (AM and weekend midday peaks)
- Challis Street, at its intersection with Morphett Street (AM peak)
- Woolley Street, at its intersection with Badham Street (weekend midday peak)
- Woolley Street, at its intersection with Cape Street (AM and PM peaks)
- Dickson Place, at its intersection with Cowper Street (PM peak)

- Dickson Shops Road, at its intersection with Cowper Street (AM peak)
- Davenport Street, at its intersection with Cowper Street (PM peak)

Recommendations to address these performance issues are presented in the following sections.

## 4.3 Summary of Issues Found and Potential Solutions

A number of recommendations have been made to allow the implementation of the master plan. These relate to:

- Pedestrian and cyclist facilities
- Car parking
- Road network and intersections

These recommendations are shown in Figure 40 and discussed in the following sections.

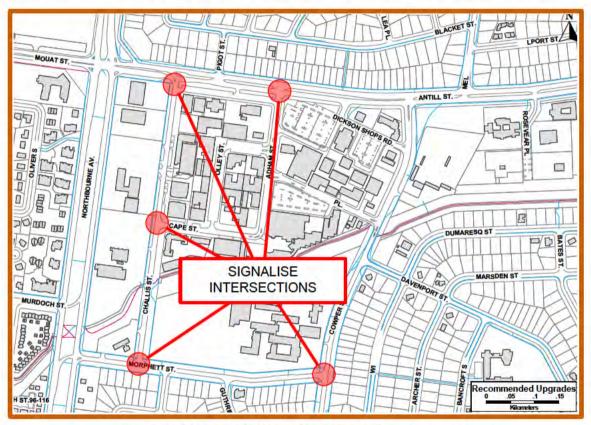


Figure 40: Recommended Upgrades for the 2031 Master Plan Scenario

*Figure* **41** through *Figure* **45** show the recommended layouts for the intersection upgrades shown in *Figure* **40**. Signalisation of these intersections is recommended over any other intersection upgrade. The intersections will not operate at an acceptable level of service if they remain priority controlled. Converting them to roundabouts, while providing some level of traffic calming, would take additional space and would not cater well to pedestrians. The midblock capacities of the roads appear to be sufficient and no major widening is required. The only widening required is additional turning lanes at the intersections, as shown in the figures below.

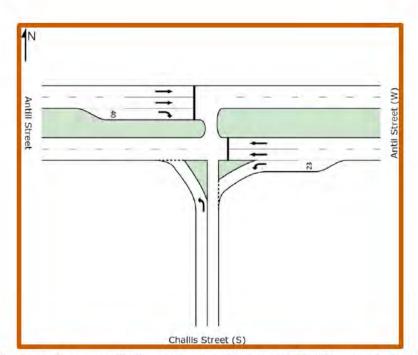


Figure 41: Recommended Layout for the Intersection of Antill Street and Challis Street

The recommended intersection Antill Street and Challis Street retains the existing footprint and number of lanes on each approach. The signalisation will allow better access and egress to and from Challis Street for cars and pedestrians without unduly delaying traffic on Antill Street.

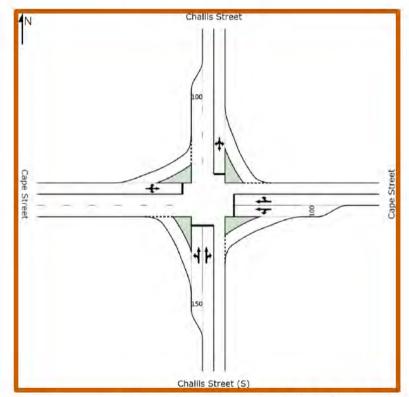


Figure 42: Recommended Layout for the Intersection of Challis Street and Cape Street

The intersection of Challis Street and Cape Street is proposed to be upgraded to a fourway intersection in the master plan. Four-way priority controlled intersections tend to perform poorly and this is no exception. Signalising this intersection improves the traffic performance and has the additional benefit of improving pedestrian accessibility. Converting this intersection to a roundabout is not recommended because of the pedestrian accessibility issues a roundabout would create.

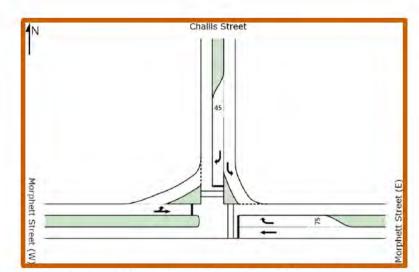


Figure 43: Recommended Layout for the Intersection of Morphett Street and Challis Street

The intersection of Morphett Street and Challis Street is also recommended to be signalised. This allows access and egress to and from Challis Street without overly delaying traffic on Morphett Street. As with the other intersections, pedestrian accessibility is important, especially this close to Daramalan College. The signals here will increase the safety for students walking and cycling to school.

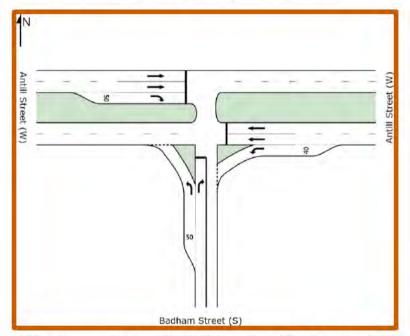


Figure 44: Recommended Layout for the Intersection of Antill Street and Badham Street

Signalising the intersection of Antill Street and Badham Street will allow better access to and from the central part of the Dickson Centre.

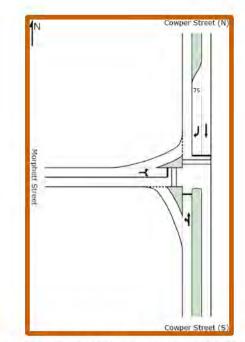
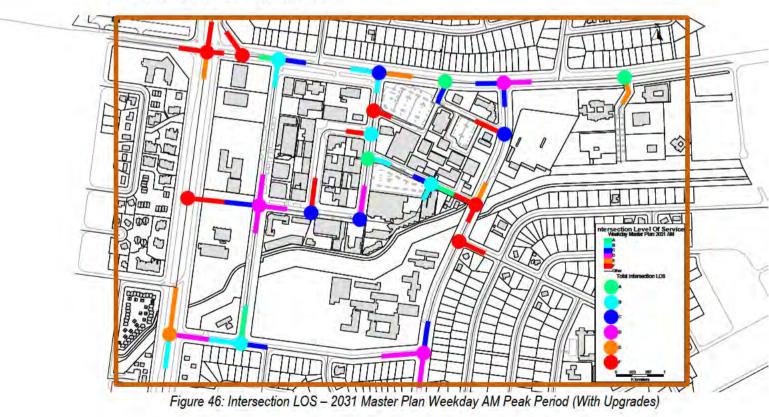


Figure 45: Recommended Layout for the Intersection of Morphett Street and Cowper Street

Signalising the intersection of Morphett Street and Cowper Street is expected to improve the traffic performance and also allow students to access Daramalan College much more safely than the current layout.

*Figure* **46** through *Figure* **49** show the intersection levels of service for the four peak periods in 2031, assuming that the master plan and the recommended intersection layouts presented above are implemented.





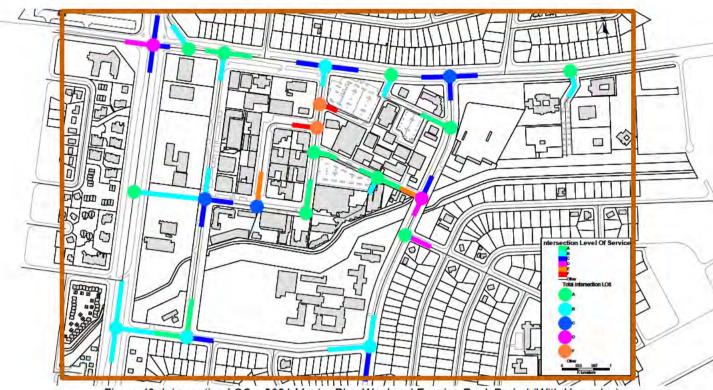


Figure 49: Intersection LOS – 2031 Master Plan Weekend Evening Peak Period (With Upgrades)

Table 13 summarises the differences in levels of service due to the recommended intersection upgrades.

the Second second	LO	S (Ma	ster Pl	LOS (With Upgrades)				
Intersection	AM	PM	MD	EV	AM	PM	MD	EV
Antill Street – Challis Street	F	F	А	А	в	А	А	Α
Antill Street – Badham Street	F	F	F	F	С	с	С	в
Challis Street – Cape Street	F	F	в	С	D	с	С	С
Northbourne Avenue – Morphett Street	в	А	А	A	E	в	в	в
Challis Street – Morphett Street	E	Α	А	A	в	в	в	в
Morphett Street – Cowper Street	F	A	А	A	D	В	В	в
Dickson Place – Cape Street Extension	F	в	F	в	в	A	в	Α

Table 13: Comparison of	Levels of	Service	(2031)
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From Table 13, it can be seen that the recommended upgrades provide a benefit in nearly all cases. An apparent exception to this is the intersection of Northbourne Avenue and Morphett Street, which has a worse overall level of service. While the overall level of service has worsened, delays on Morphett Street itself have decreased from LOS F to LOS D in the weekday AM peak and from LOS D to LOS B in all other peaks. However, southbound traffic on Northbourne Avenue is slightly delayed, going from no delay in any peak to LOS E in the AM peak and LOS B in other peaks. There is no impact on northbound traffic at this intersection.

It is noted that there are some intersections that are expected to perform poorly, especially inside the Dickson precinct. No recommendations have been made for these intersections at this stage as the focus has been on the performance of the surrounding arterial and collector road network. Intersection performance inside the centre will need to be assessed in more detail as the master plan developments are implemented over the next 20 years and details such as car park entry/exit locations and capacities are known.

## 4.3.1 Pedestrian and Cyclist Facility Improvements

The following improvements to the pedestrian and cyclist facilities are recommended:

- Providing north/south external links into Dickson by signalising the intersection of Antill Street and Badham Street
- Improving pedestrian safety at the intersection of Morphett Street and Challis Street (preferably by signalisation)
- Improving pedestrian safety at the intersection of Morphett Street and Cowper Street (preferably by signalisation)

Provide new pedestrian connections inside Dickson Precinct as shown in the master plan (*Figure 28*). It is assumed that these internal links will be developed as the surrounding buildings are further developed.

### 4.3.2 Car Parking Options

It is recommended to develop the proposed supermarkets and associated retail facilities on Blocks 19 and 21 in stages. If possible, Block 21 should also be developed in stages. By leaving some of the parking area undisturbed it is possible to for the adjacent parking areas to provide the additional parking spaces needed during the construction phase.

The master plan does not include enough details regarding the proposed land use to accurately predict the number of parking spaces required in the future. However, a brief assessment indicates that the requirement for parking is likely to rise from approximately 2,500 in 2012 to approximately 6,000 in 2031. All proposed developments should meet the parking requirements set out in the *ACT Parking and Vehicular Access General Code* or whichever code is relevant at the time of development.

In the future, it may be possible to reduce the parking requirements by considering dual use or shared parking. Dual use parking, referred to in Brown Consulting (2011), is where a number of land uses share a parking area. Users are able to visit more than one land use without requiring a second car parking space. This sharing of parking spaces has already been taken into account in the requirements of the ACT general code.

### 4.3.3 Road Network and Intersection Options

The analysis of the road network and intersection performance has identified a number of locations where the level of service is expected to be poor or unacceptable. These locations are on arterial roads around the Dickson precinct and also on local streets inside the precinct. At this stage in the process, it is considered appropriate to make recommendations to improve the level of service on the arterials roads while the local streets can tolerate a higher delay. In addition, the operation of some of the local streets is highly dependent on the location of access to future parking areas, which is not yet known.

It is recommended that the following changes to intersections are made to address identified performance issues:

• Signalisation of the following intersections:

- Antill Street and Challis Street
- Challis Street and Cape Street
- Challis Street and Morphett Street

- Antill Street and Badham Street (also recommended to improve pedestrian access to Dickson from the suburbs to the north)

- Morphett Street and Cowper Street (also recommended to improve pedestrian safety around Daramalan College)

# **5 COST ESTIMATES**

The analysis of the current situation and the long term master plan has revealed a number of transport related issues that need to be addressed to enable the proposed developments to proceed. The indicative cost estimates have been calculated for each of the changes recommended in Sections 3.4 and 4.3 and these estimates are presented in the following sections.

# 5.1 Cost of Recommendation for Current Issues

The costs of the recommendations to address current identified issues have been estimated and are presented in *Table* **14**.

Recommendation	Estimated Cost
Pedestrian crossing on Challis Street near Morphett Street	\$3,800
Pedestrian crossing on Challis Street near the Telstra Building	\$3,800
Pedestrian crossing on Antill Street near Pigot Street	\$3,800
Pedestrian crossing on Dickson Shops Road close to Cowper Street	\$3,800
Widen the concrete paths around Daramalan College to 2.0m	\$155,000
Provide better lighting on the path extension from Badham Street to the shared path to the south	\$6,600
Ensure pedestrian ramps along Challis Street have appropriate steepness for wheelchair access	\$67,000
Construct a new pedestrian/cyclist path connection from Rosevear Place to the shared path to the south	\$70,000
Signalise the intersection of Morphett Street with the southbound carriageway of Northbourne Avenue	\$120,100
Subtotal	\$433,900
Contingency (40%)	\$173,560
GST (10%)	\$43,390
Total	\$650,850

Table 14: Estimated Cost of Recommended Upgrades - Current

# 5.2 Cost of Long Term Recommendations

The costs of the recommendations to address issues in the long term have been estimated and are presented in *Table* **15**.

Recommendation	Estimated Cost
Signalise the intersection of Antill Street and Challis Street	\$140,300
Signalise the intersection of Challis Street and Cape Street	\$186,100
Signalise the intersection of Challis Street and Morphett Street	\$129,500
Signalise the intersection of Antill Street and Badham Street (also recommended to improve pedestrian access to Dickson)	\$148,500
Signalise the intersection of Morphett Street and Cowper Street (also recommended to improve pedestrian safety around Daramalan College)	\$138,000
Subtotal	\$742,400
Contingency (40%)	\$296,960
GST (10%)	\$74,240
Total	\$1,113,600

### Table 15: Estimated Cost of Recommended Upgrades – Long Term

# 6 CONCLUSIONS

The current and future transport operations in Dickson have been assessed and it was found that a small number of upgrades to existing facilities are required, both now and in the future to allow the full development of the master plan.

# 6.1 Immediate/Short Term Recommendations

The current operation of the transport network in Dickson was assessed and found to be generally good. However, addressing the following improvements should be prioritised in the short term:

- Pedestrian and cyclist infrastructure and safety:
  - Provide a pedestrian crossing on Challis Street near its intersection with Morphett Street
  - Provide a pedestrian crossing on Challis Street near the Telstra Building
  - Provide a pedestrian crossing on Antill Street near its intersection with Pigot Street (short term only)
  - Provide a pedestrian crossing on Dickson Shops Road close to the intersection with Cowper Street
  - Monitor the safety of the pedestrian crossing on Challis Street north of Daramalan College and construct a raised pedestrian crossing if required
  - Widen the 1.2m concrete paths around Daramalan College to 2.0m wide
  - Provide better lighting on the path extension from Badham Street to the shared path to the south of the precinct to improve security
  - Ensure pedestrian ramps along Challis Street have appropriate steepness for wheelchair access.
  - Construct a new pedestrian/cyclist path connection from north of Rosevear Place to shared path to the south and swimming pool
- Car parking operations:
  - Implement better signage to inform users about the location of parking areas that are currently underutilised
- Road network and intersections
  - Signalise the intersection of Morphett Street with the southbound carriageway of Northbourne Avenue. The northbound carriageway would remain as it is.

These recommendations are expected to improve the transport operations and safety in Dickson in the short term.

Also in the short term, the proposed capacity of the car parks provided as part of the development of Blocks 19 and 21 needs to be consistent with the ACT general code. Initial estimates by Brown Consulting place the additional car parking requirements at 326 spaces, in addition to the existing 359 spaces that will need to be replaced by any new development.

It is recommended that the development of Blocks 19 and 21 be staged to reduce the impact on parking supply in Dickson. Block 19 may be able to be developed without provision of significant temporary car parking but the development of Block 21 is expected to require temporary parking to be provided during construction.

# 6.2 Long Term Recommendations

In the long term, the transport demands in Dickson are expected to increase significantly, both with and without the implementation of the master plan. A number of recommendations have been made to allow implementation of the master plan and these should be implemented as required. These recommendations include:

- Pedestrian and cyclist infrastructure and safety:
  - Provide north/south external links into Dickson by signalising the intersection of Antill Street and Badham Street
  - Improve pedestrian safety at the intersection of Morphett Street and Challis Street (preferably by signalisation)
  - Improve pedestrian safety at the intersection of Morphett Street and Cowper Street (preferably by signalisation)
- Car parking operations:

- Implement an area wide parking strategy to efficiently plan parking for future developments

- Road network and intersections:
  - Signalise the intersection of Antill Street and Challis Street
  - Signalise the intersection of Challis Street and Cape Street
  - Signalise the intersection of Challis Street and Morphett Street
  - Signalise the intersection of Antill Street and Badham Street (also recommended to improve pedestrian access to Dickson from the suburbs to the north)
  - Signalise the intersection of Morphett Street and Cowper Street (also recommended to improve pedestrian safety around Daramalan College)

Site: 1 AM

1- Northbourne Avenue - Antill Street - Mouat Street

2012 AM

Signals - Fixed Time Cycle Time = 125 seconds (User-Given Phase Times)

Mover	nent Pe	rformance	e - Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbou	Irne Avenue	(S)								
1	L	127	9.9	0.136	11.4	LOS B	1.8	13.6	0.30	0.67	46.0
2	Т	534	12.1	0.340	28.7	LOS C	9.6	74.3	0.74	0.62	32.2
3	R	153	6.4	1.135	211.1	LOS F	18.6	137.1	1.00	1.41	9.0
Approa	ch	815	10.6	1.135	60.3	LOS E	18.6	137.1	0.72	0.78	22.3
East: A	ntill Stree	et									
4	L	339	4.3	0.800	41.8	LOS D	17.0	123.5	0.84	0.88	28.2
5	Т	499	4.0	1.524	368.4	LOS F	64.2	464.9	1.00	1.82	5.3
6	R	156	3.4	0.809	72.6	LOS E	10.2	73.3	1.00	0.92	20.4
Approa	ch	994	4.0	1.524	210.6	LOS F	64.2	464.9	0.95	1.36	8.8
North: N	Northbou	rne Avenue	(N)								
7	L	123	3.4	0.129	14.0	LOS B	2.4	17.6	0.38	0.68	43.5
8	Т	1725	5.0	0.837	42.4	LOS D	34.2	249.5	0.98	0.94	26.5
9	R	143	3.5	1.042	137.5	LOS F	13.6	97.9	1.00	1.23	12.7
Approa	ch	1992	4.8	1.042	47.5	LOS D	34.2	249.5	0.94	0.94	25.1
West: N	/louat Str	eet (W)									
10	L	103	6.1	1.300	326.8	LOS F	75.5	539.4	1.00	1.81	6.1
11	Т	629	1.5	1.300	326.7	LOS F	82.4	588.1	1.00	1.91	5.9
12	R	661	2.9	1.300	192.3	LOS F	82.4	588.1	1.00	1.45	9.8
Approa	ch	1394	2.5	1.300	263.0	LOS F	82.4	588.1	1.00	1.69	7.3
All Vehi		5194	4.9	1.524	138.5	LOS F	82.4	588.1	0.92	1.20	12.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Northbourne Avenue Service Road - Antill Street

2012 AM

Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles													
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
North W	est: Nor	thbourne Ave	enue Ser	vice Road										
27	L	443	0.2	0.895	34.0	LOS D	12.6	88.6	0.93	1.73	30.9			
Approac	h	443	0.2	0.895	34.0	LOS D	12.6	88.6	0.93	1.73	30.9			
West: A	ntill Stre	et (W)												
10	L	1	0.0	0.248	9.1	LOS A	0.0	0.0	0.00	1.19	48.1			
11	Т	900	2.8	0.248	0.0	LOS A	0.0	0.0	0.00	0.00	60.0			
Approac	:h	901	2.8	0.248	0.0	NA	0.0	0.0	0.00	0.00	60.0			
All Vehic	cles	1344	2.0	0.895	11.2	NA	12.6	88.6	0.31	0.57	45.8			

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average

delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Challis Street 2012 AM

Giveway / Yield (Two-Way)

Mover	ment Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flo			Dela	Serv	Vehicles	Distance	Qu	Stop	Spe
		W			У	ice			eu	Rat	ed
									ed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	96	4.4	0.232	15.9	LOS C	0.9	6.3	0.71	0.90	42.0
Approa	ich	96	4.4	0.232	15.9	LOS C	0.9	6.3	0.71	0.90	42.0
East: A	ntil Stree	t (W)									
4	L	194	2.2	0.275	10.5	LOS B	1.0	7.1	0.46	0.73	46.7
5	Т	900	3.6	0.249	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ich	1094	3.4	0.275	1.9	NA	1.0	7.1	0.08	0.13	57.1
West: A	Antill Stre	et									
11	Т	919	3.8	0.254	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	425	0.7	0.759	21.2	LOS C	5.7	40.3	0.87	1.25	37.9
Approa	ich	1344	2.8	0.759	6.7	NA	5.7	40.3	0.27	0.40	50.6
All Veh	icles	2534	3.1	0.759	5.0	NA	5.7	40.3	0.21	0.30	52.8
					0.0		•		J.=.	0.00	02.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street 2012 AM Stop (Two-Way)

Move	ment Pe	rformance	- Vehic	les							
Mov IE	) Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Badham	Street (S)									
1	L	114	2.8	0.369	21.8	LOS C	1.6	11.2	0.76	1.07	38.6
3	R	23	0.0	0.369	21.2	LOS C	1.6	11.2	0.76	1.06	38.8
Approa	ach	137	2.3	0.369	21.7	LOS C	1.6	11.2	0.76	1.06	38.7
East: A	ntill Stree	et (W)									
4	L	175	3.0	0.178	9.7	LOS A	0.7	5.0	0.40	0.68	47.6
5	Т	896	3.9	0.248	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ach	1071	3.7	0.248	1.6	NA	0.7	5.0	0.07	0.11	57.5
West: /	Antill Stre	et (W)									
11	Т	609	1.9	0.334	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	392	3.0	0.667	18.8	LOS C	4.5	32.2	0.81	1.15	39.6
Approa	ach	1001	2.3	0.667	7.3	NA	4.5	32.2	0.32	0.45	50.0
South	West: Me	dian (RT Sta	ge 2)								
32	R	23	0.0	0.033	11.0	LOS B	0.1	0.7	0.52	0.74	30.5
Approa	ach	23	0.0	0.033	11.0	LOS B	0.1	0.7	0.52	0.74	30.5
All Veh	icles	2232	3.0	0.667	5.5	NA	4.5	32.2	0.23	0.33	52.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average

delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Dickson Shops Access Road

2012 AM Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average	
		Flo			Dela	Serv	Vehicles	Distance	Qu	Stop	Spe	
		W			У	ice			eu	Rat	ed	
									ed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South: Dickson Shop Access Road												
1	L	74	4.3	0.189	16.1	LOS C	0.7	4.9	0.72	0.89	41.9	
Approad	ch	74	4.3	0.189	16.1	LOS C	0.7	4.9	0.72	0.89	41.9	
East: Antill Street (W)												
4	L	72	4.4	0.300	8.3	LOS A	0.0	0.0	0.00	1.01	49.0	
5	Т	1006	4.0	0.300	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Approad	ch	1078	4.0	0.300	0.6	NA	0.0	0.0	0.00	0.07	59.1	
All Vehi	cles	1152	4.0	0.300	1.6	NA	0.7	4.9	0.05	0.12	57.6	

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average

delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Cowper Street

2012 AM

Signals - Fixed Time Cycle Time = 45 seconds (User-Given Phase Times)

Movement Performance - Vehicles													
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South:	Cowper S	Street (S)											
1	L	158	11.3	0.242	18.6	LOS B	2.4	18.3	0.70	0.77	40.0		
3	R	233	5.0	1.025	80.0	LOS F	11.0	79.9	1.00	1.51	18.8		
Approa	Approach 39		7.5	1.025	55.2	LOS E	11.0	79.9	0.88	1.21	23.9		
East: A	ntill Stree	et (E)											
4	L	259	0.6	1.108	142.6	LOS F	18.3	128.9	1.00	1.86	12.2		
5	Т	1167	2.7	0.970	45.7	LOS D	23.0	165.0	1.00	1.44	25.2		
Approa	ch	1426	2.0	1.108	63.3	LOS E	23.0	165.0	1.00	1.51	21.1		
West: A	Antill Stre	et (W)											
11	Т	367	2.4	0.303	12.4	LOS B	3.1	22.2	0.78	0.64	42.2		
12	R	256	1.2	1.098	134.1	LOS F	17.3	122.3	1.00	1.82	12.8		
Approa	ch	623	1.9	1.098	62.4	LOS E	17.3	122.3	0.87	1.12	21.7		
All Vehi	icles	2440	2.8	1.108	61.8	LOS E	23.0	165.0	0.95	1.37	21.7		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Antill Street - Rosevear Place 2012 AM

Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	) Turn	Demand Flo w		eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South:	South: Rosevear Place (S)											
1	L	73	1.4	0.357	27.3	LOS D	1.3	9.2	0.87	1.01	34.3	
3	R	7	0.0	0.357	26.8	LOS D	1.3	9.2	0.87	1.00	34.4	
Approa	ich	80	1.3	0.357	27.2	LOS D	1.3	9.2	0.87	1.01	34.3	
East: A	East: Antill Street (E)											
4	L	38	0.0	0.320	8.2	LOS A	0.0	0.0	0.00	1.05	49.0	
5	Т	1131	1.8	0.320	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Approa	ich	1168	1.7	0.320	0.3	NA	0.0	0.0	0.00	0.03	59.6	
West: A	Antill Stre	et										
11	Т	453	3.5	0.226	1.2	LOS A	1.1	8.1	0.09	0.00	57.3	
12	R	68	0.0	0.226	20.2	LOS C	1.1	8.1	0.88	1.00	39.5	
Approa	ich	521	3.0	0.226	3.7	NA	1.1	8.1	0.19	0.13	54.1	
South West: Median (RT Stage 2)		ge 2)										
32	R	7	0.0	0.015	12.3	LOS B	0.1	0.3	0.52	0.70	28.9	
Approa	Approach		0.0	0.015	12.3	LOS B	0.1	0.3	0.52	0.70	28.9	
		1777	2.1	0.357	2.5	NA	1.3	9.2	0.10	0.11	56.0	

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road 2012 AM

Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV Deg. Satn		Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m	eu	per veh	km/h
South: E	Badham	Street (S)									
2	Т	119	0.9	0.136	3.1	LOS A	0.9	6.2	0.54	0.00	49.7
3	R	57	0.0	0.136	11.6	LOS B	0.9	6.2	0.54	0.93	47.1
Approad	h	176	0.6	0.136	5.9	NA	0.9	6.2	0.54	0.30	48.8
East: Di	ckson S	hop Access F	Road (E)								
4	L	109	1.9	0.296	14.3	LOS B	1.3	9.0	0.60	0.89	43.0
6	R	44	2.4	0.296	14.6	LOS B	1.3	9.0	0.60	0.92	42.9
Approac	ch	154	2.1	0.296	14.4	LOS B	1.3	9.0	0.60	0.90	43.0
North: B	North: Badham Street (N)										
7	L	138	0.0	0.289	8.2	LOS A	0.0	0.0	0.00	0.93	49.0
8	Т	380	4.2	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	518	3.0	0.289	2.2	NA	0.0	0.0	0.00	0.25	56.6
All Vehicles		847	2.4	0.296	5.2	NA	1.3	9.0	0.22	0.38	51.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road 2012 AM

Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper \$	Street (S)									
1	L	83	2.5	0.228	8.3	LOS A	0.0	0.0	0.00	0.96	49.0
2	Т	314	9.4	0.228	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	397	8.0	0.228	1.7	NA	0.0	0.0	0.00	0.20	57.3
North: 0	Cowper S	Street (N)									
8	Т	672	0.6	0.452	4.0	LOS A	6.1	43.1	0.60	0.00	49.6
9	R	85	1.2	0.452	12.5	LOS B	6.1	43.1	0.60	1.01	47.0
Approa	ch	757	0.7	0.452	5.0	NA	6.1	43.1	0.60	0.11	49.3
West: D	Dickson S	Shop Access I	Road (W	)							
10	L	78	1.4	0.387	21.5	LOS C	1.8	13.0	0.66	0.87	37.7
12	R	49	0.0	0.387	22.0	LOS C	1.8	13.0	0.66	0.97	37.5
Approa	ch	127	0.8	0.387	21.7	LOS C	1.8	13.0	0.66	0.91	37.6
All Vehi	icles	1281	3.0	0.452	5.7	NA	6.1	43.1	0.42	0.22	49.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: X:\PROJECT\3002303 Dickson Precinct Traffic and Parking Study\015 Traffic - Intersection Flow Modelling\Dickson Intersections\_WD\_AM.sip 8000072, SMEC AUSTRALIA PTY. LTD., FLOATING Site: 9 AM

Badham Street - Woolley Street 2012 AM

Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela v	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu	Effective Stop Rat	Average Spe ed
		veh/h	%	v/c	sec		veh	m	ed	e per veh	km/h
South: E	Badham	Street (S)	/0	v/C	360	_	Ven		_	perven	KIII/11
1	L	19	5.6	0.078	8.4	LOS A	0.0	0.0	0.00	1.01	49.0
2	Т	122	0.9	0.078	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	141	1.5	0.078	1.1	NA	0.0	0.0	0.00	0.14	58.2
North: B	Badham	Street (N)									
8	Т	454	3.5	0.286	0.8	LOS A	2.0	14.6	0.34	0.00	53.7
9	R	44	4.8	0.286	9.4	LOS A	2.0	14.6	0.34	0.95	49.0
Approac	ch	498	3.6	0.286	1.5	NA	2.0	14.6	0.34	0.08	53.2
West: W	/oolley S	Street									
10	L	26	0.0	0.065	11.8	LOS B	0.2	1.6	0.33	0.61	45.3
12	R	13	0.0	0.065	12.0	LOS B	0.2	1.6	0.33	0.81	45.1
Approac	ch	39	0.0	0.065	11.8	LOS B	0.2	1.6	0.33	0.68	45.2
All Vehic	cles	678	3.0	0.286	2.0	NA	2.0	14.6	0.27	0.13	53.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place 2012 AM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Badham	Street (S)									
2	Т	93	1.1	0.150	3.6	LOS A	0.9	6.6	0.55	0.00	49.2
3	R	75	7.0	0.150	12.3	LOS B	0.9	6.6	0.55	0.93	46.3
Approa	ch	167	3.8	0.150	7.5	NA	0.9	6.6	0.55	0.41	47.9
East: D	ickson P	lace									
4	L	57	1.9	0.203	13.6	LOS B	0.8	5.4	0.57	0.79	43.6
6	R	46	0.0	0.203	13.8	LOS B	0.8	5.4	0.57	0.88	43.5
Approa	ch	103	1.0	0.203	13.7	LOS B	0.8	5.4	0.57	0.83	43.6
North: E	Badham 3	Street (N)									
7	L	299	4.2	0.307	8.3	LOS A	0.0	0.0	0.00	0.80	49.0
8	Т	242	1.7	0.307	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	541	3.1	0.307	4.6	NA	0.0	0.0	0.00	0.44	53.4
All Vehi	icles	812	3.0	0.307	6.3	NA	0.9	6.6	0.19	0.49	50.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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INTERSECTION

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Cowper Street - Dickson Place

2012 AM

Signals - Fixed Time Cycle Time = 54 seconds (User-Given Phase Times)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper \$	Street (S)									
1	L	119	0.0	0.550	19.0	LOS B	8.9	64.9	0.75	0.89	41.1
2	Т	349	6.9	0.550	10.8	LOS B	8.9	64.9	0.75	0.66	42.9
Approa	ch	468	5.2	0.550	12.9	LOS B	8.9	64.9	0.75	0.72	42.5
North: (	Cowper S	Street (N)									
8	Т	685	0.9	0.774	15.3	LOS B	16.8	118.4	0.89	0.86	39.8
9	R	26	0.0	0.110	23.7	LOS C	0.5	3.6	0.75	0.72	36.3
Approa	ch	712	0.9	0.774	15.6	LOS B	16.8	118.4	0.88	0.86	39.6
West: D	Dickson F	Place (W)									
10	L	67	15.6	0.858	40.1	LOS D	8.6	62.7	1.00	1.02	28.7
12	R	202	2.1	0.858	39.7	LOS D	8.6	62.7	1.00	1.02	28.7
Approa	ch	269	5.5	0.858	39.8	LOS D	8.6	62.7	1.00	1.02	28.7
All Vehi	icles	1449	3.1	0.858	19.2	LOS B	16.8	118.4	0.86	0.84	37.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Challis Street - Cape Street 2012 AM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Challis S	treet (S)									
2	Т	228	0.9	0.285	3.6	LOS A	2.2	15.7	0.61	0.00	48.5
3	R	135	2.3	0.285	12.1	LOS B	2.2	15.7	0.61	0.97	46.7
Approa	ch	363	1.4	0.285	6.8	NA	2.2	15.7	0.61	0.36	47.8
East: C	ape Stre	et									
4	L	201	2.6	0.539	18.9	LOS C	3.2	23.3	0.69	1.06	39.5
6	R	52	4.1	0.539	19.2	LOS C	3.2	23.3	0.69	1.04	39.5
Approa	ch	253	2.9	0.539	18.9	LOS C	3.2	23.3	0.69	1.06	39.5
North: 0	Challis St	reet									
7	L	48	4.3	0.265	8.3	LOS A	0.0	0.0	0.00	1.03	49.0
8	Т	436	1.0	0.265	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	484	1.3	0.265	0.8	NA	0.0	0.0	0.00	0.10	58.7
All Vehi	icles	1100	1.7	0.539	6.9	NA	3.2	23.3	0.36	0.41	49.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Woolley Street 2012 AM

Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flo w		eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	216	1.5	0.128	0.9	LOS A	0.8	6.0	0.34	0.00	53.7
6	R	12	9.1	0.128	9.7	LOS A	0.8	6.0	0.34	0.99	49.1
Approa	ch	227	1.9	0.128	1.3	NA	0.8	6.0	0.34	0.05	53.5
North: V	Voolley \$	Street									
7	L	11	10.0	0.069	12.3	LOS B	0.3	1.9	0.44	0.62	45.0
9	R	27	7.7	0.069	12.6	LOS B	0.3	1.9	0.44	0.78	44.8
Approa	ch	38	8.3	0.069	12.5	LOS B	0.3	1.9	0.44	0.73	44.9
West: C	ape Stre	eet (W)									
10	L	28	0.0	0.098	8.2	LOS A	0.0	0.0	0.00	0.98	49.0
11	Т	148	2.8	0.098	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	177	2.4	0.098	1.3	NA	0.0	0.0	0.00	0.16	57.9
All Vehi	cles	442	2.6	0.128	2.3	NA	0.8	6.0	0.21	0.15	54.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street 2012 AM

Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street									
2	Т	395	5.6	0.335	15.8	LOS C	7.1	52.7	1.00	0.00	38.8
3	R	42	22.5	0.335	25.2	LOS D	7.1	52.7	1.00	1.10	38.6
Approa	ch	437	7.2	0.335	16.7	NA	7.1	52.7	1.00	0.11	38.8
East: Da	avenport	t Street (E)									
4	L	46	0.0	0.841	77.4	LOS F	5.4	37.7	0.96	1.42	19.1
6	R	77	0.0	0.841	77.6	LOS F	5.4	37.7	0.96	1.34	19.1
Approa	ch	123	0.0	0.841	77.5	LOS F	5.4	37.7	0.96	1.37	19.1
North: C	Cowper S	Street									
7	Ĺ	133	1.6	0.488	8.2	LOS A	0.0	0.0	0.00	0.99	49.0
8	Т	757	1.0	0.488	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	889	1.1	0.488	1.2	NA	0.0	0.0	0.00	0.15	58.1
All Vehi	cles	1449	2.8	0.841	12.4	NA	7.1	52.7	0.38	0.24	43.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street

2012 AM

Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flo w		eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Vorthbou	Irne Avenue (	(S)								
2	Т	1031	9.3	0.197	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	102	1.7	1.700	745.7	LOS F	31.5	223.5	1.00	2.53	2.9
Approac	ch	1133	6.8	1.700	67.2	NA	31.5	223.5	0.09	0.23	21.1
East: Mo	orphett S	Street (E)									
4	L	262	2.0	4.368	3110.8	LOS F	137.4	978.3	1.00	3.87	0.7
Approac	ch	262	2.0	4.368	3110.8	LOS F	137.4	978.3	1.00	3.87	0.7
North: N	lorthbou	rne Avenue (	N)								
7	L	115	9.2	0.484	8.5	LOS A	0.0	0.0	0.00	1.02	49.0
8	Т	2492	4.2	0.484	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	2606	4.4	0.484	0.4	NA	0.0	0.0	0.00	0.04	59.4
All Vehi	cles	4001	4.9	4.368	223.0	NA	137.4	978.3	0.09	0.35	8.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average

delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street 2012 AM

Giveway / Yield (Two-Way)	
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Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flo w	HV D	eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	94	1.1	0.259	3.3	LOS A	1.5	10.6	0.56	0.00	48.5
6	R	173	1.8	0.259	11.7	LOS B	1.5	10.6	0.56	0.89	46.2
Approad	ch	266	1.6	0.259	8.8	NA	1.5	10.6	0.56	0.58	47.0
North: C	Challis St	treet									
7	L	406	1.0	0.507	12.4	LOS B	3.4	24.4	0.58	0.92	44.7
9	R	177	2.4	0.554	25.1	LOS D	3.2	22.7	0.80	1.09	35.4
Approac	ch	583	1.4	0.554	16.3	LOS C	3.4	24.4	0.65	0.97	41.4
West: M	lorphett	Street (W)									
10	L	280	1.1	0.281	8.2	LOS A	0.0	0.0	0.00	0.80	49.0
11	Т	214	6.4	0.281	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	494	3.4	0.281	4.7	NA	0.0	0.0	0.00	0.45	53.2
All Vehi	cles	1343	2.2	0.554	10.5	NA	3.4	24.4	0.39	0.70	46.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street 2012 AM

Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flo w		eg. Satn	Average Dela y	Level of Serv ice	95% Back Vehicles	of Queue Distance	Prop. Qu eu ed	Effective Stop Rat e	Average Spe ed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	165	1.9	0.289	8.3	LOS A	0.0	0.0	0.00	0.90	49.0
2	Т	344	6.4	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	509	5.0	0.289	2.7	NA	0.0	0.0	0.00	0.29	55.9
North: C	Cowper S	Street (N)									
8	Т	706	1.3	0.544	7.1	LOS A	9.8	69.4	0.91	0.00	45.3
9	R	135	0.0	0.544	15.5	LOS C	9.8	69.4	0.91	1.13	45.1
Approad	ch	841	1.1	0.544	8.4	NA	9.8	69.4	0.91	0.18	45.2
West: N	lorphett	Street									
10	L	76	5.6	1.191	281.1	LOS F	24.5	185.3	1.00	3.21	6.9
12	R	84	12.5	1.191	281.7	LOS F	24.5	185.3	1.00	2.41	6.8
Approad	ch	160	9.2	1.191	281.4	LOS F	24.5	185.3	1.00	2.79	6.8
All Vehi	cles	1511	3.3	1.191	35.4	NA	24.5	185.3	0.61	0.49	29.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement
 LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).
 Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).
 NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street 2012 PM

Signals - Fixed Time Cycle Time = 130 seconds (User-Given Phase Times)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbou	urne Avenue	(S)								
1	L	498	2.1	0.648	17.4	LOS B	14.1	100.2	0.55	0.76	40.7
2	Т	1712	3.2	1.355	335.6	LOS F	130.7	940.0	0.98	2.21	5.8
3	R	173	3.7	0.817	74.2	LOS E	11.7	84.5	1.00	0.93	20.1
Approa	ch	2382	3.0	1.355	250.2	LOS F	130.7	940.0	0.89	1.81	7.6
East: A	ntill Stre	et									
4	L	227	2.3	0.364	15.3	LOS B	5.1	36.6	0.44	0.71	42.4
5	Т	688	1.4	1.179	147.6	LOS F	70.7	500.3	1.00	1.38	11.6
6	R	499	1.1	1.179	116.9	LOS F	70.7	500.3	1.00	1.17	14.6
Approa	ch	1415	1.4	1.179	115.5	LOS F	70.7	500.3	0.91	1.20	14.3
North: N	Northbou	irne Avenue (	N)								
7	L	219	1.0	0.232	15.3	LOS B	5.1	36.1	0.42	0.70	42.4
8	Т	685	4.6	0.435	40.1	LOS D	11.7	85.4	0.86	0.73	27.5
9	R	167	0.6	0.775	72.1	LOS E	11.1	77.8	1.00	0.89	20.4
Approa	ch	1072	3.2	0.775	40.0	LOS D	11.7	85.4	0.79	0.75	27.9
West: N	louat St	reet (W)									
10	L	247	1.7	1.146	179.6	LOS F	47.4	336.7	1.00	1.32	10.2
11	Т	460	1.8	1.146	199.4	LOS F	47.4	336.7	1.00	1.51	9.1
12	R	256	2.5	1.011	119.9	LOS F	23.5	168.3	1.00	1.25	14.3
Approa	ch	963	2.0	1.146	173.2	LOS F	47.4	336.7	1.00	1.39	10.4
All Vehi	icles	5832	2.5	1.355	166.2	LOS F	130.7	940.0	0.90	1.40	10.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue Service Road - Antill Street 2012 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North West: Northbourne Avenue Service Road											
27	L	16	0.0	0.030	12.7	LOS B	0.1	0.7	0.57	0.78	44.3
Approac	:h	16	0.0	0.030	12.7	LOS B	0.1	0.7	0.57	0.78	44.3
West: A	ntill Stre	et (W)									
10	L	1	0.0	0.231	9.1	LOS A	0.0	0.0	0.00	1.19	48.1
11	Т	842	2.1	0.231	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	:h	843	2.1	0.231	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehic	cles	859	2.1	0.231	0.2	NA	0.1	0.7	0.01	0.02	59.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street 2012 PM Giveway / Yield (Two-Way)

Mover	ment Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	224	0.9	0.718	29.8	LOS D	4.3	30.3	0.91	1.22	33.2
Approa	ich	224	0.9	0.718	29.8	LOS D	4.3	30.3	0.91	1.22	33.2
East: A	East: Antil Street (W)										
4	L	16	6.7	0.021	8.2	LOS A	0.1	0.4	0.18	0.56	48.8
5	Т	1206	1.6	0.329	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ich	1222	1.6	0.329	0.1	NA	0.1	0.4	0.00	0.01	59.8
West: A	Antill Stre	et									
11	Т	742	2.4	0.204	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	99	0.0	0.272	18.9	LOS C	0.9	6.2	0.80	0.96	39.5
Approa	ich	841	2.1	0.272	2.2	NA	0.9	6.2	0.09	0.11	56.5
All Veh	icles	2287	1.7	0.718	3.8	NA	4.3	30.3	0.13	0.16	54.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Badham Street 2012 PM Stop (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	388	0.5	0.939	45.2	LOS E	15.4	108.4	0.95	1.99	27.4
3	R	53	0.0	0.939	44.7	LOS E	15.4	108.4	0.95	1.85	27.4
Approac	ch	441	0.5	0.939	45.2	LOS E	15.4	108.4	0.95	1.98	27.4
East: Ar	ntill Stree	et (W)									
4	L	117	2.7	0.112	9.2	LOS A	0.4	3.0	0.35	0.64	47.9
5	Т	786	2.1	0.215	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	903	2.2	0.215	1.2	NA	0.4	3.0	0.05	0.08	58.1
West: A	ntill Stre	et (W)									
11	Т	447	2.8	0.246	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	335	1.3	0.499	14.9	LOS B	2.8	19.6	0.70	1.00	42.6
Approac	ch	782	2.2	0.499	6.4	NA	2.8	19.6	0.30	0.43	51.1
South W	Vest: Me	dian (RT Stag	ge 2)								
32	R	53	0.0	0.061	9.9	LOS A	0.2	1.4	0.45	0.71	32.1
Approac	ch	53	0.0	0.061	9.9	LOS A	0.2	1.4	0.45	0.71	32.1
All Vehic	cles	2179	1.8	0.939	12.2	NA	15.4	108.4	0.33	0.60	45.2

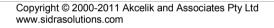
Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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INTERSECTION

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Antill Street - Dickson Shops Access Road 2012 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	uth: Dickson Shop Access Road 1 L 237 0.0 0.										
1	L	237	0.0	0.391	13.6	LOS B	2.1	14.4	0.64	0.92	43.8
Approa	ch	237	0.0	0.391	13.6	LOS B	2.1	14.4	0.64	0.92	43.8
East: A	ntill Stree	et (W)									
4	L	39	2.7	0.198	8.3	LOS A	0.0	0.0	0.00	1.02	49.0
5	Т	677	3.4	0.198	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	716	3.4	0.198	0.5	NA	0.0	0.0	0.00	0.06	59.3
All Vehi	cles	953	2.5	0.391	3.7	NA	2.1	14.4	0.16	0.27	54.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Signals - Fixed Time Cycle Time = 67 seconds (User-Given Phase Times)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	South: Cowper Street 1 L 3 R Approach East: Antill Street (E) 4 L 5 T Approach Vest: Antill Street (W 11 T 12 R										
1	L	400	4.2	0.742	33.1	LOS C	12.7	91.7	0.95	0.90	31.5
<mark>3</mark>	R	<mark>447</mark>	0.8	<mark>1.000</mark> 3	31.4	LOS C	13.9	97.9	0.98	0.86	32.3
Approac	ch	847	2.0	1.000	32.2	LOS C	13.9	97.9	0.97	0.88	31.9
East: Ar	ntill Stree	et (E)									
4	L	184	0.0	0.334	27.6	LOS C	4.7	33.0	0.81	0.79	34.1
5	Т	411	2.8	0.445	23.3	LOS C	5.8	41.7	0.89	0.73	34.7
Approac	ch	595	1.9	0.445	24.7	LOS C	5.8	41.7	0.86	0.75	34.5
West: A	ntill Stre	et (W)									
11	Т	360	2.3	0.389	22.9	LOS C	5.0	35.8	0.87	0.71	35.0
12	R	149	5.6	0.538	37.4	LOS D	4.7	34.8	0.96	0.80	29.7
Approac	h	509	3.3	0.538	27.2	LOS C	5.0	35.8	0.90	0.74	33.2
All Vehic	cles	1952	2.3	1.000	28.6	LOS C	13.9	97.9	0.92	0.80	33.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

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Antill Street - Rosevear Place 2012 PM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Rosevea	r Place (S)									
1	L	66	0.0	0.165	12.0	LOS B	0.6	4.4	0.52	0.77	45.1
3	R	34	0.0	0.165	11.6	LOS B	0.6	4.4	0.52	0.83	45.4
Approa	ch	100	0.0	0.165	11.9	LOS B	0.6	4.4	0.52	0.79	45.2
East: A	ntill Stree	et (E)									
4	L	22	0.0	0.116	8.2	LOS A	0.0	0.0	0.00	1.02	49.0
5	Т	400	1.8	0.116	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	422	1.7	0.116	0.4	NA	0.0	0.0	0.00	0.05	59.3
West: A	ntill Stre	et									
11	Т	611	1.2	0.207	0.9	LOS A	1.3	9.2	0.17	0.00	56.5
12	R	77	0.0	0.207	10.5	LOS B	1.3	9.2	0.45	0.88	48.0
Approa	ch	687	1.1	0.207	2.0	NA	1.3	9.2	0.20	0.10	55.4
South V	Vest: Me	dian (RT Sta	ge 2)								
32	R	34	0.0	0.090	15.4	LOS C	0.3	2.0	0.61	0.85	25.5
Approa	ch	34	0.0	0.090	15.4	LOS C	0.3	2.0	0.61	0.85	25.5
All Vehi	cles	1243	1.2	0.207	2.6	NA	1.3	9.2	0.17	0.16	55.1

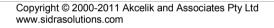
Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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INTERSECTION

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Badham Street - Dickson Shop Access Road 2012 PM Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	382	0.3	0.411	4.0	LOS A	4.4	31.0	0.69	0.00	47.6
3	R	186	0.0	0.411	12.5	LOS B	4.4	31.0	0.69	1.01	46.6
Approac	h	568	0.2	0.411	6.8	NA	4.4	31.0	0.69	0.33	47.3
East: Di	East: Dickson Shop Ac		Road (E)								
4	L	125	0.0	0.531	23.3	LOS C	3.0	21.1	0.68	1.04	36.5
6	R	63	0.0	0.531	23.5	LOS C	3.0	21.1	0.68	1.04	36.4
Approac	h	188	0.0	0.531	23.4	LOS C	3.0	21.1	0.68	1.04	36.5
North: B	adham S	Street (N)									
7	L	147	0.0	0.234	8.2	LOS A	0.0	0.0	0.00	0.89	49.0
8	Т	275	2.3	0.234	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	422	1.5	0.234	2.9	NA	0.0	0.0	0.00	0.31	55.6
All Vehic	cles	1179	0.6	0.531	8.0	NA	4.4	31.0	0.44	0.44	47.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road 2012 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	172	0.6	0.449	8.2	LOS A	0.0	0.0	0.00	0.96	49.0
2	Т	638	2.6	0.449	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	809	2.2	0.449	1.7	NA	0.0	0.0	0.00	0.20	57.3
North: C	Cowper S	Street (N)									
8	Т	247	2.6	0.296	8.1	LOS A	2.8	20.2	0.84	0.00	44.8
9	R	91	1.2	0.296	16.6	LOS C	2.8	20.2	0.84	1.04	43.5
Approac	ch	338	2.2	0.296	10.4	NA	2.8	20.2	0.84	0.28	44.5
West: D	ickson S	Shop Access	Road (W	')							
10	L	214	0.0	0.834	38.0	LOS E	8.1	56.7	0.90	1.51	29.3
12	R	74	1.4	0.834	38.6	LOS E	8.1	56.7	0.90	1.44	29.2
Approac	ch	287	0.4	0.834	38.2	LOS E	8.1	56.7	0.90	1.49	29.3
All Vehi	cles	1435	1.8	0.834	11.1	NA	8.1	56.7	0.38	0.48	45.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Woolley Street 2012 PM Giveway / Yield (Two-Way)

Movem	ient Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	58	0.0	0.243	8.2	LOS A	0.0	0.0	0.00	1.00	49.0
2	Т	389	0.0	0.243	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	447	0.0	0.243	1.1	NA	0.0	0.0	0.00	0.13	58.3
North: B	adham :	Street (N)									
8	Т	259	2.4	0.304	3.3	LOS A	2.4	17.4	0.61	0.00	48.7
9	R	143	0.0	0.304	11.8	LOS B	2.4	17.4	0.61	0.96	47.0
Approac	h	402	1.6	0.304	6.4	NA	2.4	17.4	0.61	0.34	48.0
West: W	oolley S	Street									
10	L	126	0.8	0.418	18.1	LOS C	2.1	14.6	0.66	0.98	40.0
12	R	52	0.0	0.418	18.3	LOS C	2.1	14.6	0.66	0.98	39.9
Approac	h	178	0.6	0.418	18.2	LOS C	2.1	14.6	0.66	0.98	40.0
All Vehic	cles	1027	0.7	0.418	6.1	NA	2.4	17.4	0.35	0.36	50.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Place 2012 PM Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	346	0.0	0.386	3.2	LOS A	3.7	25.8	0.62	0.00	48.4
3	R	194	0.0	0.386	11.6	LOS B	3.7	25.8	0.62	0.95	47.2
Approac	h	540	0.0	0.386	6.2	NA	3.7	25.8	0.62	0.34	48.0
East: Dickson Place		lace									
4	L	54	0.0	0.551	28.0	LOS D	3.0	20.7	0.75	1.01	33.7
6	R	101	0.0	0.551	28.3	LOS D	3.0	20.7	0.75	1.06	33.7
Approac	h	155	0.0	0.551	28.2	LOS D	3.0	20.7	0.75	1.05	33.7
North: B	adham \$	Street (N)									
7	L	166	3.8	0.207	8.3	LOS A	0.0	0.0	0.00	0.85	49.0
8	Т	204	0.0	0.207	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	:h	371	1.7	0.207	3.7	NA	0.0	0.0	0.00	0.38	54.5
All Vehic	cles	1065	0.6	0.551	8.5	NA	3.7	25.8	0.42	0.46	47.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Signals - Fixed Time Cycle Time = 92 seconds (User-Given Phase Times)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper \$	Street (S)									
1	L	135	0.0	0.931	53.6	LOS D	46.1	326.9	1.00	1.14	25.2
2	Т	675	1.7	0.931	45.4	LOS D	46.1	326.9	1.00	1.14	25.3
Approa	ch	809	1.4	0.931	46.7	LOS D	46.1	326.9	1.00	1.14	25.3
North: Cowper		Street (N)									
8	Т	308	2.7	0.279	9.0	LOS A	6.4	46.2	0.50	0.44	46.3
9	R	21	0.0	0.098	29.0	LOS C	0.5	3.5	0.91	0.71	33.3
Approa	ch	329	2.6	0.279	10.3	LOS B	6.4	46.2	0.53	0.45	45.1
West: D	Dickson F	Place (W)									
10	L	97	7.6	0.828	56.5	LOS E	10.7	77.3	1.00	0.94	23.5
12	R	120	0.0	0.828	56.3	LOS E	10.7	77.3	1.00	0.94	23.5
Approa	ch	217	3.4	0.828	56.4	LOS E	10.7	77.3	1.00	0.94	23.5
All Vehi	icles	1356	2.0	0.931	39.4	LOS D	46.1	326.9	0.89	0.94	28.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Challis Street - Cape Street 2012 PM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
2	Т	201	0.5	0.572	4.6	LOS A	6.4	45.1	0.72	0.00	45.8
3	R	449	0.0	0.572	13.1	LOS B	6.4	45.1	0.72	1.02	44.9
Approac	ch	651	0.2	0.572	10.5	NA	6.4	45.1	0.72	0.70	45.2
East: Ca	ape Stre	et									
4	L	220	0.0	0.643	23.4	LOS C	5.1	35.8	0.66	1.11	36.4
6	R	61	0.0	0.643	23.7	LOS C	5.1	35.8	0.66	1.09	36.4
Approac	h	281	0.0	0.643	23.5	LOS C	5.1	35.8	0.66	1.10	36.4
North: C	hallis St	treet									
7	L	157	0.7	0.207	8.2	LOS A	0.0	0.0	0.00	0.86	49.0
8	Т	217	0.5	0.207	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	374	0.6	0.207	3.4	NA	0.0	0.0	0.00	0.36	54.8
All Vehic	cles	1305	0.2	0.643	11.3	NA	6.4	45.1	0.50	0.69	45.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street 2012 PM Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	217	0.0	0.159	4.1	LOS A	1.4	9.9	0.63	0.00	49.1
6	R	29	0.0	0.159	12.6	LOS B	1.4	9.9	0.63	1.02	47.1
Approac	h	246	0.0	0.159	5.1	NA	1.4	9.9	0.63	0.12	48.8
North: W	Voolley S	Street									
7	L	40	0.0	0.311	20.7	LOS C	1.3	8.8	0.74	0.96	38.1
9	R	57	0.0	0.311	20.9	LOS C	1.3	8.8	0.74	0.97	38.1
Approac	h	97	0.0	0.311	20.8	LOS C	1.3	8.8	0.74	0.96	38.1
West: C	ape Stre	et (W)									
10	L	136	0.8	0.328	8.2	LOS A	0.0	0.0	0.00	0.95	49.0
11	Т	463	0.0	0.328	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	599	0.2	0.328	1.9	NA	0.0	0.0	0.00	0.21	57.1
All Vehic	cles	942	0.1	0.328	4.7	NA	1.4	9.9	0.24	0.27	52.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street 2012 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street									
2	Т	709	1.3	0.412	4.0	LOS A	6.1	43.4	0.74	0.00	47.9
3	R	24	0.0	0.412	12.4	LOS B	6.1	43.4	0.74	0.98	48.1
Approac	ch	734	1.3	0.412	4.2	NA	6.1	43.4	0.74	0.03	48.0
East: Da	avenport	Street (E)									
4	L	25	0.0	0.745	61.3	LOS F	4.1	28.7	0.90	1.24	22.2
6	R	87	0.0	0.745	61.5	LOS F	4.1	28.7	0.90	1.21	22.2
Approac	ch	113	0.0	0.745	61.5	LOS F	4.1	28.7	0.90	1.22	22.2
North: C	Cowper S	Street									
7	L	97	0.0	0.239	8.2	LOS A	0.0	0.0	0.00	0.95	49.0
8	Т	336	2.5	0.239	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	433	1.9	0.239	1.8	NA	0.0	0.0	0.00	0.21	57.1
All Vehi	cles	1279	1.4	0.745	8.5	NA	6.1	43.4	0.51	0.20	45.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbou	Irne Avenue	(S)								
2	Т	2455	2.8	0.450	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	423	0.0	0.916	39.8	LOS E	12.4	86.9	0.96	1.82	29.1
Approa	ch	2878	2.4	0.916	5.9	NA	12.4	86.9	0.14	0.27	51.7
East: M	orphett S	Street (E)									
4	L	358	0.6	1.006	75.8	LOS F	18.9	133.1	1.00	2.31	19.6
Approa	ch	358	0.6	1.006	75.8	LOS F	18.9	133.1	1.00	2.31	19.6
North: N	lorthbou	rne Avenue	(N)								
7	L	41	0.0	0.210	8.2	LOS A	0.0	0.0	0.00	1.02	49.0
8	Т	1095	3.8	0.210	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1136	3.7	0.210	0.3	NA	0.0	0.0	0.00	0.04	59.5
All Vehi	cles	4372	2.6	1.006	10.1	NA	18.9	133.1	0.17	0.37	47.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street 2012 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	57	3.7	0.339	3.2	LOS A	2.0	14.0	0.56	0.00	48.1
6	R	274	0.4	0.339	11.5	LOS B	2.0	14.0	0.56	0.88	45.9
Approac	:h	331	1.0	0.339	10.1	NA	2.0	14.0	0.56	0.73	46.2
North: C	hallis St	treet									
7	L	211	0.5	0.238	10.0	LOS B	1.0	6.8	0.42	0.72	47.0
9	R	303	0.0	0.930	54.0	LOS F	13.3	92.9	0.96	1.82	24.0
Approac	:h	514	0.2	0.930	35.9	LOS E	13.3	92.9	0.74	1.37	30.1
West: M	orphett	Street (W)									
10	L	326	0.0	0.248	8.2	LOS A	0.0	0.0	0.00	0.74	49.0
11	Т	116	0.0	0.248	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	:h	442	0.0	0.248	6.0	NA	0.0	0.0	0.00	0.55	51.4
All Vehic	cles	1286	0.3	0.930	19.0	NA	13.3	92.9	0.44	0.92	39.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street 2012 PM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Cowper S	Street (S)									
1	L	98	2.2	0.378	8.3	LOS A	0.0	0.0	0.00	1.00	49.0
2	Т	589	1.4	0.378	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	687	1.5	0.378	1.2	NA	0.0	0.0	0.00	0.14	58.1
North: C	Cowper S	Street (N)									
8	Т	267	3.9	0.213	5.3	LOS A	1.9	13.5	0.68	0.00	48.3
9	R	43	0.0	0.213	13.8	LOS B	1.9	13.5	0.68	1.03	46.0
Approac	ch	311	3.4	0.213	6.5	NA	1.9	13.5	0.68	0.14	48.0
West: M	lorphett	Street									
10	L	99	4.3	0.676	31.6	LOS D	4.3	30.9	0.86	1.23	32.0
12	R	100	1.1	0.676	31.8	LOS D	4.3	30.9	0.86	1.19	32.0
Approac	ch	199	2.6	0.676	31.7	LOS D	4.3	30.9	0.86	1.21	32.0
All Vehic	cles	1197	2.2	0.676	7.6	NA	4.3	30.9	0.32	0.32	48.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street 2012 MD

Signals - Fixed Time Cycle Time = 122 seconds (User-Given Phase Times)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbo	urne Avenue (	(S)								
1	L	216	2.0	0.261	15.0	LOS B	4.5	32.2	0.44	0.71	42.6
2	Т	832	4.2	0.695	41.7	LOS D	18.7	135.4	0.93	0.80	26.8
3	R	134	0.8	0.582	62.6	LOS E	7.7	54.4	0.99	0.80	22.4
Approa	ch	1181	3.4	0.695	39.2	LOS D	18.7	135.4	0.85	0.78	28.1
East: Ar	ntill Stre	et									
4	L	179	2.4	0.312	18.4	LOS B	4.6	32.6	0.51	0.72	40.0
5	Т	493	0.6	0.897	60.0	LOS E	20.3	143.2	0.99	0.95	21.9
6	R	275	1.1	0.872	70.5	LOS E	18.2	128.6	1.00	0.99	20.7
Approa	ch	946	1.1	0.897	55.2	LOS E	20.3	143.2	0.90	0.92	23.5
North: N	Vorthbou	Irne Avenue (	N)								
7	L	299	1.4	0.286	13.4	LOS B	6.0	42.2	0.41	0.71	43.9
8	Т	1095	1.9	0.717	43.1	LOS D	19.8	140.6	0.96	0.84	26.3
9	R	223	1.9	0.979	66.7	LOS E	13.8	97.9	1.00	0.83	21.4
Approa	ch	1617	1.8	0.979	40.9	LOS D	19.8	140.6	0.86	0.81	27.5
West: N	louat St	reet (W)									
10	L	212	1.0	0.805	58.0	LOS E	18.4	129.8	1.00	1.04	23.6
11	Т	407	0.5	0.805	53.4	LOS D	18.4	129.8	1.00	0.97	23.1
12	R	306	1.7	0.805	62.7	LOS E	17.7	124.8	1.00	0.93	22.5
Approa	ch	925	1.0	0.805	57.6	LOS E	18.4	129.8	1.00	0.97	23.0
All Vehi	cles	4669	1.9	0.979	46.7	LOS D	20.3	143.2	0.89	0.86	25.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

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INTERSECTION



Northbourne Avenue Service Road - Antill Street 2012 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North W	/est: Nor	thbourne Ave	nue Ser	vice Road							
27	L	25	0.0	0.047	12.7	LOS B	0.2	1.2	0.57	0.80	44.4
Approa	ch	25	0.0	0.047	12.7	LOS B	0.2	1.2	0.57	0.80	44.4
West: A	ntill Stre	et (W)									
10	L	1	0.0	0.226	9.1	LOS A	0.0	0.0	0.00	1.19	48.1
11	Т	829	0.8	0.226	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	831	0.8	0.226	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	856	0.7	0.226	0.4	NA	0.2	1.2	0.02	0.02	59.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street 2012 MD Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Challis S	treet (S)									
1	L	117	1.8	0.234	13.8	LOS B	0.9	6.4	0.65	0.87	43.7
Approa	ch	117	1.8	0.234	13.8	LOS B	0.9	6.4	0.65	0.87	43.7
East: A	ntil Stree	t (W)									
4	L	22	0.0	0.028	8.0	LOS A	0.1	0.5	0.18	0.56	48.8
5	Т	832	0.8	0.226	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	854	0.7	0.226	0.2	NA	0.1	0.5	0.00	0.01	59.6
West: A	ntill Stre	et									
11	Т	744	0.8	0.202	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	106	1.0	0.174	13.1	LOS B	0.6	4.0	0.62	0.87	44.2
Approa	ch	851	0.9	0.202	1.6	NA	0.6	4.0	0.08	0.11	57.4
All Vehi	cles	1821	0.9	0.234	1.7	NA	0.9	6.4	0.08	0.11	57.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street 2012 MD Stop (Two-Way)

Movem	ent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: B	adham	Street (S)									
1	L	241	0.4	0.536	19.1	LOS C	3.3	23.4	0.68	1.15	40.4
3	R	55	0.0	0.536	18.6	LOS C	3.3	23.4	0.68	1.12	40.6
Approac	h	296	0.4	0.536	19.0	LOS C	3.3	23.4	0.68	1.14	40.4
East: An	till Stree	et (W)									
4	L	153	0.7	0.158	9.6	LOS A	0.6	4.3	0.40	0.68	47.6
5	Т	559	0.9	0.152	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	712	0.9	0.158	2.1	NA	0.6	4.3	0.09	0.15	56.8
West: Ar	ntill Stre	et (W)									
11	Т	364	1.2	0.198	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	406	0.3	0.471	12.4	LOS B	2.8	20.0	0.56	0.90	44.7
Approac	h	771	0.7	0.471	6.6	NA	2.8	20.0	0.29	0.48	50.8
South W	'est: Me	dian (RT Stag	ge 2)								
32	R	55	0.0	0.058	9.4	LOS A	0.2	1.3	0.40	0.67	32.9
Approac	h	55	0.0	0.058	9.4	LOS A	0.2	1.3	0.40	0.67	32.9
All Vehic	les	1833	0.7	0.536	6.9	NA	3.3	23.4	0.28	0.46	50.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Dickson Shops Access Road 2012 MD Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	th: Dickson Shop Access Road L 202 0.5 C										
1	L	202	0.5	0.279	11.0	LOS B	1.2	8.6	0.54	0.79	46.2
Approa	ch	202	0.5	0.279	11.0	LOS B	1.2	8.6	0.54	0.79	46.2
East: A	ntill Stre	et (W)									
4	L	40	0.0	0.152	8.2	LOS A	0.0	0.0	0.00	0.99	49.0
5	Т	516	1.0	0.152	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	556	0.9	0.152	0.6	NA	0.0	0.0	0.00	0.07	59.0
All Veh	icles	758	0.8	0.279	3.4	NA	1.2	8.6	0.15	0.26	54.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street 2012 MD

Signals - Fixed Time Cycle Time = 59 seconds (User-Given Phase Times)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	147	2.1	0.358	29.3	LOS C	3.7	26.2	0.88	0.79	33.2
3	R	334	1.3	0.805	36.1	LOS D	10.6	74.7	1.00	0.97	30.2
Approa	ch	481	1.5	0.805	34.0	LOS C	10.6	74.7	0.96	0.91	31.1
East: A	ntill Stree	et (E)									
4	L	242	0.9	0.583	30.6	LOS C	6.5	45.5	0.94	0.82	32.5
5	Т	408	0.5	0.408	19.6	LOS B	5.0	35.0	0.87	0.71	37.0
Approa	ch	651	0.6	0.583	23.7	LOS C	6.5	45.5	0.89	0.75	35.2
West: A	ntill Stre	et (W)									
11	Т	265	0.8	0.266	18.7	LOS B	3.1	21.7	0.83	0.66	37.6
12	R	154	1.4	0.472	32.4	LOS C	4.2	29.5	0.94	0.79	31.8
Approa	ch	419	1.0	0.472	23.7	LOS C	4.2	29.5	0.87	0.71	35.2
All Vehi	cles	1551	1.0	0.805	26.9	LOS C	10.6	74.7	0.91	0.79	33.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Antill Street - Rosevear Place 2012 MD Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: F	Rosevea	r Place (S)									
1	L	22	0.0	0.042	11.6	LOS B	0.1	1.0	0.50	0.74	45.5
3	R	4	0.0	0.042	11.2	LOS B	0.1	1.0	0.50	0.77	45.8
Approac	ch	26	0.0	0.042	11.5	LOS B	0.1	1.0	0.50	0.74	45.5
East: Ar	ntill Stree	et (E)									
4	L	2	0.0	0.134	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	492	0.6	0.134	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	494	0.6	0.134	0.0	NA	0.0	0.0	0.00	0.00	59.9
West: A	ntill Stre	et									
11	Т	465	0.7	0.135	1.3	LOS A	1.0	6.9	0.22	0.00	55.7
12	R	15	0.0	0.135	10.7	LOS B	1.0	6.9	0.48	0.93	48.2
Approac	ch	480	0.7	0.135	1.5	NA	1.0	6.9	0.23	0.03	55.5
South W	/est: Me	dian (RT Sta	ge 2)								
32	R	4	0.0	0.009	12.4	LOS B	0.0	0.2	0.52	0.68	28.7
Approac	h	4	0.0	0.009	12.4	LOS B	0.0	0.2	0.52	0.68	28.7
All Vehic	cles	1004	0.6	0.135	1.1	NA	1.0	6.9	0.13	0.04	57.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

 $\label{eq:minor Road} \mbox{ Approach LOS values are based on average delay for all vehicle movements}.$ 

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road 2012 MD Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID Turn		Demand	HV Deg. Satn		Average	Level of	95% Back of Queue		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	255	0.0	0.276	3.7	LOS A	2.3	15.8	0.62	0.00	48.5
3	R	114	0.0	0.276	12.2	LOS B	2.3	15.8	0.62	0.98	46.8
Approach		368	0.0	0.276	6.4	NA	2.3	15.8	0.62	0.30	48.0
East: Dickson Shop Access Road (E)											
4	L	123	0.0	0.491	19.9	LOS C	2.7	19.0	0.68	1.03	38.7
6	R	77	0.0	0.491	20.2	LOS C	2.7	19.0	0.68	1.02	38.6
Approach		200	0.0	0.491	20.0	LOS C	2.7	19.0	0.68	1.02	38.6
North: Badham Street (N)											
7	L	201	0.0	0.277	8.2	LOS A	0.0	0.0	0.00	0.86	49.0
8	Т	300	1.1	0.277	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approach		501	0.6	0.277	3.3	NA	0.0	0.0	0.00	0.35	55.0
All Vehicles		1069	0.3	0.491	7.5	NA	2.7	19.0	0.34	0.46	48.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road 2012 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	169	0.6	0.274	8.2	LOS A	0.0	0.0	0.00	0.89	49.0
2	Т	323	2.3	0.274	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	493	1.7	0.274	2.8	NA	0.0	0.0	0.00	0.31	55.7
North: C	Cowper S	Street (N)									
8	Т	259	1.6	0.297	3.5	LOS A	2.2	15.7	0.54	0.00	49.6
9	R	137	0.0	0.297	12.0	LOS B	2.2	15.7	0.54	0.93	46.6
Approad	ch	396	1.1	0.297	6.5	NA	2.2	15.7	0.54	0.32	48.5
West: D	ickson S	Shop Access	Road (W	')							
10	L	155	0.0	0.509	18.0	LOS C	3.3	23.4	0.67	0.98	40.0
12	R	92	0.0	0.509	18.6	LOS C	3.3	23.4	0.67	1.03	39.9
Approad	ch	246	0.0	0.509	18.3	LOS C	3.3	23.4	0.67	1.00	39.9
All Vehi	cles	1135	1.1	0.509	7.4	NA	3.3	23.4	0.34	0.46	49.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Woolley Street 2012 MD Giveway / Yield (Two-Way)

Flow         Delay         Service         Vehicles         Distance         Question           veh/h         %         v/c         sec         veh         m         m           South: Badham Street (S)         1         L         53         0.0         0.150         8.2         LOS A         0.0         0.0           2         T         222         0.0         0.150         0.0         LOS A         0.0         0.0	Prop. Effective eued Stop Rate per vel	e Speed n km/h
veh/h         %         v/c         sec         veh         m           South: Badham Street (S)         1         L         53         0.0         0.150         8.2         LOS A         0.0         0.0           2         T         222         0.0         0.150         0.0         LOS A         0.0         0.0	0.00 0.97	n km/h
South: Badham Street (S)           1         L         53         0.0         0.150         8.2         LOS A         0.0         0.0           2         T         222         0.0         0.150         0.0         LOS A         0.0         0.0	0.00 0.97	
1         L         53         0.0         0.150         8.2         LOS A         0.0         0.0           2         T         222         0.0         0.150         0.0         LOS A         0.0         0.0		7 49.0
2 T 222 0.0 0.150 0.0 LOS A 0.0 0.0		49.0
	0.00 0.00	
	0.00 0.00	) 60.0
Approach 275 0.0 0.150 1.6 NA 0.0 0.0	0.00 0.19	9 57.5
North: Badham Street (N)		
8 T 284 0.7 0.280 1.6 LOS A 1.9 13.6	0.47 0.00	) 50.9
9 R 137 0.0 0.280 10.1 LOS B 1.9 13.6	0.47 0.87	7 48.4
Approach 421 0.5 0.280 4.4 NA 1.9 13.6	0.47 0.28	3 50.1
West: Woolley Street		
10 L 113 0.9 0.247 12.4 LOS B 1.0 6.7	0.48 0.7	44.7
12 R 36 0.0 0.247 12.6 LOS B 1.0 6.7	0.48 0.86	6 44.6
Approach 148 0.7 0.247 12.4 LOS B 1.0 6.7	0.48 0.75	5 44.7
All Vehicles 844 0.4 0.280 4.9 NA 1.9 13.6	0.32 0.33	3 51.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place 2012 MD Giveway / Yield (Two-Way)

Movem	ent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: B	adham	Street (S)									
2	Т	174	0.0	0.209	2.0	LOS A	1.3	9.2	0.48	0.00	50.4
3	R	115	0.9	0.209	10.5	LOS B	1.3	9.2	0.48	0.87	47.9
Approac	h	288	0.4	0.209	5.4	NA	1.3	9.2	0.48	0.35	49.4
East: Did	kson Pl	ace									
4	L	106	0.0	0.424	15.9	LOS C	2.4	16.5	0.60	0.85	41.6
6	R	108	0.0	0.424	16.2	LOS C	2.4	16.5	0.60	0.96	41.5
Approac	h	215	0.0	0.424	16.0	LOS C	2.4	16.5	0.60	0.91	41.6
North: Ba	adham S	Street (N)									
7	L	152	1.4	0.195	8.2	LOS A	0.0	0.0	0.00	0.85	49.0
8	Т	200	0.0	0.195	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	352	0.6	0.195	3.6	NA	0.0	0.0	0.00	0.37	54.7
All Vehic	les	855	0.4	0.424	7.3	NA	2.4	16.5	0.32	0.50	49.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place 2012 MD

Signals - Fixed Time Cycle Time = 73 seconds (User-Given Phase Times)

Moven	nent Pe	rformance	- Vehi	cles							
Mov ID		Demand Flow		Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	187	0.0	0.809	33.8	LOS C	20.8	146.1	0.95	0.97	32.3
2	Т	396	0.8	0.809	25.6	LOS C	20.8	146.1	0.95	0.93	32.7
Approa	ch	583	0.5	0.809	28.2	LOS C	20.8	146.1	0.95	0.94	32.6
North: C	Cowper S	Street (N)									
8	Т	285	1.8	0.278	8.9	LOS A	5.3	37.4	0.55	0.47	46.2
9	R	45	0.0	0.149	22.3	LOS C	0.8	5.5	0.85	0.74	37.1
Approa	ch	331	1.6	0.278	10.7	LOS B	5.3	37.4	0.59	0.51	44.7
West: D	ickson F	Place (W)									
10	L	95	4.4	0.859	49.8	LOS D	9.3	66.4	1.00	0.99	25.3
12	R	129	0.8	0.859	49.7	LOS D	9.3	66.4	1.00	0.99	25.3
Approa	ch	224	2.3	0.859	49.8	LOS D	9.3	66.4	1.00	0.99	25.3
All Vehi	cles	1138	1.2	0.859	27.4	LOS C	20.8	146.1	0.86	0.83	33.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Challis Street - Cape Street 2012 MD Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
2	Т	57	3.7	0.214	0.6	LOS A	1.1	7.5	0.27	0.00	53.5
3	R	233	0.0	0.214	9.1	LOS A	1.1	7.5	0.27	0.70	47.8
Approac	ch	289	0.7	0.214	7.4	NA	1.1	7.5	0.27	0.56	48.8
East: Ca	ape Stree	et									
4	L	232	0.5	0.338	10.0	LOS B	1.6	10.9	0.29	0.63	47.0
6	R	61	0.0	0.338	10.3	LOS B	1.6	10.9	0.29	0.80	46.8
Approac	ch	293	0.4	0.338	10.1	LOS B	1.6	10.9	0.29	0.66	47.0
North: C	Challis St	reet									
7	L	61	1.7	0.072	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
8	Т	68	0.0	0.072	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	129	0.8	0.072	3.9	NA	0.0	0.0	0.00	0.39	54.2
All Vehic	cles	712	0.6	0.338	7.9	NA	1.6	10.9	0.23	0.57	48.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Woolley Street 2012 MD Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stree	et (E)									
5	Т	229	0.5	0.163	1.5	LOS A	1.1	7.8	0.45	0.00	51.8
6	R	41	0.0	0.163	10.0	LOS A	1.1	7.8	0.45	0.93	48.9
Approac	:h	271	0.4	0.163	2.8	NA	1.1	7.8	0.45	0.14	51.3
North: W	/oolley S	Street									
7	L	47	0.0	0.215	13.3	LOS B	0.8	5.8	0.53	0.71	43.8
9	R	63	1.7	0.215	13.6	LOS B	0.8	5.8	0.53	0.87	43.7
Approac	:h	111	1.0	0.215	13.5	LOS B	0.8	5.8	0.53	0.80	43.7
West: Ca	ape Stre	et (W)									
10	L	93	1.1	0.163	8.2	LOS A	0.0	0.0	0.00	0.90	49.0
11	Т	203	0.0	0.163	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	:h	296	0.4	0.163	2.6	NA	0.0	0.0	0.00	0.28	56.0
All Vehic	cles	677	0.5	0.215	4.4	NA	1.1	7.8	0.26	0.31	51.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street 2012 MD Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper S	Street									
2	Т	501	1.1	0.291	2.8	LOS A	3.0	21.5	0.63	0.00	49.4
3	R	18	0.0	0.291	11.3	LOS B	3.0	21.5	0.63	0.94	48.8
Approa	ch	519	1.0	0.291	3.1	NA	3.0	21.5	0.63	0.03	49.4
East: D	avenport	Street (E)									
4	L	21	0.0	0.426	27.5	LOS D	1.9	13.4	0.78	0.92	33.9
6	R	83	0.0	0.426	27.8	LOS D	1.9	13.4	0.78	1.02	33.9
Approa	ch	104	0.0	0.426	27.8	LOS D	1.9	13.4	0.78	1.00	33.9
North: 0	Cowper S	Street									
7	L	91	0.0	0.232	8.2	LOS A	0.0	0.0	0.00	0.95	49.0
8	Т	332	1.6	0.232	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	422	1.2	0.232	1.8	NA	0.0	0.0	0.00	0.20	57.2
All Vehi	icles	1045	1.0	0.426	5.0	NA	3.0	21.5	0.39	0.20	49.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

Processed: Thursday, 7 June 2012 10:24:43 AM SIDRA INTERSECTION 5.1.12.2089



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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
2	Т	1177	2.6	0.216	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	293	0.7	1.127	172.0	LOS F	29.8	209.8	1.00	2.95	10.7
Approa	ch	1469	2.2	1.127	34.3	NA	29.8	209.8	0.20	0.59	31.0
East: R	oadNam	е									
4	L	222	0.9	1.192	234.1	LOS F	29.4	207.5	1.00	2.90	8.2
Approa	ch	222	0.9	1.192	234.1	LOS F	29.4	207.5	1.00	2.90	8.2
North: N	Northbou	rne Avenue	(N)								
7	L	25	4.2	0.289	8.3	LOS A	0.0	0.0	0.00	1.07	49.0
8	Т	1562	1.7	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1587	1.7	0.289	0.1	NA	0.0	0.0	0.00	0.02	59.8
All Vehi	icles	3279	1.9	1.192	31.3	NA	29.8	209.8	0.16	0.47	32.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Morphett Street - Challis Street 2012 MD Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	26	4.0	0.084	1.4	LOS A	0.4	2.7	0.39	0.00	51.3
6	R	73	0.0	0.084	9.7	LOS A	0.4	2.7	0.39	0.74	47.7
Approac	ch	99	1.1	0.084	7.5	NA	0.4	2.7	0.39	0.54	48.6
North: C	Challis St	reet									
7	L	94	0.0	0.097	9.3	LOS A	0.4	2.5	0.32	0.66	47.6
9	R	205	0.5	0.361	13.4	LOS B	1.9	13.7	0.56	0.86	43.8
Approac	ch	299	0.4	0.361	12.1	LOS B	1.9	13.7	0.49	0.80	44.9
West: N	lorphett	Street (W)									
10	L	218	1.0	0.176	8.2	LOS A	0.0	0.0	0.00	0.75	49.0
11	Т	95	1.1	0.176	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	313	1.0	0.176	5.7	NA	0.0	0.0	0.00	0.53	51.9
All Vehi	cles	711	0.7	0.361	8.7	NA	1.9	13.7	0.26	0.64	48.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street 2012 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street (S)									
1	L	40	0.0	0.274	8.2	LOS A	0.0	0.0	0.00	1.04	49.0
2	Т	462	0.9	0.274	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	502	0.8	0.274	0.7	NA	0.0	0.0	0.00	0.08	58.9
North: C	Cowper S	Street (N)									
8	Т	320	0.7	0.203	3.2	LOS A	1.7	11.7	0.52	0.00	50.8
9	R	25	4.2	0.203	11.8	LOS B	1.7	11.7	0.52	1.01	47.7
Approa	ch	345	0.9	0.203	3.8	NA	1.7	11.7	0.52	0.07	50.6
West: N	/lorphett	Street									
10	L	79	0.0	0.352	18.2	LOS C	1.6	11.5	0.67	0.93	39.9
12	R	61	0.0	0.352	18.4	LOS C	1.6	11.5	0.67	0.96	39.8
Approa	ch	140	0.0	0.352	18.3	LOS C	1.6	11.5	0.67	0.95	39.9
All Vehi	cles	987	0.7	0.352	4.3	NA	1.7	11.7	0.28	0.20	52.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street 2012 EV

Signals - Fixed Time Cycle Time = 121 seconds (User-Given Phase Times)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles		Prop. Queued	Effective Stop Rate	Average Speed
			0/			OCIVICC		Distance	Queucu		
0 11 1		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbol	irne Avenue (	. ,								
1	L	67	3.1	0.071	10.9	LOS B	0.9	6.3	0.29	0.66	46.3
2	Т	419	1.5	0.400	41.4	LOS D	8.8	62.2	0.88	0.72	27.0
3	R	104	1.0	0.424	59.7	LOS E	5.8	40.8	0.96	0.78	23.1
Approa	ch	591	1.6	0.424	41.2	LOS D	8.8	62.2	0.83	0.72	27.5
East: A	ntill Stree	et									
4	L	171	0.0	0.222	11.6	LOS B	2.6	18.2	0.34	0.68	45.5
5	Т	393	0.8	0.710	50.6	LOS D	13.7	96.2	0.97	0.82	24.1
6	R	219	1.4	0.691	59.0	LOS E	12.5	88.6	0.99	0.85	23.2
Approa	ch	782	0.8	0.710	44.4	LOS D	13.7	96.2	0.84	0.80	26.6
North: N	Vorthbou	rne Avenue (	N)								
7	L	207	0.0	0.184	11.1	LOS B	3.0	21.3	0.32	0.68	46.0
8	Т	598	1.4	0.454	42.5	LOS D	10.1	71.6	0.90	0.75	26.6
9	R	98	0.0	0.395	59.7	LOS E	5.4	37.8	0.95	0.78	23.0
Approa	ch	903	0.9	0.454	37.2	LOS D	10.1	71.6	0.77	0.74	28.9
West: N	louat Str	reet (W)									
10	L	40	0.0	0.512	57.7	LOS E	10.8	75.7	0.92	0.89	24.0
11	Т	386	0.5	0.512	47.1	LOS D	11.0	77.4	0.93	0.79	25.1
12	R	176	0.6	0.449	51.2	LOS D	9.1	63.7	0.91	0.80	25.4
Approa	ch	602	0.5	0.512	49.0	LOS D	11.0	77.4	0.92	0.80	25.1
All Vehi	cles	2878	1.0	0.710	42.4	LOS D	13.7	96.2	0.83	0.77	27.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

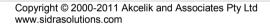
Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

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INTERSECTION



Northbourne Avenue Service Road - Antill Street 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North W	/est: Nor	thbourne Ave	nue Ser	vice Road							
27	L	16	0.0	0.026	11.6	LOS B	0.1	0.6	0.53	0.74	45.4
Approa	ch	16	0.0	0.026	11.6	LOS B	0.1	0.6	0.53	0.74	45.4
West: A	ntill Stre	et (W)									
10	L	2	0.0	0.193	9.1	LOS A	0.0	0.0	0.00	1.19	48.1
11	Т	712	0.4	0.193	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	714	0.4	0.193	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	729	0.4	0.193	0.3	NA	0.1	0.6	0.01	0.02	59.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Challis S	treet (S)									
1	L	136	0.8	0.220	11.8	LOS B	0.8	5.9	0.57	0.83	45.4
Approad	ch	136	0.8	0.220	11.8	LOS B	0.8	5.9	0.57	0.83	45.4
East: Antil Street (W)		t (W)									
4	L	26	0.0	0.034	8.3	LOS A	0.1	0.6	0.24	0.57	48.5
5	Т	647	0.8	0.176	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	674	0.8	0.176	0.3	NA	0.1	0.6	0.01	0.02	59.4
West: A	ntill Stre	et									
11	Т	546	0.6	0.148	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	169	0.0	0.222	11.6	LOS B	0.8	5.5	0.53	0.82	45.5
Approad	ch	716	0.4	0.222	2.8	NA	0.8	5.5	0.13	0.19	55.8
All Vehi	cles	1525	0.6	0.222	2.5	NA	0.8	5.9	0.11	0.17	56.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street 2012 EV Stop (Two-Way)

Movem	ent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Ba	adham	Street (S)									
1	L	247	0.4	0.495	16.9	LOS C	3.1	21.9	0.62	1.09	41.9
3	R	69	0.0	0.495	16.5	LOS C	3.1	21.9	0.62	1.09	42.2
Approach	h	317	0.3	0.495	16.8	LOS C	3.1	21.9	0.62	1.09	42.0
East: Ant	till Stree	et (W)									
4	L	152	0.7	0.151	9.4	LOS A	0.6	4.1	0.39	0.66	47.7
5	Т	415	0.8	0.113	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approach	h	566	0.7	0.151	2.5	NA	0.6	4.1	0.10	0.18	56.1
West: An	ntill Stre	et (W)									
11	Т	209	0.0	0.113	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	374	0.8	0.375	10.6	LOS B	1.9	13.4	0.45	0.76	46.5
Approach	h	583	0.5	0.375	6.8	NA	1.9	13.4	0.29	0.49	50.6
South W	est: Me	dian (RT Stag	ge 2)								
32	R	69	0.0	0.063	8.6	LOS A	0.2	1.5	0.30	0.63	33.6
Approach	h	69	0.0	0.063	8.6	LOS A	0.2	1.5	0.30	0.63	33.6
All Vehic	les	1536	0.5	0.495	7.4	NA	3.1	21.9	0.29	0.50	49.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Dickson Shops Access Road 2012 EV Giveway / Yield (Two-Way)

Moven	Movement Performance - Vehicles Mov ID Turn Demand HV Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average														
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed				
		veh/h	%	v/c	sec		veh	m		per veh	km/h				
South: [	Dickson	Shop Access	Road												
1	L	157	0.0	0.195	9.9	LOS A	0.8	5.6	0.47	0.71	47.3				
Approa	ch	157	0.0	0.195	9.9	LOS A	0.8	5.6	0.47	0.71	47.3				
East: Ar	ntill Stree	et (W)													
4	L	42	0.0	0.124	8.2	LOS A	0.0	0.0	0.00	0.97	49.0				
5	Т	412	1.0	0.124	0.0	LOS A	0.0	0.0	0.00	0.00	60.0				
Approa	ch	454	0.9	0.124	0.8	NA	0.0	0.0	0.00	0.09	58.8				
All Vehi	cles	611	0.7	0.195	3.1	NA	0.8	5.6	0.12	0.25	55.3				

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street 2012 EV

Signals - Fixed Time Cycle Time = 51 seconds (User-Given Phase Times)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	97	3.3	0.239	26.1	LOS C	2.0	14.7	0.85	0.77	35.0
3	R	166	1.3	0.405	26.6	LOS C	3.7	26.0	0.89	0.79	34.8
Approa	ch	263	2.0	0.405	26.4	LOS C	3.7	26.0	0.87	0.78	34.8
East: Antill Street		et (E)									
4	L	89	1.2	0.218	25.9	LOS C	1.9	13.2	0.84	0.76	35.0
5	Т	348	0.3	0.344	16.6	LOS B	3.6	25.3	0.84	0.69	39.0
Approa	ch	438	0.5	0.344	18.5	LOS B	3.6	25.3	0.84	0.70	38.1
West: A	ntill Stre	et (W)									
11	Т	216	0.0	0.212	15.9	LOS B	2.1	15.0	0.81	0.64	39.6
12	R	55	0.0	0.226	30.6	LOS C	1.3	9.1	0.93	0.74	32.6
Approa	ch	271	0.0	0.226	18.8	LOS B	2.1	15.0	0.83	0.66	38.0
All Vehi	cles	972	0.8	0.405	20.7	LOS C	3.7	26.0	0.85	0.71	37.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Antill Street - Rosevear Place 2012 EV Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: F	Rosevea	r Place (S)									
1	L	40	0.0	0.048	9.8	LOS A	0.2	1.2	0.38	0.67	47.2
3	R	1	0.0	0.048	9.4	LOS A	0.2	1.2	0.38	0.69	47.6
Approac	h	41	0.0	0.048	9.8	LOS A	0.2	1.2	0.38	0.68	47.3
East: Ar	ntill Stree	et (E)									
4	L	2	0.0	0.083	8.2	LOS A	0.0	0.0	0.00	1.08	49.0
5	Т	304	0.3	0.083	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	306	0.3	0.083	0.1	NA	0.0	0.0	0.00	0.01	59.9
West: A	ntill Stre	et									
11	Т	280	0.4	0.076	0.7	LOS A	0.5	3.3	0.18	0.00	56.6
12	R	1	0.0	0.076	9.4	LOS A	0.5	3.3	0.36	0.91	49.2
Approac	h	281	0.4	0.076	0.7	NA	0.5	3.3	0.18	0.00	56.5
South W	/est: Me	dian (RT Sta	ge 2)								
32	R	1	0.0	0.002	10.1	LOS B	0.0	0.0	0.41	0.59	31.9
Approac	h	1	0.0	0.002	10.1	LOS B	0.0	0.0	0.41	0.59	31.9
All Vehic	cles	629	0.3	0.083	1.0	NA	0.5	3.3	0.11	0.05	57.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	246	0.9	0.325	4.4	LOS A	2.8	19.9	0.66	0.00	47.8
3	R	152	0.0	0.325	12.9	LOS B	2.8	19.9	0.66	1.02	45.9
Approad	Approach		0.5	0.325	7.7	NA	2.8	19.9	0.66	0.39	47.1
East: Dickson Shop Access Road (E		Road (E)									
4	L	126	0.0	0.666	28.4	LOS D	4.4	30.5	0.80	1.22	33.6
6	R	93	0.0	0.666	28.6	LOS D	4.4	30.5	0.80	1.16	33.5
Approad	ch	219	0.0	0.666	28.5	LOS D	4.4	30.5	0.80	1.20	33.6
North: E	Badham	Street (N)									
7	L	129	0.0	0.289	8.2	LOS A	0.0	0.0	0.00	0.94	49.0
8	Т	397	0.8	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	526	0.6	0.289	2.0	NA	0.0	0.0	0.00	0.23	56.9
All Vehi	cles	1143	0.5	0.666	9.0	NA	4.4	30.5	0.38	0.47	47.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road 2012 EV Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Cowper S	Street (S)									
1	L	126	0.0	0.168	8.2	LOS A	0.0	0.0	0.00	0.85	49.0
2	Т	175	3.0	0.168	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	301	1.7	0.168	3.4	NA	0.0	0.0	0.00	0.36	54.8
North: Cowper Street (N)		Street (N)									
8	Т	105	1.0	0.094	1.4	LOS A	0.5	3.6	0.30	0.00	53.9
9	R	40	2.6	0.094	9.9	LOS A	0.5	3.6	0.30	0.89	48.2
Approac	ch	145	1.4	0.094	3.7	NA	0.5	3.6	0.30	0.25	52.2
West: D	ickson S	Shop Access I	Road (W	)							
10	L	89	0.0	0.190	10.2	LOS B	0.8	5.8	0.42	0.63	46.7
12	R	54	0.0	0.190	10.8	LOS B	0.8	5.8	0.42	0.80	46.4
Approac	ch	143	0.0	0.190	10.4	LOS B	0.8	5.8	0.42	0.69	46.6
All Vehic	cles	589	1.3	0.190	5.2	NA	0.8	5.8	0.18	0.41	52.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Woolley Street 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	239	0.0	0.252	8.2	LOS A	0.0	0.0	0.00	0.81	49.0
2	Т	215	0.0	0.252	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	454	0.0	0.252	4.3	NA	0.0	0.0	0.00	0.43	53.6
North: Badham Street (N)		Street (N)									
8	Т	285	1.5	0.319	3.5	LOS A	2.7	19.4	0.62	0.00	48.4
9	R	143	0.0	0.319	12.0	LOS B	2.7	19.4	0.62	0.98	46.9
Approad	ch	428	1.0	0.319	6.4	NA	2.7	19.4	0.62	0.33	47.9
West: W	Voolley S	Street									
10	L	160	1.3	0.690	25.7	LOS D	5.5	38.7	0.73	1.21	35.1
12	R	118	0.0	0.690	25.9	LOS D	5.5	38.7	0.73	1.17	35.0
Approad	ch	278	0.8	0.690	25.8	LOS D	5.5	38.7	0.73	1.19	35.0
All Vehi	cles	1160	0.5	0.690	10.2	NA	5.5	38.7	0.40	0.57	45.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	253	0.0	0.280	3.0	LOS A	2.1	14.8	0.59	0.00	49.0
3	R	128	0.0	0.280	11.5	LOS B	2.1	14.8	0.59	0.94	47.3
Approad	Approach 3		0.0	0.280	5.9	NA	2.1	14.8	0.59	0.32	48.4
East: Dickson Place		lace									
4	L	121	0.0	0.617	23.4	LOS C	4.2	29.3	0.72	1.12	36.4
6	R	121	0.0	0.617	23.7	LOS C	4.2	29.3	0.72	1.10	36.3
Approad	ch	242	0.0	0.617	23.5	LOS C	4.2	29.3	0.72	1.11	36.3
North: E	Badham 3	Street (N)									
7	L	185	1.7	0.242	8.2	LOS A	0.0	0.0	0.00	0.85	49.0
8	Т	249	0.4	0.242	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	435	1.0	0.242	3.5	NA	0.0	0.0	0.00	0.36	54.7
All Vehi	cles	1058	0.4	0.617	8.9	NA	4.2	29.3	0.38	0.52	47.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place 2012 EV

Signals - Fixed Time Cycle Time = 65 seconds (User-Given Phase Times)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID		Demand Flow		eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper \$	Street (S)									
1	L	205	0.0	0.669	28.0	LOS C	11.8	83.1	0.90	0.87	34.9
2	Т	221	1.0	0.669	19.8	LOS B	11.8	83.1	0.90	0.79	35.5
Approach		426	0.5	0.669	23.8	LOS C	11.8	83.1	0.90	0.83	35.2
North: Cowper Street (N		Street (N)									
8	Т	142	0.7	0.143	8.1	LOS A	2.2	15.8	0.53	0.43	47.1
9	R	19	0.0	0.055	19.6	LOS B	0.3	2.1	0.79	0.70	38.9
Approa	ch	161	0.7	0.143	9.4	LOS A	2.2	15.8	0.56	0.46	46.0
West: D	Dickson F	Place (W)									
10	L	66	4.8	0.551	36.5	LOS D	5.0	35.4	0.96	0.80	29.9
12	R	96	0.0	0.551	36.4	LOS D	5.0	35.4	0.96	0.80	29.9
Approa	ch	162	1.9	0.551	36.4	LOS D	5.0	35.4	0.96	0.80	29.9
All Vehi	icles	749	0.8	0.669	23.4	LOS C	11.8	83.1	0.84	0.75	35.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Challis Street - Cape Street 2012 EV Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
2	Т	84	0.0	0.279	1.2	LOS A	1.5	10.5	0.39	0.00	51.3
3	R	273	0.4	0.279	9.6	LOS A	1.5	10.5	0.39	0.74	47.6
Approa	Approach 3		0.3	0.279	7.6	NA	1.5	10.5	0.39	0.56	48.4
East: Cape Street		et									
4	L	266	0.0	0.498	13.3	LOS B	3.7	25.7	0.46	0.74	43.9
6	R	92	0.0	0.498	13.5	LOS B	3.7	25.7	0.46	0.91	43.8
Approa	ch	358	0.0	0.498	13.3	LOS B	3.7	25.7	0.46	0.78	43.9
North: 0	Challis St	reet									
7	L	105	0.0	0.118	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
8	Т	108	0.0	0.118	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	214	0.0	0.118	4.0	NA	0.0	0.0	0.00	0.41	54.0
All Vehi	icles	928	0.1	0.498	9.0	NA	3.7	25.7	0.32	0.61	47.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Woolley Street 2012 EV Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	238	0.0	0.226	2.2	LOS A	1.6	11.0	0.51	0.00	50.3
6	R	96	1.1	0.226	10.7	LOS B	1.6	11.0	0.51	0.91	48.1
Approac	Approach 334		0.3	0.226	4.6	NA	1.6	11.0	0.51	0.26	49.6
North: Woolley Street		Street									
7	L	149	0.0	0.536	17.9	LOS C	3.6	25.2	0.64	0.95	40.1
9	R	119	0.0	0.536	18.2	LOS C	3.6	25.2	0.64	1.02	40.0
Approac	ch	268	0.0	0.536	18.0	LOS C	3.6	25.2	0.64	0.98	40.1
West: C	ape Stre	et (W)									
10	L	172	0.6	0.202	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
11	Т	193	0.0	0.202	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	364	0.3	0.202	3.9	NA	0.0	0.0	0.00	0.39	54.2
All Vehic	cles	966	0.2	0.536	8.0	NA	3.6	25.2	0.35	0.51	48.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	cles							
Mov ID	Turn	Demand Flow	HV C	0eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street									
2	Т	368	0.6	0.220	1.3	LOS A	1.8	12.9	0.43	0.00	52.2
3	R	23	0.0	0.220	9.7	LOS A	1.8	12.9	0.43	0.93	49.1
Approad	ch	392	0.5	0.220	1.8	NA	1.8	12.9	0.43	0.05	52.0
East: Da	avenport	Street (E)									
4	L	18	0.0	0.182	15.1	LOS C	0.7	4.8	0.55	0.64	42.2
6	R	58	0.0	0.182	15.4	LOS C	0.7	4.8	0.55	0.87	42.1
Approad	ch	76	0.0	0.182	15.3	LOS C	0.7	4.8	0.55	0.81	42.1
North: C	Cowper S	Street									
7	L	58	0.0	0.135	8.2	LOS A	0.0	0.0	0.00	0.94	49.0
8	Т	188	0.6	0.135	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	246	0.4	0.135	1.9	NA	0.0	0.0	0.00	0.22	57.0
All Vehi	cles	714	0.4	0.220	3.3	NA	1.8	12.9	0.30	0.19	52.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 EV Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	)eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
2	Т	722	1.5	0.131	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	315	0.3	0.553	16.8	LOS C	3.5	24.4	0.77	1.06	41.4
Approa	ch	1037	1.1	0.553	5.1	NA	3.5	24.4	0.23	0.32	52.7
East: M	lorphett S	Street (E)									
4	L	191	0.0	0.432	17.3	LOS C	2.1	14.5	0.75	0.99	40.8
Approa	ch	191	0.0	0.432	17.3	LOS C	2.1	14.5	0.75	0.99	40.8
North: N	Northbou	rne Avenue	(N)								
7	L	18	0.0	0.173	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
8	Т	938	1.0	0.173	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	956	1.0	0.173	0.2	NA	0.0	0.0	0.00	0.02	59.7
All Vehi	icles	2183	1.0	0.553	4.0	NA	3.5	24.4	0.18	0.25	54.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Morphett Street - Challis Street 2012 EV Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	18	0.0	0.097	1.5	LOS A	0.4	2.9	0.40	0.00	50.9
6	R	91	0.0	0.097	9.8	LOS A	0.4	2.9	0.40	0.73	47.5
Approac	ch	108	0.0	0.097	8.4	NA	0.4	2.9	0.40	0.61	48.0
North: C	Challis St	reet									
7	L	117	0.0	0.119	9.2	LOS A	0.4	3.1	0.31	0.66	47.6
9	R	185	0.0	0.325	13.0	LOS B	1.6	11.3	0.55	0.84	44.1
Approac	ch	302	0.0	0.325	11.5	LOS B	1.6	11.3	0.46	0.77	45.4
West: M	lorphett	Street (W)									
10	L	258	0.4	0.182	8.2	LOS A	0.0	0.0	0.00	0.72	49.0
11	Т	64	0.0	0.182	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	322	0.3	0.182	6.6	NA	0.0	0.0	0.00	0.58	50.8
All Vehic	cles	733	0.1	0.325	8.9	NA	1.6	11.3	0.25	0.66	48.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street 2012 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street (S)									
1	L	39	0.0	0.198	8.2	LOS A	0.0	0.0	0.00	1.02	49.0
2	Т	324	0.3	0.198	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	363	0.3	0.198	0.9	NA	0.0	0.0	0.00	0.11	58.6
North: C	Cowper S	Street (N)									
8	Т	191	0.6	0.121	1.8	LOS A	0.8	5.7	0.41	0.00	52.6
9	R	18	0.0	0.121	10.3	LOS B	0.8	5.7	0.41	0.97	48.6
Approa	ch	208	0.5	0.121	2.5	NA	0.8	5.7	0.41	0.08	52.2
West: N	/lorphett	Street									
10	L	68	1.5	0.209	12.6	LOS B	0.8	6.0	0.53	0.73	44.5
12	R	53	0.0	0.209	12.8	LOS B	0.8	6.0	0.53	0.87	44.4
Approa	ch	121	0.9	0.209	12.7	LOS B	0.8	6.0	0.53	0.79	44.5
All Vehi	cles	693	0.5	0.209	3.4	NA	0.8	6.0	0.22	0.22	53.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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# APPENDIX B 2031 DO NOTHING SIDRA RESULTS

#### **MOVEMENT SUMMARY**

Site: 1 AM

1- Northbourne Avenue - Antill Street - Mouat Street Do Nothing 2031 AM Signals - Fixed Time Cycle Time = 110 seconds (Optimum Cycle Time - Minimum Delay)

Movem	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Vorthbou	urne Avenue	(S)								
1	L	143	17.6	0.163	11.7	LOS B	1.9	15.4	0.34	0.68	45.9
2	Т	622	20.9	0.603	38.0	LOS D	12.6	102.0	0.91	0.77	27.8
3	R	132	7.2	1.235	288.7	LOS F	18.4	137.1	1.00	1.57	6.8
Approac	ch	897	16.0	1.235	70.6	LOS E	18.4	137.1	0.83	0.87	20.0
East: Ar	ntill Stree	et									
<mark>4</mark>	L	<mark>721</mark>	3.7	<mark>1.000</mark> 3	39.2	LOS D	29.9	216.2	1.00	0.99	29.1
5	Т	641	5.9	1.570	336.3	LOS F	105.2	769.3	1.00	2.03	5.8
6	R	486	3.9	1.570	258.9	LOS F	105.2	769.3	1.00	1.66	7.6
Approac	ch	1848	4.4	1.570	200.0	LOS F	105.2	769.3	1.00	1.53	9.3
North: N	lorthbou	Irne Avenue	(N)								
7	L	225	13.1	0.262	16.0	LOS B	5.2	40.2	0.49	0.72	42.0
8	Т	1753	7.4	1.256	296.1	LOS F	86.9	647.1	1.00	2.27	6.5
9	R	125	1.2	1.127	195.3	LOS F	13.8	97.9	1.00	1.37	9.6
Approac	ch	2103	7.5	1.256	260.1	LOS F	86.9	647.1	0.95	2.05	7.3
West: M	louat Sti	reet (W)									
10	L	42	0.0	1.163	195.4	LOS F	54.2	390.4	1.00	1.47	9.6
11	Т	777	3.8	1.163	199.1	LOS F	60.4	438.4	1.00	1.64	9.1
12	R	571	5.2	1.163	111.9	LOS F	60.4	438.4	1.00	1.23	15.1
Approac	ch	1389	4.2	1.163	163.2	LOS F	60.4	438.4	1.00	1.47	10.9
All Vehic	cles	6238	7.1	1.570	193.4	LOS F	105.2	769.3	0.96	1.60	9.5

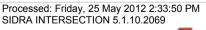
Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	ent Performance -	Pedestriar	າຣ					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bao Pedestrian		Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	19.8	LOS B	0.1	0.1	0.60	0.60
P2	Across S approach	72	42.8	LOS E	0.2	0.2	0.88	0.88
P3	Across E approach	107	41.0	LOS E	0.3	0.3	0.86	0.86
P5	Across N approach	47	19.8	LOS B	0.1	0.1	0.60	0.60
P6	Across N approach	47	37.6	LOS D	0.1	0.1	0.83	0.83
P7	Across W approach	16	41.0	LOS E	0.0	0.0	0.86	0.86
All Pede	strians	361	33.9	LOS D			0.78	0.78

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.





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Northbourne Avenue Service Road - Antill Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North West: Northbourne Avenue Service Road											
27	L	451	0.0	1.580	555.6	LOS F	111.8	782.7	1.00	6.07	3.7
Approac	ch	451	0.0	1.580	555.6	LOS F	111.8	782.7	1.00	6.07	3.7
West: A	ntill Stre	et (W)									
10	L	1	0.0	0.364	9.1	LOS A	0.0	0.0	0.00	1.19	48.1
11	Т	1295	6.2	0.364	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	1296	6.2	0.364	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehic	cles	1746	4.6	1.580	143.3	NA	111.8	782.7	0.26	1.57	12.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Project: X:\PROJECT\3002303 Dickson Precinct Traffic and Parking Study\015 Traffic - Intersection Flow Modelling\Dickson Intersections\_WD\_AM\_DoNothing2031.sip 8000072, SMEC AUSTRALIA PTY. LTD., FLOATING

Site: 2 AM

Antill Street - Challis Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
1	L	516	8.6	2.512	1396.8	LOS F	203.5	1529.3	1.00	7.48	1.5
Approac	:h	516	8.6	2.512	1396.8	LOS F	203.5	1529.3	1.00	7.48	1.5
East: Ar	ntill Stree	et (W)									
4	L	219	7.7	0.299	9.7	LOS A	1.0	7.2	0.38	0.68	47.7
5	Т	1333	2.8	0.367	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	:h	1552	3.5	0.367	1.4	NA	1.0	7.2	0.05	0.10	57.9
West: A	ntill Stre	et									
11	Т	1483	1.8	0.416	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	262	9.9	1.005	86.4	LOS F	13.0	98.6	1.00	2.00	17.8
Approac	h	1745	4.6	1.005	13.0	NA	13.0	98.6	0.15	0.30	44.3
All Vehic	cles	3813	4.7	2.512	195.5	NA	203.5	1529.3	0.23	1.19	9.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Badham Street Do Nothing 2031 AM Stop (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles		Prop. Queued	Effective Stop Rate	Average Speed
			%	v/c		0011100		Distance	Quodoa		
		veh/h	70	V/C	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	438	0.0	2.987	1827.6	LOS F	260.2	1843.6	1.00	8.40	1.2
3	R	162	5.2	2.987	1827.5	LOS F	260.2	1843.6	1.00	7.33	1.2
Approac	ch	600	1.4	2.987	1827.6	LOS F	260.2	1843.6	1.00	8.11	1.2
East: Ar	ntill Stree	et (W)									
4	L	181	3.5	0.190	9.8	LOS A	0.7	5.3	0.42	0.69	47.5
5	Т	1158	4.9	0.323	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	1339	4.7	0.323	1.3	NA	0.7	5.3	0.06	0.09	57.9
West: A	ntill Stre	et (W)									
11	Т	695	0.0	0.376	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	568	3.0	1.373	363.8	LOS F	104.9	752.9	1.00	6.17	5.5
Approac	ch	1263	1.3	1.373	163.7	NA	104.9	752.9	0.45	2.78	10.9
South W	Vest: Me	dian (RT Sta	ge 2)								
32	R	162	5.2	0.264	13.1	LOS B	1.1	6.9	0.61	0.89	28.2
Approac	ch	162	5.2	0.264	13.1	LOS B	1.1	6.9	0.61	0.89	28.2
All Vehi	cles	3364	2.9	2.987	388.6	NA	260.2	1843.6	0.40	2.57	5.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Do Nothing 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Dickson	Shop Access	Road								
1	L	177	0.0	0.540	23.7	LOS C	2.7	18.6	0.85	1.07	36.5
Approa	ch	177	0.0	0.540	23.7	LOS C	2.7	18.6	0.85	1.07	36.5
East: A	ntill Stree	et (W)									
4	L	1	0.0	0.324	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	1158	5.5	0.324	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1159	5.4	0.324	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	1336	4.7	0.540	3.1	NA	2.7	18.6	0.11	0.14	55.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Do Nothing 2031 AM Signals - Fixed Time Cycle Time = 65 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	375	0.0	0.415	18.4	LOS B	7.3	52.3	0.65	0.80	39.9
<mark>3</mark>	R	<mark>391</mark>	5.4	<mark>1.000</mark> 3	38.0	LOS D	13.4	97.9	1.00	0.88	29.5
Approac	ch	766	4.1	1.000	28.4	LOS C	13.4	97.9	0.83	0.84	33.8
East: Ar	ntill Stree	et (E)									
4	L	389	0.0	0.958	60.8	LOS E	18.4	128.9	1.00	1.17	22.4
5	Т	977	6.5	0.941	46.5	LOS D	22.2	163.8	1.00	1.25	25.3
Approac	ch	1366	4.6	0.958	50.6	LOS D	22.2	163.8	1.00	1.23	24.4
West: A	ntill Stre	et (W)									
11	Т	623	1.4	0.581	21.7	LOS C	8.7	61.4	0.91	0.77	35.6
12	R	236	0.0	0.669	36.0	LOS D	7.4	51.9	0.98	0.86	30.2
Approac	ch	859	1.0	0.669	25.6	LOS C	8.7	61.4	0.92	0.79	33.9
All Vehi	cles	2992	3.4	1.000	37.7	LOS D	22.2	163.8	0.93	1.00	28.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movem	ent Performance -	Pedestriar	าร					
Maria	Description	Demand	Average				Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	26.8	LOS C	0.1	0.1	0.91	0.91
P3	Across E approach	107	26.8	LOS C	0.2	0.2	0.91	0.91
P4	Across E approach	33	24.1	LOS C	0.1	0.1	0.86	0.86
All Pede	estrians	212	26.4	LOS C			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Rosevear Place (S)											
1	L	95	0.0	0.514	35.2	LOS E	2.0	13.8	0.92	1.06	30.4
3	R	1	0.0	0.514	34.8	LOS D	2.0	13.8	0.92	1.06	30.5
Approach 9		96	0.0	0.514	35.2	LOS E	2.0	13.8	0.92	1.06	30.4
East: Antill Street (E)											
4	L	57	0.0	0.368	8.2	LOS A	0.0	0.0	0.00	1.03	49.0
5	Т	1272	3.6	0.368	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approach		1328	3.5	0.368	0.4	NA	0.0	0.0	0.00	0.04	59.4
West: Antill Street											
11	Т	851	4.2	0.376	4.5	LOS A	3.4	24.7	0.20	0.00	52.1
12	R	72	0.0	0.376	30.1	LOS D	3.4	24.7	1.00	1.06	34.3
Approach	า	922	3.9	0.376	6.5	NA	3.4	24.7	0.27	0.08	50.1
South West: Median (RT Stage 2)											
32	R	1	0.0	0.004	20.3	LOS C	0.0	0.1	0.73	0.77	21.6
Approach	ı	1	0.0	0.004	20.3	LOS C	0.0	0.1	0.73	0.77	21.6
All Vehicl	les	2347	3.5	0.514	4.2	NA	3.4	24.7	0.14	0.10	53.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID Turn		Demand	HV Deg. Satn		Average	Level of	95% Back of Queue		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Badham Street (S)											
2	Т	331	0.0	0.392	8.6	LOS A	4.8	33.4	0.95	0.00	43.9
3	R	124	0.0	0.392	17.0	LOS C	4.8	33.4	0.95	1.10	43.3
Approad	ch	455	0.0	0.392	10.9	NA	4.8	33.4	0.95	0.30	43.7
East: Dickson Shop Access Road (E)											
4	L	185	0.0	2.454	1356.6	LOS F	158.6	1110.1	1.00	6.94	1.6
6	R	217	0.0	2.454	1356.9	LOS F	158.6	1110.1	1.00	5.79	1.6
Approach 40		402	0.0	2.454	1356.8	LOS F	158.6	1110.1	1.00	6.32	1.6
North: Badham Street (N)											
7	L	131	0.0	0.400	8.2	LOS A	0.0	0.0	0.00	0.97	49.0
8	Т	589	3.6	0.400	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approach		720	2.9	0.400	1.5	NA	0.0	0.0	0.00	0.18	57.6
All Vehicles		1577	1.3	2.454	349.8	NA	158.6	1110.1	0.53	1.78	5.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Do Nothing 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	114	0.0	0.343	8.2	LOS A	0.0	0.0	0.00	0.97	49.0
2	Т	497	5.9	0.343	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	611	4.8	0.343	1.5	NA	0.0	0.0	0.00	0.18	57.6
North: C	lorth: Cowper Street (N)										
8	Т	493	0.0	0.441	6.8	LOS A	6.0	41.7	0.85	0.00	45.9
9	R	133	0.0	0.441	15.3	LOS C	6.0	41.7	0.85	1.09	44.9
Approad	ch	625	0.0	0.441	8.6	NA	6.0	41.7	0.85	0.23	45.6
West: D	ickson S	Shop Access	Road (W	)							
10	L	263	0.0	0.413	13.7	LOS B	2.4	16.5	0.64	0.94	43.6
12	R	1	0.0	0.413	14.3	LOS B	2.4	16.5	0.64	0.97	43.5
Approad	ch	264	0.0	0.413	13.7	LOS B	2.4	16.5	0.64	0.94	43.6
All Vehi	cles	1500	2.0	0.441	6.6	NA	6.0	41.7	0.46	0.34	49.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	21	0.0	0.187	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
2	Т	324	0.0	0.187	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	345	0.0	0.187	0.5	NA	0.0	0.0	0.00	0.06	59.2
North: E	North: Badham Street (N)										
8	Т	718	2.9	0.467	3.6	LOS A	6.6	47.1	0.71	0.00	48.1
9	R	72	0.0	0.467	12.1	LOS B	6.6	47.1	0.71	1.02	48.0
Approad	ch	789	2.7	0.467	4.4	NA	6.6	47.1	0.71	0.09	48.1
West: W	Voolley S	Street									
10	L	149	0.0	0.371	16.5	LOS C	1.7	12.0	0.60	0.87	41.2
12	R	19	0.0	0.371	16.8	LOS C	1.7	12.0	0.60	0.93	41.1
Approad	ch	168	0.0	0.371	16.5	LOS C	1.7	12.0	0.60	0.88	41.2
All Vehi	cles	1303	1.6	0.467	4.9	NA	6.6	47.1	0.51	0.19	49.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	122	0.0	0.693	25.0	LOS C	5.5	43.1	1.00	0.00	32.1
3	R	179	23.5	0.693	34.4	LOS D	5.5	43.1	1.00	1.22	32.0
Approac	ch	301	14.0	0.693	30.6	NA	5.5	43.1	1.00	0.73	32.0
East: Di	East: Dickson Place										
4	L	124	30.5	1.326	350.1	LOS F	44.4	353.2	1.00	3.75	5.6
6	R	116	0.0	1.326	349.2	LOS F	44.4	353.2	1.00	3.52	5.6
Approac	ch	240	15.8	1.326	349.6	LOS F	44.4	353.2	1.00	3.64	5.6
North: B	adham	Street (N)									
7	L	328	5.1	0.488	8.4	LOS A	0.0	0.0	0.00	0.88	49.0
8	Т	543	0.8	0.488	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	872	2.4	0.488	3.2	NA	0.0	0.0	0.00	0.33	55.3
All Vehic	cles	1413	7.2	1.326	67.9	NA	44.4	353.2	0.38	0.98	20.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Do Nothing 2031 AM Signals - Fixed Time Cycle Time = 130 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	223	0.0	0.885	53.6	LOS D	45.5	335.6	0.99	0.99	25.0
2	Т	467	9.5	0.885	45.4	LOS D	45.5	335.6	0.99	0.98	25.1
Approa	ch	691	6.4	0.885	48.0	LOS D	45.5	335.6	0.99	0.98	25.1
North: C	Cowper S	Street (N)									
8	Т	507	1.7	0.515	20.3	LOS C	20.2	143.5	0.69	0.62	36.9
9	R	84	45.0	0.687	44.6	LOS D	3.1	30.0	1.00	0.84	27.4
Approa	ch	592	7.8	0.687	23.8	LOS C	20.2	143.5	0.73	0.65	35.2
West: D	Dickson F	Place (W)									
10	L	208	20.2	0.905	72.9	LOS E	31.8	241.2	1.00	0.98	20.0
12	R	227	0.0	0.905	72.3	LOS E	31.8	241.2	1.00	0.98	20.0
Approa	ch	436	9.7	0.905	72.6	LOS E	31.8	241.2	1.00	0.98	20.0
All Vehi	cles	1718	7.7	0.905	45.9	LOS D	45.5	335.6	0.90	0.87	26.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Moverr	nent Performance -	Pedestria	าร					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bac Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	19.9	LOS B	0.1	0.1	0.55	0.55
P2	Across S approach	72	59.1	LOS E	0.3	0.3	0.95	0.95
P7	Across W approach	16	8.9	LOS A	0.0	0.0	0.37	0.37
P8	Across W approach	38	59.1	LOS E	0.1	0.1	0.95	0.95
All Pede	estrians	198	40.8	LOS E			0.76	0.76

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
2	Т	789	2.1	0.808	31.6	LOS D	17.4	123.9	1.00	0.00	30.0
3	R	200	0.0	0.808	40.1	LOS E	17.4	123.9	1.00	1.55	29.9
Approac	ch	989	1.7	0.808	33.3	NA	17.4	123.9	1.00	0.31	30.0
East: Ca	East: Cape Street										
4	L	560	0.0	2.362	1255.3	LOS F	235.7	1730.5	1.00	10.17	1.7
6	R	69	51.5	2.362	1257.7	LOS F	235.7	1730.5	1.00	7.32	1.7
Approac	ch	629	5.7	2.362	1255.6	LOS F	235.7	1730.5	1.00	9.86	1.7
North: C	hallis St	treet									
7	L	112	37.7	0.427	9.6	LOS A	0.0	0.0	0.00	1.05	49.0
8	Т	619	5.4	0.427	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	731	10.4	0.427	1.5	NA	0.0	0.0	0.00	0.16	58.0
All Vehic	cles	2349	5.5	2.362	350.9	NA	235.7	1730.5	0.69	2.82	5.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV C	)eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: C	ape Stre	et (E)									
5	Т	566	6.7	0.320	2.3	LOS A	2.9	21.2	0.59	0.00	50.2
6	R	1	0.0	0.320	10.7	LOS B	2.9	21.2	0.59	0.99	49.2
Approa	ch	567	6.7	0.320	2.3	NA	2.9	21.2	0.59	0.00	50.2
North: V	lorth: Woolley Street										
7	L	1	0.0	0.324	30.1	LOS D	1.2	8.6	0.84	0.86	32.6
9	R	61	0.0	0.324	30.4	LOS D	1.2	8.6	0.84	1.00	32.6
Approa	ch	62	0.0	0.324	30.4	LOS D	1.2	8.6	0.84	1.00	32.6
West: C	Cape Stre	eet (W)									
10	L	109	0.0	0.187	8.2	LOS A	0.0	0.0	0.00	0.89	49.0
11	Т	204	20.6	0.187	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	314	13.4	0.187	2.9	NA	0.0	0.0	0.00	0.31	55.6
All Vehi	cles	943	8.5	0.324	4.3	NA	2.9	21.2	0.41	0.17	50.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street									
2	Т	619	7.1	0.523	11.5	LOS B	10.5	77.7	1.00	0.00	41.8
3	R	112	5.7	0.523	20.2	LOS C	10.5	77.7	1.00	1.21	41.6
Approad	ch	731	6.9	0.523	12.8	NA	10.5	77.7	1.00	0.18	41.7
East: Davenport Street (E)		Street (E)									
4	L	13	0.0	1.199	324.0	LOS F	14.9	104.0	1.00	2.28	6.0
6	R	76	0.0	1.199	324.3	LOS F	14.9	104.0	1.00	1.90	6.0
Approad	ch	88	0.0	1.199	324.2	LOS F	14.9	104.0	1.00	1.96	6.0
North: C	Cowper S	Street									
7	L	122	0.0	0.402	8.2	LOS A	0.0	0.0	0.00	0.98	49.0
8	Т	611	1.4	0.402	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	733	1.1	0.402	1.4	NA	0.0	0.0	0.00	0.16	57.8
All Vehi	cles	1552	3.8	1.199	25.2	NA	14.9	104.0	0.53	0.28	34.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	lorphett	Street (E)									
4	L	579	5.8	1.346	340.2	LOS F	104.5	767.8	1.00	5.92	5.9
Approa	ch	579	5.8	1.346	340.2	LOS F	104.5	767.8	1.00	5.92	5.9
North: I	Northbou	Irne Avenue (	N)								
7	L	118	5.4	0.493	8.4	LOS A	0.0	0.0	0.00	1.01	49.0
8	Т	742	8.5	0.493	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	860	8.1	0.493	1.1	NA	0.0	0.0	0.00	0.14	58.2
All Veh	icles	1439	7.2	1.346	137.6	NA	104.5	767.8	0.40	2.46	12.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Movem	Movement Performance - Vehicles Mov ID Turn Demand HV Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average			
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: Northbourne Avenue (S)														
2	Т	1015	13.4	0.198	0.0	LOS A	0.0	0.0	0.00	0.00	60.0			
3	R	392	3.0	1.096	134.4	LOS F	31.1	223.4	1.00	3.20	13.0			
Approac	h	1407	9.3	1.096	37.5	NA	31.1	223.4	0.28	0.89	29.7			
North: N	lorthbou	rne Avenue	(N)											
7	L	118	5.4	0.469	8.4	LOS A	0.0	0.0	0.00	1.00	49.0			
8	Т	2389	5.4	0.469	0.0	LOS A	0.0	0.0	0.00	0.00	60.0			
Approac	h	2507	5.4	0.469	0.4	NA	0.0	0.0	0.00	0.05	59.4			
All Vehic	cles	3915	6.8	1.096	13.7	NA	31.1	223.4	0.10	0.35	43.6			

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	76	0.0	0.776	12.4	LOS B	8.3	58.0	0.99	0.00	38.8
6	R	459	0.0	0.776	20.7	LOS C	8.3	58.0	0.99	1.38	38.5
Approac	h	535	0.0	0.776	19.6	NA	8.3	58.0	0.99	1.18	38.6
North: C	orth: Challis Street										
7	L	1175	0.0	1.624	579.4	LOS F	297.8	2120.2	1.00	10.05	3.5
9	R	153	6.7	1.074	166.2	LOS F	15.1	111.8	1.00	2.04	10.7
Approac	h	1328	2.5	1.624	531.7	LOS F	297.8	2120.2	1.00	9.12	3.8
West: M	lorphett	Street (W)									
10	L	503	3.3	0.390	8.3	LOS A	0.0	0.0	0.00	0.74	49.0
11	Т	177	3.6	0.390	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	680	3.4	0.390	6.1	NA	0.0	0.0	0.00	0.55	51.4
All Vehic	cles	2543	2.2	1.624	283.5	NA	297.8	2120.2	0.73	5.16	6.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Do Nothing 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	301	0.0	0.495	8.2	LOS A	0.0	0.0	0.00	0.89	49.0
2	Т	566	8.9	0.495	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	867	5.8	0.495	2.8	NA	0.0	0.0	0.00	0.31	55.7
North: C	lorth: Cowper Street (N)										
8	Т	467	1.8	0.511	14.8	LOS B	7.0	49.5	1.00	0.00	39.1
9	R	124	0.0	0.511	23.2	LOS C	7.0	49.5	1.00	1.15	38.9
Approad	ch	592	1.4	0.511	16.6	NA	7.0	49.5	1.00	0.24	39.0
West: N	1orphett	Street									
10	L	179	0.0	3.655	2444.6	LOS F	227.0	1619.5	1.00	7.05	0.9
12	R	293	3.6	3.655	2445.0	LOS F	227.0	1619.5	1.00	5.79	0.9
Approad	ch	472	2.2	3.655	2444.8	LOS F	227.0	1619.5	1.00	6.27	0.9
All Vehi	cles	1931	3.6	3.655	603.6	NA	227.0	1619.5	0.55	1.74	3.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street Do Nothing 2031 PM Signals - Fixed Time Cycle Time = 130 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Po	rformance	- Vehic								
Mov ID		Demand		les leg. Satn	Average	Level of	95% Back	of Outouto	Dron	Effective	Average
	' Turri	Flow		ey. Salli	Delay	Service	Vehicles	Distance	Prop. Queued	Stop Rate	Average Speed
		veh/h	%	v/c	sec		ven	m		per veh	km/h
South <sup>.</sup>	Northbou	Irne Avenue		10	000		VGII				NIT // T
1		474	2.2	0.606	16.8	LOS B	12.7	90.9	0.53	0.75	41.2
2	T	1862	4.8	1.349	329.1	LOS F	141.6	1031.9	0.98	2.18	5.9
3	R	132	1.1	1.223	289.5	LOS F	19.4	137.1	1.00	1.48	6.9
Approa		2467	4.0	1.349	267.0	LOS F	141.6	1031.9	0.89	1.87	7.2
	ntill Stree										
4	L	371	1.1	0.631	17.6	LOS B	10.4	73.2	0.54	0.75	40.6
5	Т	836	4.3	1.316	213.0	LOS F	105.4	766.7	1.00	1.64	8.6
6	R	518	5.3	1.316	161.7	LOS F	105.4	766.7	0.99	1.33	11.3
Approa	ich	1724	3.9	1.316	155.6	LOS F	105.4	766.7	0.90	1.36	11.4
North: I	Northbou	rne Avenue	(N)								
7	L	246	6.8	0.247	12.2	LOS B	4.4	32.4	0.34	0.68	45.2
8	Т	855	7.0	0.499	38.1	LOS D	14.5	107.9	0.86	0.74	28.1
9	R	120	0.0	1.107	190.2	LOS F	13.8	96.8	1.00	1.29	9.8
Approa	ich	1221	5.9	1.107	47.8	LOS D	14.5	107.9	0.77	0.78	25.3
West: N	Mouat Str	eet (W)									
10	L	259	0.0	1.323	335.0	LOS F	70.0	501.4	1.00	1.61	5.9
11	Т	476	6.6	1.323	353.2	LOS F	70.0	501.4	1.00	1.85	5.5
12	R	276	3.1	1.323	163.9	LOS F	55.9	412.5	1.00	1.39	11.2
Approa	ich	1011	4.0	1.323	296.9	LOS F	70.0	501.4	1.00	1.66	6.6
All Veh	icles	6423	4.3	1.349	200.1	LOS F	141.6	1031.9	0.89	1.49	9.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestrian	S					
		Demand	Average	Level of	Average Bac	k of Queue	Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	24.6	LOS C	0.2 0.2		0.62	0.62
P2	Across S approach	72	40.8	LOS E	0.2	0.2	0.79	0.79
P3	Across E approach	107	40.0	LOS E	0.3	0.3	0.78	0.78
P5	Across N approach	47	24.6	LOS C	0.1	0.1	0.62	0.62
P6	Across N approach	47	54.5	LOS E	0.2	0.2	0.92	0.92
P7	Across W approach	16	40.0	LOS E	0.0	0.0	0.78	0.78
All Pede	estrians	361	37.0	LOS D			0.75	0.75

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: X:\PROJECT\3002303 Dickson Precinct Traffic and Parking Study\015 Traffic - Intersection Flow Modelling\Dickson Intersections\_WD\_PM\_DoNothing2031.sip 8000072, SMEC AUSTRALIA PTY. LTD., FLOATING

Site: 1 PM

#### Northbourne Avenue Service Road - Antill Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North W	Vest: Nor	thbourne Ave	enue Serv	/ice Road							
27	L	27	0.0	0.055	13.4	LOS B	0.2	1.4	0.60	0.83	43.7
Approa	ch	27	0.0	0.055	13.4	LOS B	0.2	1.4	0.60	0.83	43.7
West: A	Antill Stre	et (W)									
10	L	40	10.5	0.256	9.5	LOS A	0.0	0.0	0.00	1.13	48.1
11	Т	874	5.3	0.256	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	914	5.5	0.256	0.4	NA	0.0	0.0	0.00	0.05	59.3
All Vehi	icles	941	5.4	0.256	0.8	NA	0.2	1.4	0.02	0.07	58.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Challis Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
1	L	272	23.3	2.117	1059.5	LOS F	97.3	816.6	1.00	4.98	2.0
Approac	ch	272	23.3	2.117	1059.5	LOS F	97.3	816.6	1.00	4.98	2.0
East: Ar	East: Antill Street (W)										
4	L	32	53.3	0.070	10.9	LOS B	0.2	1.7	0.31	0.61	47.8
5	Т	1472	0.4	0.399	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	1503	1.5	0.399	0.2	NA	0.2	1.7	0.01	0.01	59.7
West: A	ntill Stre	et									
11	Т	752	0.6	0.206	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	149	22.7	1.057	151.6	LOS F	11.9	99.2	1.00	1.92	11.7
Approac	ch	901	5.1	1.057	25.1	NA	11.9	99.2	0.17	0.32	35.6
All Vehi	cles	2676	5.0	2.117	116.1	NA	97.3	816.6	0.16	0.62	14.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Badham Street Do Nothing 2031 PM Stop (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	568	0.0	1.728	684.2	LOS F	182.4	1291.2	1.00	8.37	3.0
3	R	86	9.8	1.728	684.4	LOS F	182.4	1291.2	1.00	7.20	3.0
Approa	ch	655	1.3	1.728	684.2	LOS F	182.4	1291.2	1.00	8.22	3.0
East: Ar	ntill Stree	et (W)									
4	L	67	0.0	0.070	9.5	LOS A	0.3	1.8	0.38	0.65	47.7
5	Т	931	2.3	0.255	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	998	2.1	0.255	0.6	NA	0.3	1.8	0.03	0.04	59.0
West: A	ntill Stre	et (W)									
11	Т	434	0.5	0.235	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	408	0.0	0.725	20.6	LOS C	5.3	37.3	0.85	1.22	38.3
Approa	ch	842	0.3	0.725	10.0	NA	5.3	37.3	0.41	0.59	47.1
South V	Vest: Me	dian (RT Stag	ge 2)								
32	R	86	9.8	0.106	10.5	LOS B	0.4	2.7	0.46	0.73	31.8
Approa	ch	86	9.8	0.106	10.5	LOS B	0.4	2.7	0.46	0.73	31.8
All Vehi	cles	2581	1.5	1.728	177.4	NA	182.4	1291.2	0.41	2.32	10.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	South: Dickson Shop Access Road										
1	L	229	0.0	0.415	14.8	LOS B	2.2	15.3	0.67	0.95	42.9
Approa	ch	229	0.0	0.415	14.8	LOS B	2.2	15.3	0.67	0.95	42.9
East: A	ntill Stree	et (W)									
4	L	1	0.0	0.213	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	773	2.7	0.213	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	774	2.7	0.213	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	1003	2.1	0.415	3.4	NA	2.2	15.3	0.15	0.22	54.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Do Nothing 2031 PM

Signals - Fixed Time Cycle Time = 70 seconds (Optimum Cycle Time - Minimum Delay)

Movem	<b>Novement Performance - Vehicles</b> Nov ID Turn Demand HV Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average													
Mov ID	Turn	Demand	HV D	eg. Satn	Average		95% Back	of Queue	Prop.		Average			
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed			
		veh/h	%	v/c	sec		veh	m		per veh	km/h			
South: C	Cowper	Street (S)												
1	L	323	0.9	0.461	25.1	LOS C	8.2	58.0	0.78	0.82	35.5			
<mark>3</mark>	R	<mark>481</mark>	1.1	<mark>1.000</mark> ³	28.9	LOS C	13.9	97.9	0.98	0.87	33.5			
Approac	ch	804	1.0	1.000	27.4	LOS C	13.9	97.9	0.90	0.85	34.3			
East: Ar	ntill Stree	et (E)												
4	L	232	0.9	0.395	24.0	LOS C	5.5	39.0	0.73	0.79	36.1			
5	Т	526	3.6	0.926	47.9	LOS D	11.7	84.8	1.00	1.15	24.8			
Approac	ch	758	2.8	0.926	40.6	LOS D	11.7	84.8	0.92	1.04	27.5			
West: A	ntill Stre	et (W)												
11	Т	385	2.7	0.674	32.5	LOS C	6.7	48.0	0.99	0.85	30.2			
12	R	139	0.0	0.425	36.3	LOS D	4.4	30.6	0.93	0.79	30.0			
Approac	ch	524	2.0	0.674	33.5	LOS C	6.7	48.0	0.98	0.84	30.1			
All Vehic	cles	2086	1.9	1.000	33.7	LOS C	13.9	97.9	0.93	0.91	30.5			

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movem	nent Performance -	Pedestrian	IS					
Mov	Description	Demand	Average	Level of	Average Bag		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	20.1	LOS C	0.1	0.1	0.76	0.76
P3	Across E approach	107	29.3	LOS C	0.2	0.2	0.91	0.91
P4	Across E approach	13	26.6	LOS C	0.0	0.0	0.87	0.87
All Pede	estrians	192	25.6	LOS C			0.85	0.85

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: F	Rosevea	r Place (S)									
1	L	84	0.0	0.256	14.4	LOS B	1.0	7.3	0.61	0.89	42.9
3	R	42	0.0	0.256	14.0	LOS B	1.0	7.3	0.61	0.88	43.2
Approac	ch	126	0.0	0.256	14.3	LOS B	1.0	7.3	0.61	0.89	43.0
East: Ar	ntill Stree	et (E)									
4	L	17	0.0	0.154	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
5	Т	539	3.5	0.154	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	556	3.4	0.154	0.2	NA	0.0	0.0	0.00	0.03	59.6
West: A	ntill Stre	et									
11	Т	589	2.1	0.216	1.2	LOS A	1.4	9.8	0.18	0.00	56.2
12	R	88	0.0	0.216	11.6	LOS B	1.4	9.8	0.56	0.91	47.0
Approac	ch	678	1.9	0.216	2.5	NA	1.4	9.8	0.23	0.12	54.8
South W	Vest: Me	dian (RT Sta	ge 2)								
32	R	42	0.0	0.109	15.1	LOS C	0.4	2.4	0.60	0.85	25.8
Approac	ch	42	0.0	0.109	15.1	LOS C	0.4	2.4	0.60	0.85	25.8
All Vehi	cles	1402	2.3	0.256	3.1	NA	1.4	9.8	0.19	0.18	54.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
2	Т	318	0.0	0.445	5.0	LOS A	4.8	33.4	0.73	0.00	46.6
3	R	232	0.0	0.445	13.4	LOS B	4.8	33.4	0.73	1.06	45.5
Approa	ch	549	0.0	0.445	8.5	NA	4.8	33.4	0.73	0.45	46.1
East: D	ickson S	hop Access I	Road (E)								
4	L	76	0.0	0.739	42.8	LOS E	4.8	33.5	0.86	1.34	27.4
6	R	95	0.0	0.739	43.1	LOS E	4.8	33.5	0.86	1.23	27.4
Approa	ch	171	0.0	0.739	43.0	LOS E	4.8	33.5	0.86	1.28	27.4
North: E	Badham 3	Street (N)									
7	L	120	0.0	0.262	8.2	LOS A	0.0	0.0	0.00	0.93	49.0
8	Т	358	0.0	0.262	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	478	0.0	0.262	2.1	NA	0.0	0.0	0.00	0.23	56.8
All Vehi	icles	1198	0.0	0.739	10.8	NA	4.8	33.5	0.46	0.48	45.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1			0.0	0.384	8.2	LOS A	0.0	0.0	0.00	1.00	49.0
2	Т	606	1.0	0.384	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	701	0.9	0.384	1.1	NA	0.0	0.0	0.00	0.14	58.2
North: C	Cowper S	Street (N)									
8	Т	322	0.7	0.250	5.8	LOS A	2.5	17.3	0.72	0.00	47.9
9	R	51	0.0	0.250	14.2	LOS B	2.5	17.3	0.72	1.04	45.7
Approad	ch	373	0.6	0.250	6.9	NA	2.5	17.3	0.72	0.14	47.6
West: D	ickson S	Shop Access	Road (W	)							
10	L	200	1.1	0.594	22.0	LOS C	3.8	27.1	0.78	1.12	37.4
12	R	46	0.0	0.594	22.6	LOS C	3.8	27.1	0.78	1.11	37.3
Approad	ch	246	0.9	0.594	22.1	LOS C	3.8	27.1	0.78	1.12	37.4
All Vehi	cles	1320	0.8	0.594	6.7	NA	3.8	27.1	0.35	0.32	49.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	147	0.0	0.171	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
2	Т	162	0.0	0.171	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	309	0.0	0.171	3.9	NA	0.0	0.0	0.00	0.40	54.2
North: B	Badham 3	Street (N)									
8	Т	272	0.0	0.366	2.3	LOS A	2.9	20.3	0.54	0.00	49.4
9	R	229	0.0	0.366	10.8	LOS B	2.9	20.3	0.54	0.87	47.6
Approac	ch	501	0.0	0.366	6.2	NA	2.9	20.3	0.54	0.40	48.5
West: W	Voolley S	street									
10	L	389	0.0	0.678	17.6	LOS C	7.4	51.8	0.61	1.00	40.4
12	R	65	0.0	0.678	17.8	LOS C	7.4	51.8	0.61	1.06	40.3
Approad	ch	455	0.0	0.678	17.6	LOS C	7.4	51.8	0.61	1.01	40.4
All Vehi	cles	1265	0.0	0.678	9.7	NA	7.4	51.8	0.43	0.62	46.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	8	0.0	0.209	3.7	LOS A	0.9	7.9	0.54	0.00	48.1
3	R	135	31.3	0.209	13.4	LOS B	0.9	7.9	0.54	0.83	45.0
Approad	ch	143	29.4	0.209	12.8	NA	0.9	7.9	0.54	0.78	45.2
East: Di	ckson P	lace									
4	L	53	68.0	0.752	28.1	LOS D	7.5	57.6	0.81	1.31	35.0
6	R	267	0.0	0.752	25.9	LOS D	7.5	57.6	0.81	1.30	35.0
Approac	ch	320	11.2	0.752	26.2	LOS D	7.5	57.6	0.81	1.30	35.0
North: B	Badham	Street (N)									
7	L	196	0.0	0.223	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
8	Т	206	0.0	0.223	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	402	0.0	0.223	4.0	NA	0.0	0.0	0.00	0.40	54.1
All Vehi	cles	865	9.0	0.752	13.7	NA	7.5	57.6	0.39	0.80	43.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Do Nothing 2031 PM Signals - Fixed Time Cycle Time = 130 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	- Vehi	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	299	0.0	0.870	41.5	LOS D	49.9	354.4	0.94	0.96	28.9
2	Т	543	2.7	0.870	33.3	LOS C	49.9	354.4	0.94	0.91	29.2
Approad	ch	842	1.8	0.870	36.2	LOS D	49.9	354.4	0.94	0.93	29.1
North: C	Cowper S	Street (N)									
8	Т	324	3.2	0.284	11.4	LOS B	9.0	65.1	0.48	0.42	44.1
9	R	84	42.5	0.655	42.3	LOS D	3.4	32.6	0.98	0.84	28.2
Approad	ch	408	11.3	0.655	17.7	LOS B	9.0	65.1	0.59	0.51	39.6
West: D	ickson F	Place (W)									
10	L	126	33.3	0.886	77.6	LOS E	20.3	160.1	1.00	0.96	19.3
12	R	156	0.0	0.886	76.4	LOS E	20.3	160.1	1.00	0.96	19.3
Approad	ch	282	14.9	0.886	76.9	LOS E	20.3	160.1	1.00	0.96	19.3
All Vehi	cles	1533	6.7	0.886	38.8	LOS D	49.9	354.4	0.86	0.82	28.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Moverr	nent Performance -	Pedestria	าร					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bac Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	59.1	LOS E	0.3	0.3	0.95	0.95
P2	Across S approach	72	59.1	LOS E	0.3	0.3	0.95	0.95
P7	Across W approach	16	59.1	LOS E	0.1	0.1	0.95	0.95
P8	Across W approach	40	59.1	LOS E	0.1	0.1	0.95	0.95
All Pede	estrians	200	59.1	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
2	Т	299	11.3	0.762	8.3	LOS A	14.4	104.5	1.00	0.00	42.1
3	R	562	0.0	0.762	16.8	LOS C	14.4	104.5	1.00	1.29	42.2
Approac	ch	861	3.9	0.762	13.9	NA	14.4	104.5	1.00	0.84	42.1
East: Ca	ape Stre	et									
4	L	375	0.0	1.718	694.1	LOS F	129.1	969.0	1.00	8.06	3.0
6	R	76	50.0	1.718	696.4	LOS F	129.1	969.0	1.00	4.95	3.0
Approac	h	451	8.4	1.718	694.4	LOS F	129.1	969.0	1.00	7.54	3.0
North: C	hallis St	treet									
7	L	69	60.6	0.211	10.4	LOS B	0.0	0.0	0.00	1.04	49.0
8	Т	276	6.1	0.211	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	345	17.1	0.211	2.1	NA	0.0	0.0	0.00	0.21	57.4
All Vehic	cles	1657	7.9	1.718	196.5	NA	129.1	969.0	0.79	2.53	9.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	265	13.5	0.158	4.6	LOS A	1.6	12.6	0.68	0.00	49.0
6	R	1	0.0	0.158	13.1	LOS B	1.6	12.6	0.68	1.07	47.1
Approad	ch	266	13.4	0.158	4.7	NA	1.6	12.6	0.68	0.00	49.0
North: V	Voolley S	Street									
7	L	1	0.0	0.581	28.6	LOS D	3.2	22.4	0.84	1.15	33.3
9	R	158	0.0	0.581	28.9	LOS D	3.2	22.4	0.84	1.11	33.3
Approad	ch	159	0.0	0.581	28.9	LOS D	3.2	22.4	0.84	1.11	33.3
West: C	Cape Stre	eet (W)									
10	L	491	0.0	0.369	8.2	LOS A	0.0	0.0	0.00	0.73	49.0
11	Т	141	29.9	0.369	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	632	6.7	0.369	6.4	NA	0.0	0.0	0.00	0.57	51.1
All Vehi	cles	1057	7.4	0.581	9.3	NA	3.2	22.4	0.30	0.51	46.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Cowper S	Street									
2	Т	726	1.4	0.415	4.6	LOS A	6.7	47.7	0.79	0.00	47.4
3	R	17	0.0	0.415	13.1	LOS B	6.7	47.7	0.79	1.01	47.7
Approac	h	743	1.4	0.415	4.8	NA	6.7	47.7	0.79	0.02	47.4
East: Da	avenport	Street (E)									
4	L	11	0.0	1.096	203.8	LOS F	14.5	104.3	1.00	2.41	9.0
6	R	116	3.6	1.096	204.2	LOS F	14.5	104.3	1.00	1.96	9.0
Approac	h	126	3.3	1.096	204.2	LOS F	14.5	104.3	1.00	2.00	9.0
North: C	owper S	Street									
7	L	86	0.0	0.264	8.2	LOS A	0.0	0.0	0.00	0.97	49.0
8	Т	392	2.7	0.264	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	478	2.2	0.264	1.5	NA	0.0	0.0	0.00	0.18	57.7
All Vehic	cles	1347	1.9	1.096	22.3	NA	14.5	104.3	0.53	0.26	35.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	lorphett	Street (E)									
4	L	262	2.0	0.625	22.4	LOS C	4.1	29.0	0.83	1.14	37.4
Approa	ch	262	2.0	0.625	22.4	LOS C	4.1	29.0	0.83	1.14	37.4
North: N	Northbou	urne Avenue (	(N)								
7	L	115	9.2	0.509	8.5	LOS A	0.0	0.0	0.00	1.02	49.0
8	Т	779	6.8	0.509	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	894	7.1	0.509	1.1	NA	0.0	0.0	0.00	0.13	58.3
All Vehi	icles	1156	5.9	0.625	5.9	NA	4.1	29.0	0.19	0.36	51.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehi	cles							
Mov ID	Turn	Demand	ΗV	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Northbou	Irne Avenue	(S)								
2	Т	767	9.3	0.147	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	366	1.7	1.110	147.8	LOS F	31.5	223.5	1.00	3.20	12.1
Approac	ch	1133	6.8	1.110	47.7	NA	31.5	223.5	0.32	1.03	26.1
North: N	lorthbou	rne Avenue (	(N)								
7	L	115	9.2	0.484	8.5	LOS A	0.0	0.0	0.00	1.02	49.0
8	Т	2492	4.2	0.484	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	2606	4.4	0.484	0.4	NA	0.0	0.0	0.00	0.04	59.4
All Vehic	cles	3739	5.2	1.110	14.7	NA	31.5	223.5	0.10	0.34	42.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	55	3.8	0.378	5.4	LOS A	2.3	15.9	0.68	0.00	46.3
6	R	242	0.0	0.378	13.7	LOS B	2.3	15.9	0.68	0.98	43.9
Approac	h	297	0.7	0.378	12.2	NA	2.3	15.9	0.68	0.80	44.4
North: C	hallis St	treet									
7	L	295	0.0	0.371	11.4	LOS B	1.9	13.5	0.53	0.83	45.6
<mark>9</mark>	R	<mark>296</mark>	4.1	<mark>1.000</mark> 3	70.5	LOS F	15.4	111.8	1.00	1.92	20.3
Approac	h	592	2.8	1.000	41.0	LOS E	15.4	111.8	0.76	1.38	28.1
West: M	orphett	Street (W)									
10	L	499	6.8	0.356	8.4	LOS A	0.0	0.0	0.00	0.72	49.0
11	Т	109	0.0	0.356	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	608	5.5	0.356	6.9	NA	0.0	0.0	0.00	0.59	50.6
All Vehic	cles	1497	3.5	1.000	21.4	NA	15.4	111.8	0.44	0.94	37.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

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Cowper Street - Morphett Street Do Nothing 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	84	0.0	0.355	8.2	LOS A	0.0	0.0	0.00	1.00	49.0
2	Т	562	1.5	0.355	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	646	1.3	0.355	1.1	NA	0.0	0.0	0.00	0.13	58.3
North: C	Cowper S	Street (N)									
8	Т	299	3.5	0.296	5.6	LOS A	2.7	19.6	0.72	0.00	47.5
9	R	93	0.0	0.296	14.0	LOS B	2.7	19.6	0.72	1.04	45.5
Approac	ch	392	2.7	0.296	7.6	NA	2.7	19.6	0.72	0.25	47.0
West: M	lorphett	Street									
10	L	181	1.2	0.607	23.6	LOS C	3.9	27.2	0.78	1.14	36.3
12	R	55	0.0	0.607	23.8	LOS C	3.9	27.2	0.78	1.11	36.3
Approad	ch	236	0.9	0.607	23.6	LOS C	3.9	27.2	0.78	1.14	36.3
All Vehi	cles	1274	1.7	0.607	7.2	NA	3.9	27.2	0.36	0.35	49.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street Do Nothing 2031 MD Signals - Fixed Time Cycle Time = 90 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	erformance	- Vehic	les							
Mov ID		Demand Flow		eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	irne Avenue (	(S)								
1	L	101	6.3	0.108	12.7	LOS B	1.4	10.4	0.42	0.69	44.7
2	Т	663	3.8	0.693	35.6	LOS D	11.7	84.9	0.96	0.82	29.0
3	R	139	1.5	0.717	52.9	LOS D	6.4	45.6	1.00	0.87	24.9
Approa	ich	903	3.7	0.717	35.7	LOS D	11.7	84.9	0.90	0.82	29.4
East: A	ntill Stree	et									
4	L	341	0.0	0.412	13.1	LOS B	5.5	38.5	0.49	0.73	44.2
5	Т	474	5.8	0.898	45.8	LOS D	15.9	116.5	0.98	0.96	25.4
6	R	316	4.0	0.898	59.7	LOS E	15.9	116.5	1.00	1.08	23.1
Approa	ich	1131	3.5	0.898	39.8	LOS D	15.9	116.5	0.84	0.92	28.3
North:	Northbou	rne Avenue (	N)								
7	L	389	3.2	0.373	13.8	LOS B	7.3	52.5	0.53	0.74	43.6
8	Т	808	2.3	0.665	35.6	LOS D	11.2	79.8	0.97	0.83	28.9
9	R	175	0.0	0.893	62.2	LOS E	9.1	63.7	1.00	1.04	22.4
Approa	ich	1373	2.3	0.893	32.8	LOS C	11.2	79.8	0.85	0.83	30.7
West: I	Vouat Str	reet (W)									
10	L	145	0.0	0.886	58.2	LOS E	15.2	109.4	1.00	1.14	23.7
11	Т	444	5.7	0.886	50.9	LOS D	15.2	109.4	1.00	1.09	23.8
12	R	152	4.2	0.532	45.5	LOS D	6.3	45.8	0.96	0.80	27.2
Approa	ich	741	4.3	0.886	51.2	LOS D	15.2	109.4	0.99	1.04	24.4
All Veh	icles	4147	3.3	0.898	38.6	LOS D	15.9	116.5	0.88	0.89	28.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moverr	nent Performance -	Pedestrian	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bao Pedestrian	ck of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	20.0	LOS B	0.1	0.1	0.67	0.67
P2	Across S approach	72	35.6	LOS D	0.2	0.2	0.89	0.89
P3	Across E approach	107	39.2	LOS D	0.3	0.3	0.93	0.93
P5	Across N approach	47	20.0	LOS B	0.1	0.1	0.67	0.67
P6	Across N approach	47	37.4	LOS D	0.1	0.1	0.91	0.91
P7	Across W approach	16	39.2	LOS D	0.0	0.0	0.93	0.93
All Pede	estrians	361	31.9	LOS D			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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# SIDRA ----INTERSECTION

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Project: X:\PROJECT\3002303 Dickson Precinct Traffic and Parking Study\015 Traffic - Intersection Flow Modelling\Dickson Intersections\_WE\_MD\_DoNothing2031.sip 8000072, SMEC AUSTRALIA PTY. LTD., FLOATING

#### Site: 1 MD

#### Northbourne Avenue Service Road - Antill Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back of Queue		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North W	/est: Nor	thbourne Av	enue Ser	vice Road							
27	L	38	11.1	0.094	15.8	LOS C	0.3	2.5	0.66	0.89	42.1
Approa	ch	38	11.1	0.094	15.8	LOS C	0.3	2.5	0.66	0.89	42.1
West: A	ntill Stre	et (W)									
10	L	8	25.0	0.270	9.9	LOS A	0.0	0.0	0.00	1.22	48.1
11	Т	964	3.9	0.270	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	973	4.1	0.270	0.1	NA	0.0	0.0	0.00	0.01	59.9
All Vehi	cles	1011	4.4	0.270	0.7	NA	0.3	2.5	0.02	0.04	58.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Mover	ment Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV Deg. Satn		Average	Level of	95% Back of Queue		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	206	17.3	0.611	24.9	LOS C	3.4	27.6	0.85	1.13	36.1
Approa	ich	206	17.3	0.611	24.9	LOS C	3.4	27.6	0.85	1.13	36.1
East: A	ntill Stree	et (W)									
4	L	42	40.0	0.088	11.2	LOS B	0.2	2.2	0.37	0.64	47.1
5	Т	941	0.4	0.255	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ich	983	2.1	0.255	0.5	NA	0.2	2.2	0.02	0.03	59.3
West: A	Antill Stre	et									
11	Т	760	0.3	0.206	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	244	16.4	0.576	21.4	LOS C	2.8	22.7	0.83	1.08	38.2
Approa	ich	1004	4.2	0.576	5.2	NA	2.8	22.7	0.20	0.26	52.7
All Veh	icles	2194	4.5	0.611	4.9	NA	3.4	27.6	0.18	0.24	53.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street Do Nothing 2031 MD Stop (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	499	0.0	1.415	403.5	LOS F	132.8	939.5	1.00	7.21	5.0
3	R	158	5.3	1.415	403.4	LOS F	132.8	939.5	1.00	5.70	4.9
Approa	ch	657	1.3	1.415	403.5	LOS F	132.8	939.5	1.00	6.85	5.0
East: Ar	ntill Stree	et (W)									
4	L	154	0.0	0.204	11.2	LOS B	0.7	5.2	0.53	0.79	46.0
5	Т	566	3.7	0.157	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	720	2.9	0.204	2.4	NA	0.7	5.2	0.11	0.17	56.3
West: A	ntill Stre	et (W)									
11	Т	267	0.8	0.145	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	600	0.0	0.702	15.2	LOS C	6.9	48.1	0.70	1.12	42.3
Approa	ch	867	0.2	0.702	10.5	NA	6.9	48.1	0.48	0.78	46.5
South West: Median (RT Stage 2)											
32	R	158	5.3	0.152	9.2	LOS A	0.6	4.0	0.36	0.67	33.1
Approa	ch	158	5.3	0.152	9.2	LOS A	0.6	4.0	0.36	0.67	33.1
All Vehi	cles	2402	1.7	1.415	115.5	NA	132.8	939.5	0.51	2.25	13.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

 $\label{eq:model} \ensuremath{\mathsf{Minor}}\xspace \ensuremath{\mathsf{Road}}\xspace \ensuremath{\mathsf{Approach}}\xspace \ensuremath{\mathsf{LOS}}\xspace \ensuremath{\mathsf{values}}\xspace \ensuremath{\mathsf{aproach}}\xspace \ensuremath{\mathsf{aproach$ 

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV Deg. Satn		Average Delav	Level of Service	95% Back of Queue Vehicles Distance		Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		ven	m		per veh	km/h
South: [	Dickson	Shop Access	Road								
1	L	211	0.0	0.283	10.8	LOS B	1.3	8.8	0.54	0.78	46.4
Approa	ch	211	0.0	0.283	10.8	LOS B	1.3	8.8	0.54	0.78	46.4
East: Ar	ntill Stree	et (W)									
4	L	1	0.0	0.142	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	509	4.1	0.142	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	511	4.1	0.142	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	721	2.9	0.283	3.2	NA	1.3	8.8	0.16	0.23	55.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Do Nothing 2031 MD Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	99	2.1	0.190	25.4	LOS C	2.2	15.7	0.78	0.76	35.3
3	R	326	0.6	0.684	29.0	LOS C	8.8	61.9	0.91	0.86	33.5
Approa	ch	425	1.0	0.684	28.2	LOS C	8.8	61.9	0.88	0.84	33.9
East: A	ntill Stree	et (E)									
4	L	185	2.3	0.356	26.5	LOS C	4.4	31.2	0.83	0.80	34.7
5	Т	411	4.6	0.623	26.1	LOS C	5.9	43.2	0.97	0.82	33.1
Approa	ch	596	3.9	0.623	26.2	LOS C	5.9	43.2	0.93	0.81	33.6
West: A	ntill Stre	et (W)									
11	Т	288	3.6	0.435	24.6	LOS C	3.9	28.5	0.93	0.75	33.9
12	R	139	0.0	0.364	30.5	LOS C	3.6	25.3	0.89	0.79	32.7
Approa	ch	427	2.5	0.435	26.5	LOS C	3.9	28.5	0.92	0.76	33.5
All Vehi	cles	1448	2.6	0.684	26.9	LOS C	8.8	61.9	0.91	0.80	33.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bac Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	Across S approach	72	23.4	LOS C	0.1	0.1	0.88	0.88				
P2	Across S approach	72	21.7	LOS C	0.1	0.1	0.85	0.85				
P3	Across E approach	107	24.3	LOS C	0.2	0.2	0.90	0.90				
P4	Across E approach	12	21.7	LOS C	0.0	0.0	0.85	0.85				
All Pede	estrians	263	23.2	LOS C			0.88	0.88				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Rosevea	r Place (S)									
1	L	40	0.0	0.060	11.1	LOS B	0.2	1.5	0.49	0.75	45.9
3	R	1	0.0	0.060	10.7	LOS B	0.2	1.5	0.49	0.78	46.3
Approa	ch	41	0.0	0.060	11.1	LOS B	0.2	1.5	0.49	0.75	45.9
East: A	ntill Stree	et (E)									
4	L	1	0.0	0.134	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	484	3.9	0.134	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	485	3.9	0.134	0.0	NA	0.0	0.0	0.00	0.00	60.0
West: A	ntill Stre	et									
11	Т	499	2.1	0.137	1.3	LOS A	1.0	7.4	0.24	0.00	55.5
12	R	1	0.0	0.137	10.7	LOS B	1.0	7.4	0.49	0.95	48.5
Approa	ch	500	2.1	0.137	1.3	NA	1.0	7.4	0.24	0.00	55.5
South V	Vest: Me	dian (RT Stag	ge 2)								
32	R	1	0.0	0.002	12.8	LOS B	0.0	0.1	0.54	0.64	28.3
Approa	ch	1	0.0	0.002	12.8	LOS B	0.0	0.1	0.54	0.64	28.3
All Vehi	cles	1027	2.9	0.137	1.1	NA	1.0	7.4	0.14	0.03	57.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

 $\label{eq:model} \ensuremath{\mathsf{Minor}}\xspace \ensuremath{\mathsf{Road}}\xspace \ensuremath{\mathsf{Approach}}\xspace \ensuremath{\mathsf{LOS}}\xspace \ensuremath{\mathsf{values}}\xspace \ensuremath{\mathsf{aproach}}\xspace \ensuremath{\mathsf{aproach$ 

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	352	0.0	0.658	14.4	LOS B	8.0	55.7	1.00	0.00	38.6
3	R	259	0.0	0.658	22.9	LOS C	8.0	55.7	1.00	1.25	38.4
Approad	ch	611	0.0	0.658	18.0	NA	8.0	55.7	1.00	0.53	38.5
East: Di	pproach 611 ast: Dickson Shop Access 1 4 L 53		Road (E)								
4	L	53	0.0	1.946	936.4	LOS F	59.4	416.1	1.00	4.10	2.2
6	R	120	0.0	1.946	936.6	LOS F	59.4	416.1	1.00	3.41	2.2
Approad	ch	173	0.0	1.946	936.6	LOS F	59.4	416.1	1.00	3.62	2.2
North: E	Badham	Street (N)									
7	L	164	0.0	0.410	8.2	LOS A	0.0	0.0	0.00	0.95	49.0
8	Т	585	0.0	0.410	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	749	0.0	0.410	1.8	NA	0.0	0.0	0.00	0.21	57.2
All Vehi	cles	1533	0.0	1.946	113.5	NA	59.4	416.1	0.51	0.72	14.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	225	0.0	0.258	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
2	Т	238	1.8	0.258	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	463	0.9	0.258	4.0	NA	0.0	0.0	0.00	0.40	54.1
North: C	Cowper S	Street (N)									
8	Т	189	2.2	0.249	2.9	LOS A	1.6	11.1	0.48	0.00	50.5
9	R	133	0.0	0.249	11.4	LOS B	1.6	11.1	0.48	0.89	46.9
Approac	ch	322	1.3	0.249	6.4	NA	1.6	11.1	0.48	0.36	48.9
West: D	ickson S	Shop Access	Road (W	)							
10	L	185	0.0	0.289	11.1	LOS B	1.3	9.0	0.51	0.72	46.0
12	R	25	0.0	0.289	11.7	LOS B	1.3	9.0	0.51	0.88	45.7
Approac	ch	211	0.0	0.289	11.2	LOS B	1.3	9.0	0.51	0.74	46.0
All Vehi	cles	996	0.8	0.289	6.3	NA	1.6	11.1	0.26	0.46	50.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	118	0.0	0.206	8.2	LOS A	0.0	0.0	0.00	0.90	49.0
2	Т	257	0.0	0.206	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	375	0.0	0.206	2.6	NA	0.0	0.0	0.00	0.28	56.0
North: B	Badham	Street (N)									
8	Т	383	0.0	0.559	4.9	LOS A	7.5	52.8	0.78	0.00	45.7
9	R	341	0.0	0.559	13.3	LOS B	7.5	52.8	0.78	1.04	45.5
Approac	ch	724	0.0	0.559	8.9	NA	7.5	52.8	0.78	0.49	45.6
West: W	loolley S	Street									
10	L	366	0.0	1.009	89.5	LOS F	31.2	218.5	1.00	2.80	17.3
12	R	72	0.0	1.009	89.7	LOS F	31.2	218.5	1.00	2.12	17.2
Approac	ch	438	0.0	1.009	89.5	LOS F	31.2	218.5	1.00	2.69	17.3
All Vehi	cles	1537	0.0	1.009	30.3	NA	31.2	218.5	0.65	1.07	32.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV C	)eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	67	0.0	0.306	6.2	LOS A	1.9	15.6	0.67	0.00	46.1
3	R	154	26.0	0.306	15.7	LOS C	1.9	15.6	0.67	0.98	43.4
Approa	ch	221	18.1	0.306	12.8	NA	1.9	15.6	0.67	0.68	44.2
East: Di	ickson P	lace									
4	L	152	20.8	1.136	167.7	LOS F	44.3	331.1	1.00	3.74	10.7
6	R	253	0.0	1.136	167.2	LOS F	44.3	331.1	1.00	3.32	10.6
Approa	ch	404	7.8	1.136	167.4	LOS F	44.3	331.1	1.00	3.48	10.7
North: E	Badham	Street (N)									
7	L	179	0.0	0.289	8.2	LOS A	0.0	0.0	0.00	0.89	49.0
8	Т	347	0.0	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	526	0.0	0.289	2.8	NA	0.0	0.0	0.00	0.30	55.7
All Vehi	cles	1152	6.2	1.136	62.5	NA	44.3	331.1	0.48	1.49	22.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Do Nothing 2031 MD Signals - Fixed Time Cycle Time = 130 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	e - Vehi	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	389	0.0	0.856	46.2	LOS D	43.2	307.3	0.96	0.95	27.0
2	Т	326	3.9	0.856	38.0	LOS D	43.2	307.3	0.96	0.91	27.2
Approad	ch	716	1.8	0.856	42.5	LOS D	43.2	307.3	0.96	0.93	27.1
North: C	Cowper S	Street (N)									
8	Т	179	7.1	0.180	14.6	LOS B	5.4	40.2	0.52	0.44	41.3
9	R	63	50.0	0.508	40.3	LOS D	2.1	20.6	0.95	0.78	29.1
Approad	ch	242	18.3	0.508	21.3	LOS C	5.4	40.2	0.63	0.53	37.3
West: D	ickson F	Place (W)									
10	L	143	27.9	0.865	70.8	LOS E	23.1	178.4	1.00	0.94	20.5
12	R	192	0.0	0.865	69.9	LOS E	23.1	178.4	1.00	0.94	20.5
Approad	ch	335	11.9	0.865	70.3	LOS E	23.1	178.4	1.00	0.94	20.5
All Vehi	cles	1293	7.5	0.865	45.7	LOS D	43.2	307.3	0.91	0.86	26.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	IS					
Mov	Description	Demand	Average	Level of			Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	59.1	LOS E	0.3	0.3	0.95	0.95
P2	Across S approach	72	59.1	LOS E	0.3	0.3	0.95	0.95
P7	Across W approach	16	59.1	LOS E	0.1	0.1	0.95	0.95
P8	Across W approach	12	59.1	LOS E	0.0	0.0	0.95	0.95
All Pede	estrians	172	59.1	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Challis S	treet (S)									
2	Т	95	13.3	0.482	3.7	LOS A	3.9	27.9	0.61	0.00	47.1
3	R	411	0.0	0.482	12.1	LOS B	3.9	27.9	0.61	0.94	45.4
Approad	ch	505	2.5	0.482	10.5	NA	3.9	27.9	0.61	0.76	45.7
East: Ca	ape Stre	et									
4	L	459	0.0	1.366	369.5	LOS F	114.4	838.5	1.00	6.02	5.4
6	R	118	26.8	1.366	370.8	LOS F	114.4	838.5	1.00	4.33	5.3
Approad	ch	577	5.5	1.366	369.8	LOS F	114.4	838.5	1.00	5.68	5.4
North: C	Challis S	treet									
7	L	173	23.2	0.211	9.0	LOS A	0.0	0.0	0.00	0.83	49.0
8	Т	168	10.0	0.211	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	341	16.7	0.211	4.6	NA	0.0	0.0	0.00	0.42	53.9
All Vehi	cles	1423	7.1	1.366	154.7	NA	114.4	838.5	0.62	2.67	11.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	377	8.4	0.216	4.4	LOS A	2.3	17.1	0.70	0.00	48.7
6	R	1	0.0	0.216	12.8	LOS B	2.3	17.1	0.70	1.06	47.5
Approac	ch	378	8.4	0.216	4.4	NA	2.3	17.1	0.70	0.00	48.7
North: V	Approach Iorth: Woolley Street 7 L										
7	L	1	0.0	0.756	44.5	LOS E	4.8	33.4	0.93	1.37	26.8
9	R	160	0.0	0.756	44.8	LOS E	4.8	33.4	0.93	1.27	26.8
Approad	ch	161	0.0	0.756	44.8	LOS E	4.8	33.4	0.93	1.27	26.8
West: C	ape Stre	eet (W)									
10	L	354	0.0	0.338	8.2	LOS A	0.0	0.0	0.00	0.78	49.0
11	Т	227	17.6	0.338	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	581	6.9	0.338	5.0	NA	0.0	0.0	0.00	0.48	52.8
All Vehi	cles	1120	6.4	0.756	10.5	NA	4.8	33.4	0.37	0.43	45.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street									
2	Т	634	2.0	0.367	2.6	LOS A	4.0	28.6	0.64	0.00	49.3
3	R	21	0.0	0.367	11.1	LOS B	4.0	28.6	0.64	0.92	49.0
Approad	ch	655	1.9	0.367	2.9	NA	4.0	28.6	0.64	0.03	49.3
East: Da	avenport	t Street (E)									
4	L	2	0.0	0.524	39.2	LOS E	2.3	16.1	0.89	0.95	28.7
6	R	88	0.0	0.524	39.5	LOS E	2.3	16.1	0.89	1.08	28.6
Approad	ch	91	0.0	0.524	39.5	LOS E	2.3	16.1	0.89	1.07	28.6
North: C	Cowper S	Street									
7	L	97	2.2	0.205	8.3	LOS A	0.0	0.0	0.00	0.93	49.0
8	Т	269	3.9	0.205	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	366	3.4	0.205	2.2	NA	0.0	0.0	0.00	0.25	56.6
All Vehi	cles	1112	2.3	0.524	5.6	NA	4.0	28.6	0.45	0.19	48.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett \$	Street (E)									
4	L	387	3.3	0.487	12.3	LOS B	3.7	26.6	0.61	0.87	45.1
Approa	ch	387	3.3	0.487	12.3	LOS B	3.7	26.6	0.61	0.87	45.1
North: N	Northbou	irne Avenue (	(N)								
7	L	29	14.3	0.234	8.7	LOS A	0.0	0.0	0.00	1.07	49.0
8	Т	395	1.9	0.234	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	424	2.7	0.234	0.6	NA	0.0	0.0	0.00	0.07	59.1
All Vehi	cles	812	3.0	0.487	6.2	NA	3.7	26.6	0.29	0.46	51.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les									
Mov ID	Turn	Demand	HV C	)eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South: Northbourne Avenue (S)         Z         T         1023         2.7         0.188         0.0         LOS A         0.0         0.0         0.00         60													
2	Т	1023	2.7	0.188	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
3	R	417	3.0	0.426	11.4	LOS B	2.4	16.9	0.65	0.93	46.0		
Approa	ch	1440	2.8	0.426	3.3	NA	2.4	16.9	0.19	0.27	55.1		
North: N	lorthbou	rne Avenue (	(N)										
7	L	29	14.3	0.229	8.7	LOS A	0.0	0.0	0.00	1.07	49.0		
8	Т	1227	1.2	0.229	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approa	ch	1257	1.5	0.229	0.2	NA	0.0	0.0	0.00	0.03	59.7		
All Vehi	cles	2697	2.2	0.426	1.9	NA	2.4	16.9	0.10	0.16	57.1		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	11	0.0	0.125	2.3	LOS A	0.5	3.6	0.48	0.00	49.3
6	R	105	0.0	0.125	10.6	LOS B	0.5	3.6	0.48	0.77	46.5
Approad	ch	116	0.0	0.125	9.9	NA	0.5	3.6	0.48	0.70	46.8
North: C	Challis S	treet									
7	L	164	2.6	0.180	9.8	LOS A	0.7	5.0	0.38	0.70	47.3
9	R	377	3.4	0.742	22.5	LOS C	8.3	59.8	0.81	1.27	37.0
Approad	ch	541	3.1	0.742	18.7	LOS C	8.3	59.8	0.68	1.10	39.6
West: N	/lorphett	Street (W)									
10	L	387	3.3	0.258	8.3	LOS A	0.0	0.0	0.00	0.70	49.0
11	Т	59	7.1	0.258	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	446	3.8	0.258	7.2	NA	0.0	0.0	0.00	0.61	50.2
All Vehi	cles	1103	3.1	0.742	13.1	NA	8.3	59.8	0.38	0.86	44.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Do Nothing 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehio	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	38	0.0	0.335	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
2	Т	573	1.8	0.335	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	611	1.7	0.335	0.5	NA	0.0	0.0	0.00	0.06	59.2
North: C	Cowper S	Street (N)									
8	Т	240	4.4	0.170	4.1	LOS A	1.4	10.2	0.58	0.00	49.9
9	R	27	0.0	0.170	12.5	LOS B	1.4	10.2	0.58	1.01	46.9
Approac	ch	267	3.9	0.170	5.0	NA	1.4	10.2	0.58	0.10	49.5
West: M	lorphett	Street									
10	L	80	2.6	0.247	15.9	LOS C	1.0	7.0	0.65	0.90	41.7
12	R	27	0.0	0.247	16.1	LOS C	1.0	7.0	0.65	0.92	41.7
Approad	ch	107	2.0	0.247	15.9	LOS C	1.0	7.0	0.65	0.90	41.7
All Vehi	cles	985	2.4	0.335	3.4	NA	1.4	10.2	0.23	0.17	53.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street Do Nothing 2031 EV Signals - Fixed Time Cycle Time = 85 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	erformance	- Vehic	les							
Mov IE	) Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue (	(S)								
1	L	53	0.0	0.046	11.3	LOS B	0.6	4.2	0.38	0.67	45.8
2	Т	408	4.6	0.405	29.9	LOS C	6.1	44.7	0.88	0.72	31.5
3	R	76	0.0	0.522	50.5	LOS D	3.2	22.7	1.00	0.77	25.5
Approa	ach	537	3.5	0.522	31.0	LOS C	6.1	44.7	0.85	0.72	31.4
East: A	ntill Stree	et									
4	L	276	0.8	0.325	12.1	LOS B	3.8	26.7	0.45	0.72	45.1
5	Т	461	3.2	0.818	39.0	LOS D	12.5	90.1	0.98	0.89	27.7
6	R	213	4.0	0.659	43.7	LOS D	8.6	62.3	0.98	0.85	27.6
Approa	ach	949	2.7	0.818	32.2	LOS C	12.5	90.1	0.83	0.83	31.2
North:	Northbou	rne Avenue (	N)								
7	L	227	3.7	0.202	11.2	LOS B	2.8	20.1	0.40	0.70	46.0
8	Т	718	2.1	0.557	31.5	LOS C	8.9	63.7	0.93	0.78	30.7
9	R	114	0.0	0.783	54.8	LOS D	5.2	36.5	1.00	0.90	24.2
Approa	ach	1059	2.2	0.783	29.7	LOS C	8.9	63.7	0.82	0.77	32.0
West:	Mouat Str	reet (W)									
10	L	51	0.0	0.809	52.8	LOS D	11.1	79.5	1.00	1.00	25.4
11	Т	446	3.8	0.809	43.7	LOS D	11.1	79.5	1.00	0.98	26.0
12	R	166	1.3	0.578	43.8	LOS D	6.6	46.9	0.97	0.81	27.7
Approa	ich	663	2.9	0.809	44.5	LOS D	11.1	79.5	0.99	0.93	26.3
All Veh	icles	3208	2.7	0.818	33.7	LOS C	12.5	90.1	0.86	0.81	30.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bao Pedestrian	ck of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	19.1	LOS B	0.1	0.1	0.67	0.67
P2	Across S approach	72	34.0	LOS D	0.2	0.2	0.89	0.89
P3	Across E approach	107	36.7	LOS D	0.2	0.2	0.93	0.93
P5	Across N approach	47	19.1	LOS B	0.1	0.1	0.67	0.67
P6	Across N approach	47	35.8	LOS D	0.1	0.1	0.92	0.92
P7	Across W approach	16	36.7	LOS D	0.0	0.0	0.93	0.93
All Pede	estrians	361	30.2	LOS D			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### Northbourne Avenue Service Road - Antill Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North West: Northbourne Avenue Service Road											
27	L	40	0.0	0.068	12.1	LOS B	0.2	1.7	0.55	0.79	44.9
Approac	ch	40	0.0	0.068	12.1	LOS B	0.2	1.7	0.55	0.79	44.9
West: A	ntill Stre	et (W)									
10	L	2	0.0	0.208	9.1	LOS A	0.0	0.0	0.00	1.19	48.1
11	Т	749	3.4	0.208	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	752	3.4	0.208	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	792	3.2	0.208	0.6	NA	0.2	1.7	0.03	0.04	59.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Challis S	treet (S)									
1	L	196	11.8	0.401	15.9	LOS C	2.0	15.2	0.68	0.95	42.2
Approa	ch	196	11.8	0.401	15.9	LOS C	2.0	15.2	0.68	0.95	42.2
East: Ar	ntil Stree	t (W)									
4	L	21	40.0	0.045	11.6	LOS B	0.1	1.2	0.39	0.64	46.7
5	Т	749	0.3	0.203	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	771	1.4	0.203	0.3	NA	0.1	1.2	0.01	0.02	59.5
West: A	ntill Stre	et									
11	Т	482	0.4	0.131	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	309	7.5	0.478	15.1	LOS C	2.5	18.3	0.69	0.99	42.6
Approa	ch	792	3.2	0.478	5.9	NA	2.5	18.3	0.27	0.39	51.8
All Vehi	cles	1758	3.4	0.478	4.6	NA	2.5	18.3	0.20	0.29	53.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street Do Nothing 2031 EV Stop (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
1	L	411	0.0	0.922	37.1	LOS E	17.1	120.4	0.87	1.96	30.4
3	R	114	3.7	0.922	36.9	LOS E	17.1	120.4	0.87	1.74	30.5
Approa	ch	524	0.8	0.922	37.0	LOS E	17.1	120.4	0.87	1.91	30.4
East: A	ntill Stree	et (W)									
4	L	232	0.0	0.258	10.2	LOS B	1.0	7.2	0.46	0.73	47.0
5	Т	444	1.9	0.122	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	676	1.2	0.258	3.5	NA	1.0	7.2	0.16	0.25	54.7
West: A	Antill Stre	et (W)									
11	Т	211	1.0	0.115	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	465	0.0	0.480	11.5	LOS B	3.1	21.9	0.50	0.83	45.6
Approa	ch	676	0.3	0.480	7.9	NA	3.1	21.9	0.34	0.57	49.3
South V	Vest: Me	dian (RT Stag	ge 2)								
32	R	114	3.7	0.103	8.8	LOS A	0.4	2.6	0.31	0.64	33.5
Approa	ch	114	3.7	0.103	8.8	LOS A	0.4	2.6	0.31	0.64	33.5
All Vehi	icles	1989	1.0	0.922	14.2	NA	17.1	120.4	0.42	0.82	43.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

 $\label{eq:model} \ensuremath{\mathsf{Minor}}\xspace \ensuremath{\mathsf{Road}}\xspace \ensuremath{\mathsf{Approach}}\xspace \ensuremath{\mathsf{LOS}}\xspace \ensuremath{\mathsf{values}}\xspace \ensuremath{\mathsf{aproach}}\xspace \ensuremath{\mathsf{aproach$ 

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Do Nothing 2031 EV Giveway / Yield (Two-Way)

Movem	ent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Dickson Shop Access Road											
1	L	126	0.0	0.177	10.7	LOS B	0.7	4.9	0.52	0.76	46.5
Approac	h	126	0.0	0.177	10.7	LOS B	0.7	4.9	0.52	0.76	46.5
East: An	till Stree	et (W)									
4	L	1	0.0	0.150	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	547	1.5	0.150	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	548	1.5	0.150	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehic	les	675	1.2	0.177	2.0	NA	0.7	4.9	0.10	0.14	56.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Do Nothing 2031 EV Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	122	0.0	0.433	30.1	LOS C	2.9	20.2	0.95	0.78	32.8
3	R	133	0.0	0.470	29.9	LOS C	3.2	22.1	0.95	0.78	33.0
Approad	ch	255	0.0	0.470	30.0	LOS C	3.2	22.1	0.95	0.78	32.9
East: Ar	ntill Stree	et (E)									
4	L	145	4.3	0.531	30.7	LOS C	3.5	25.5	0.96	0.80	32.5
5	Т	432	2.0	0.537	19.6	LOS B	4.9	35.0	0.93	0.76	36.8
Approad	ch	577	2.6	0.537	22.4	LOS C	4.9	35.0	0.94	0.77	35.6
West: A	ntill Stre	et (W)									
11	Т	251	2.5	0.313	18.5	LOS B	2.7	19.2	0.88	0.70	37.7
12	R	76	0.0	0.165	24.0	LOS C	1.5	10.4	0.81	0.75	36.2
Approad	ch	326	1.9	0.313	19.7	LOS B	2.7	19.2	0.86	0.71	37.3
All Vehi	cles	1158	1.8	0.537	23.3	LOS C	4.9	35.0	0.92	0.76	35.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestriar	IS					
Maria	Description	Demand	Average	Level of			Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	18.5	LOS B	0.1	0.1	0.86	0.86
P2	Across S approach	72	16.8	LOS B	0.1	0.1	0.82	0.82
P3	Across E approach	107	19.4	LOS B	0.1	0.1	0.88	0.88
P4	P4 Across E approach		16.8	LOS B	0.0	0.0	0.82	0.82
All Pede	estrians	258	18.3	LOS B			0.86	0.86

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	Movement Performance - Vehicles Mov ID Turn Demand HV Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average											
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South: F	Rosevea	r Place (S)										
1	L	40	0.0	0.057	10.8	LOS B	0.2	1.4	0.46	0.73	46.3	
3	R	1	0.0	0.057	10.4	LOS B	0.2	1.4	0.46	0.76	46.6	
Approa	ch	41	0.0	0.057	10.8	LOS B	0.2	1.4	0.46	0.73	46.3	
East: Ar	ntill Stree	et (E)										
4	L	1	0.0	0.121	8.2	LOS A	0.0	0.0	0.00	1.09	49.0	
5	Т	442	1.9	0.121	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Approa	ch	443	1.9	0.121	0.0	NA	0.0	0.0	0.00	0.00	60.0	
West: A	ntill Stre	et										
11	Т	307	2.1	0.085	1.1	LOS A	0.6	4.2	0.22	0.00	55.9	
12	R	1	0.0	0.085	10.2	LOS B	0.6	4.2	0.44	0.93	48.8	
Approa	ch	308	2.0	0.085	1.1	NA	0.6	4.2	0.22	0.00	55.9	
South V	Vest: Me	dian (RT Stag	ge 2)									
32	R	1	0.0	0.002	10.3	LOS B	0.0	0.0	0.43	0.60	31.5	
Approa	ch	1	0.0	0.002	10.3	LOS B	0.0	0.0	0.43	0.60	31.5	
All Vehi	cles	794	1.9	0.121	1.0	NA	0.6	4.2	0.11	0.04	57.4	

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

 $\label{eq:model} \ensuremath{\mathsf{Minor}}\xspace \ensuremath{\mathsf{Road}}\xspace \ensuremath{\mathsf{Approach}}\xspace \ensuremath{\mathsf{LOS}}\xspace \ensuremath{\mathsf{values}}\xspace \ensuremath{\mathsf{aproach}}\xspace \ensuremath{\mathsf{aproach$ 

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
2	Т	276	0.0	0.313	7.1	LOS A	3.3	23.1	0.83	0.00	45.9
3	R	99	0.0	0.313	15.5	LOS C	3.3	23.1	0.83	1.06	44.4
Approa	ch	375	0.0	0.313	9.3	NA	3.3	23.1	0.83	0.28	45.5
East: D	East: Dickson Shop Acc		Road (E)								
4	L	61	0.0	0.754	49.7	LOS E	4.3	30.4	0.92	1.29	25.2
6	R	82	0.0	0.754	49.9	LOS E	4.3	30.4	0.92	1.24	25.2
Approa	ch	143	0.0	0.754	49.8	LOS E	4.3	30.4	0.92	1.26	25.2
North: E	Badham	Street (N)									
7	L	99	0.0	0.380	8.2	LOS A	0.0	0.0	0.00	1.00	49.0
8	Т	600	0.0	0.380	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	699	0.0	0.380	1.2	NA	0.0	0.0	0.00	0.14	58.1
All Vehi	cles	1217	0.0	0.754	9.4	NA	4.3	30.4	0.36	0.32	46.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	69	0.0	0.138	8.2	LOS A	0.0	0.0	0.00	0.92	49.0
2	Т	183	0.0	0.138	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	253	0.0	0.138	2.3	NA	0.0	0.0	0.00	0.25	56.5
North: Cowper S		Street (N)									
8	Т	168	3.8	0.137	1.2	LOS A	0.8	5.6	0.29	0.00	54.2
9	R	51	0.0	0.137	9.6	LOS A	0.8	5.6	0.29	0.91	48.5
Approac	ch	219	2.9	0.137	3.1	NA	0.8	5.6	0.29	0.21	52.8
West: D	ickson S	Shop Access	Road (W	)							
10	L	72	0.0	0.098	9.5	LOS A	0.4	2.8	0.35	0.62	47.6
12	R	13	0.0	0.098	10.0	LOS B	0.4	2.8	0.35	0.79	47.2
Approac	ch	84	0.0	0.098	9.6	LOS A	0.4	2.8	0.35	0.64	47.5
All Vehi	cles	556	1.1	0.138	3.7	NA	0.8	5.6	0.17	0.29	53.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	255	0.0	0.203	8.2	LOS A	0.0	0.0	0.00	0.75	49.0
2	Т	107	0.0	0.203	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	362	0.0	0.203	5.8	NA	0.0	0.0	0.00	0.53	51.8
North: Badham S		Street (N)									
8	Т	349	0.0	0.617	5.4	LOS A	9.0	63.1	0.83	0.00	44.8
9	R	421	0.0	0.617	13.8	LOS B	9.0	63.1	0.83	1.07	44.9
Approac	ch	771	0.0	0.617	10.0	NA	9.0	63.1	0.83	0.58	44.8
West: W	Voolley S	Street									
10	L	267	0.0	0.969	75.3	LOS F	22.2	155.2	0.72	2.15	19.5
12	R	93	0.0	0.969	75.5	LOS F	22.2	155.2	0.72	1.77	19.4
Approac	ch	360	0.0	0.969	75.3	LOS F	22.2	155.2	0.72	2.05	19.5
All Vehi	cles	1493	0.0	0.969	24.7	NA	22.2	155.2	0.60	0.92	35.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	65	0.0	0.169	5.6	LOS A	1.0	8.4	0.62	0.00	47.6
3	R	72	32.4	0.169	15.4	LOS C	1.0	8.4	0.62	0.96	44.3
Approa	ch	137	16.9	0.169	10.7	NA	1.0	8.4	0.62	0.50	45.8
East: Di	ickson P	lace									
4	L	112	17.0	0.942	49.6	LOS E	16.8	122.9	0.94	2.06	25.4
6	R	274	0.0	0.942	49.2	LOS E	16.8	122.9	0.94	1.93	25.4
Approa	ch	385	4.9	0.942	49.4	LOS E	16.8	122.9	0.94	1.97	25.4
North: E	Badham	Street (N)									
7	L	181	0.0	0.281	8.2	LOS A	0.0	0.0	0.00	0.88	49.0
8	Т	331	0.0	0.281	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	512	0.0	0.281	2.9	NA	0.0	0.0	0.00	0.31	55.6
All Vehi	cles	1034	4.1	0.942	21.2	NA	16.8	122.9	0.43	0.95	37.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Do Nothing 2031 EV Signals - Fixed Time Cycle Time = 100 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehi	cles							
Mov ID	Turn	Demand Flow	ΗV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street (S)									
1	L	404	0.0	0.759	37.2	LOS D	23.3	164.1	0.93	0.89	30.0
2	Т	135	3.1	0.759	29.0	LOS C	23.3	164.1	0.93	0.84	30.4
Approa	ch	539	0.8	0.759	35.1	LOS D	23.3	164.1	0.93	0.88	30.1
North: C	Cowper S	Street (N)									
8	Т	139	7.6	0.152	13.3	LOS B	3.5	26.0	0.55	0.45	42.3
9	R	61	31.0	0.339	30.3	LOS C	1.6	14.6	0.88	0.76	33.2
Approa	ch	200	14.7	0.339	18.5	LOS B	3.5	26.0	0.65	0.55	39.1
West: D	Dickson F	Place (W)									
10	L	76	30.6	0.754	52.0	LOS D	13.5	101.2	0.99	0.89	24.9
12	R	200	0.0	0.754	51.0	LOS D	13.5	101.2	0.99	0.89	24.9
Approa	ch	276	8.4	0.754	51.3	LOS D	13.5	101.2	0.99	0.89	24.9
All Vehi	icles	1015	5.6	0.759	36.2	LOS D	23.3	164.1	0.89	0.81	29.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestrian	IS					
Mov	Description	Demand Flow	Average Delav	Level of Service	Average Bac Pedestrian	ck of Queue Distance	Prop. Queued	Effective Stop Rate
ID			,			Diotarioo		
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	44.2	LOS E	0.2	0.2	0.94	0.94
P2	Across S approach	72	44.2	LOS E	0.2	0.2	0.94	0.94
P7	Across W approach	16	44.2	LOS E	0.0	0.0	0.94	0.94
P8	Across W approach	12	44.2	LOS E	0.0	0.0	0.94	0.94
All Pede	estrians	172	44.2	LOS E			0.94	0.94

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV C	)eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
2	Т	122	6.9	0.637	5.6	LOS A	7.2	50.7	0.75	0.00	45.0
3	R	528	0.0	0.637	14.1	LOS B	7.2	50.7	0.75	1.10	43.7
Approa	ch	651	1.3	0.637	12.5	NA	7.2	50.7	0.75	0.89	43.9
East: C	ape Stre	et									
4	L	526	0.0	1.607	586.7	LOS F	157.3	1131.1	1.00	8.13	3.5
6	R	76	25.0	1.607	588.0	LOS F	157.3	1131.1	1.00	5.31	3.5
Approa	ch	602	3.1	1.607	586.9	LOS F	157.3	1131.1	1.00	7.77	3.5
North: (	Challis S	treet									
7	L	116	20.0	0.229	8.9	LOS A	0.0	0.0	0.00	0.94	49.0
8	Т	280	3.0	0.229	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	396	8.0	0.229	2.6	NA	0.0	0.0	0.00	0.27	56.3
All Vehi	icles	1648	3.6	1.607	219.9	NA	157.3	1131.1	0.66	3.26	8.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV C	)eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: C	ape Stre	et (E)									
5	Т	314	6.0	0.178	4.8	LOS A	2.0	14.4	0.70	0.00	48.6
6	R	1	0.0	0.178	13.3	LOS B	2.0	14.4	0.70	1.06	47.1
Approa	ch	315	6.0	0.178	4.8	NA	2.0	14.4	0.70	0.00	48.6
North: Woolley		Street									
7	L	1	0.0	0.980	80.5	LOS F	14.7	102.6	0.99	2.26	18.6
9	R	253	0.0	0.980	80.8	LOS F	14.7	102.6	0.99	1.98	18.5
Approa	ch	254	0.0	0.980	80.8	LOS F	14.7	102.6	0.99	1.98	18.5
West: C	Cape Stre	eet (W)									
10	L	484	0.0	0.369	8.2	LOS A	0.0	0.0	0.00	0.74	49.0
11	Т	160	14.5	0.369	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	644	3.6	0.369	6.2	NA	0.0	0.0	0.00	0.55	51.3
All Vehi	cles	1213	3.5	0.980	21.4	NA	14.7	102.6	0.39	0.71	37.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street									
2	Т	440	1.0	0.255	2.0	LOS A	2.4	16.9	0.54	0.00	50.7
3	R	17	0.0	0.255	10.4	LOS B	2.4	16.9	0.54	0.93	49.2
Approad	ch	457	0.9	0.255	2.3	NA	2.4	16.9	0.54	0.03	50.7
East: Davenpor		t Street (E)									
4	L	17	0.0	0.380	22.6	LOS C	1.7	12.2	0.73	0.83	36.8
6	R	97	0.0	0.380	22.9	LOS C	1.7	12.2	0.73	0.99	36.7
Approad	ch	114	0.0	0.380	22.8	LOS C	1.7	12.2	0.73	0.96	36.7
North: C	Cowper S	Street									
7	L	61	10.3	0.189	8.6	LOS A	0.0	0.0	0.00	0.99	49.0
8	Т	278	1.5	0.189	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	339	3.1	0.189	1.5	NA	0.0	0.0	0.00	0.18	57.7
All Vehi	cles	909	1.6	0.380	4.6	NA	2.4	16.9	0.36	0.20	50.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
2	Т	669	3.2	0.123	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	504	1.7	1.043	88.8	LOS F	31.5	223.6	1.00	2.95	17.8
Approa	ch	1173	2.5	1.043	38.2	NA	31.5	223.6	0.43	1.27	29.5
East: M	lorphett S	Street (E)									
4	L	343	2.5	0.916	43.7	LOS E	10.9	77.6	0.96	1.75	27.5
Approa	ch	343	2.5	0.916	43.7	LOS E	10.9	77.6	0.96	1.75	27.5
North: N	Northbou	rne Avenue (	(N)								
7	L	46	0.0	0.201	8.2	LOS A	0.0	0.0	0.00	1.00	49.0
8	Т	1053	1.8	0.201	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1099	1.7	0.201	0.3	NA	0.0	0.0	0.00	0.04	59.4
All Vehi	icles	2615	2.2	1.043	23.0	NA	31.5	223.6	0.32	0.82	36.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	13	0.0	0.190	3.4	LOS A	0.8	5.5	0.55	0.00	48.0
6	R	139	0.0	0.190	11.7	LOS B	0.8	5.5	0.55	0.85	45.5
Approad	ch	152	0.0	0.190	11.0	NA	0.8	5.5	0.55	0.78	45.7
North: Challis Stre		treet									
7	L	272	0.0	0.324	10.6	LOS B	1.5	10.4	0.48	0.78	46.4
9	R	331	2.5	0.782	27.9	LOS D	8.3	59.1	0.84	1.36	33.9
Approad	ch	602	1.4	0.782	20.1	LOS C	8.3	59.1	0.68	1.10	38.6
West: N	1orphett	Street (W)									
10	L	469	1.8	0.317	8.3	LOS A	0.0	0.0	0.00	0.71	49.0
11	Т	86	0.0	0.317	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	556	1.5	0.317	7.0	NA	0.0	0.0	0.00	0.60	50.4
All Vehi	cles	1309	1.3	0.782	13.5	NA	8.3	59.1	0.38	0.85	43.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Do Nothing 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	21	0.0	0.222	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
2	Т	385	1.1	0.222	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	406	1.0	0.222	0.4	NA	0.0	0.0	0.00	0.05	59.3
North: Cowper S		Street (N)									
8	Т	240	1.8	0.184	2.3	LOS A	1.2	8.9	0.45	0.00	51.7
9	R	51	0.0	0.184	10.7	LOS B	1.2	8.9	0.45	0.94	48.0
Approac	ch	291	1.4	0.184	3.8	NA	1.2	8.9	0.45	0.16	51.0
West: N	lorphett	Street									
10	L	72	0.0	0.219	13.6	LOS B	0.9	6.0	0.57	0.77	43.6
12	R	42	0.0	0.219	13.8	LOS B	0.9	6.0	0.57	0.88	43.5
Approad	ch	114	0.0	0.219	13.7	LOS B	0.9	6.0	0.57	0.81	43.6
All Vehi	cles	811	1.0	0.222	3.5	NA	1.2	8.9	0.24	0.20	53.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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# APPENDIX C 2031 MASTERPLAN SIDRA RESULTS

#### **MOVEMENT SUMMARY**

Site: 1 AM

1- Northbourne Avenue - Antill Street - Mouat Street Master Plan 2031 AM Signals - Fixed Time Cycle Time = 110 seconds (Optimum Cycle Time - Minimum Delay)

Flow         Delay         Service         Vehicles         Distance         Queued         St           South: Northbourne Avenue (S)         1         1         1         112         9.4         0.127         12.2         LOS B         1.6         12.3         0.36           2         T         666         24.6         0.757         44.9         LOS D         15.2         124.3         0.97           3         R         134         4.7         1.236         289.8         LOS F         18.8         137.1         1.00           Approach         912         15.9         1.236         76.9         LOS E         18.8         137.1         0.90           East: Antill Street									
South: Northbourne Avenue (S)           1         L         112         9.4         0.127         12.2         LOS B         1.6         12.3         0.36           2         T         666         24.6         0.757         44.9         LOS D         15.2         124.3         0.97           3         R         134         4.7         1.236         289.8         LOS F         18.8         137.1         1.00           Approach         912         15.9         1.236         76.9         LOS E         18.8         137.1         0.90           East: Antill Street								Effective Stop Rate	Average Speed
1         L         112         9.4         0.127         12.2         LOS B         1.6         12.3         0.36           2         T         666         24.6         0.757         44.9         LOS D         15.2         124.3         0.97           3         R         134         4.7         1.236         289.8         LOS F         18.8         137.1         1.00           Approach         912         15.9         1.236         76.9         LOS E         18.8         137.1         0.90           East: Antill Street		sec	sec		veh	m		per veh	km/h
2         T         666         24.6         0.757         44.9         LOS D         15.2         124.3         0.97           3         R         134         4.7         1.236         289.8         LOS F         18.8         137.1         1.00           Approach         912         15.9         1.236         76.9         LOS E         18.8         137.1         0.90           East: Antill Street									
3         R         134         4.7         1.236         289.8         LOS F         18.8         137.1         1.00           Approach         912         15.9         1.236         76.9         LOS E         18.8         137.1         0.90           East: Antill Street         4         L         707         4.8         0.874         32.6         LOS C         27.2         198.4         0.96           5         T         592         6.4         1.149         122.6         LOS F         57.6         420.8         0.99           6         R         594         2.8         1.149         102.9         LOS F         57.6         420.8         0.98           North: Northbourne Avenue (N)         7         L         221         13.3         0.250         15.0         LOS B         4.7         36.3         0.46           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.45	L	12.2	12.2	LOS B	1.6	12.3	0.36	0.68	45.3
Approach         912         15.9         1.236         76.9         LOS E         18.8         137.1         0.90           East: Antill Street         4         L         707         4.8         0.874         32.6         LOS C         27.2         198.4         0.96           5         T         592         6.4         1.149         122.6         LOS F         57.6         420.8         0.99           6         R         594         2.8         1.149         102.9         LOS F         57.6         420.8         1.00           Approach         1893         4.7         1.149         82.8         LOS F         57.6         420.8         0.98           North: Northbourne Avenue (N)         7         L         221         13.3         0.250         15.0         LOS B         4.7         36.3         0.46           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453	L	44.9	44.9	LOS D	15.2	124.3	0.97	0.87	25.5
East: Antill Street         4       L       707       4.8       0.874       32.6       LOS C       27.2       198.4       0.96         5       T       592       6.4       1.149       122.6       LOS F       57.6       420.8       0.99         6       R       594       2.8       1.149       102.9       LOS F       57.6       420.8       1.00         Approach       1893       4.7       1.149       82.8       LOS F       57.6       420.8       0.98         North: Northbourne Avenue (N)       7       L       221       13.3       0.250       15.0       LOS B       4.7       36.3       0.46         8       T       1738       8.4       1.453       472.5       LOS F       110.1       826.1       1.00         9       R       125       1.1       1.127       195.2       LOS F       13.8       97.8       1.00         Approach       2084       8.3       1.453       407.3       LOS F       110.1       826.1       0.94         West: Mouat Street (W)       W       Kest       Kouat Street (W)       Kest       Kest       Kest       Kest       Kest       <	L	289.8	289.8	LOS F	18.8	137.1	1.00	1.57	6.8
4         L         707         4.8         0.874         32.6         LOS C         27.2         198.4         0.96           5         T         592         6.4         1.149         122.6         LOS F         57.6         420.8         0.99           6         R         594         2.8         1.149         102.9         LOS F         57.6         420.8         0.99           6         R         594         2.8         1.149         102.9         LOS F         57.6         420.8         0.98           Approach         1893         4.7         1.149         82.8         LOS F         57.6         420.8         0.98           North: Northbourne Avenue (N)         7         L         221         13.3         0.250         15.0         LOS B         4.7         36.3         0.46           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3	L	76.9	76.9	LOS E	18.8	137.1	0.90	0.95	18.8
5         T         592         6.4         1.149         122.6         LOS F         57.6         420.8         0.99           6         R         594         2.8         1.149         102.9         LOS F         57.6         420.8         1.00           Approach         1893         4.7         1.149         82.8         LOS F         57.6         420.8         0.98           North: Northbourne Avenue (N)         V         V         V         V         V         V           7         L         221         13.3         0.250         15.0         LOS B         4.7         36.3         0.46           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94									
6         R         594         2.8         1.149         102.9         LOS F         57.6         420.8         1.00           Approach         1893         4.7         1.149         82.8         LOS F         57.6         420.8         0.98           North: Northbourne Avenue (N)         7         L         221         13.3         0.250         15.0         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           9 proach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W)         5         110.1         826.1         0.94         0.94	L	32.6	32.6	LOS C	27.2	198.4	0.96	0.95	31.9
Approach         1893         4.7         1.149         82.8         LOS F         57.6         420.8         0.98           North: Northbourne Avenue (N)         7         L         221         13.3         0.250         15.0         LOS F         110.1         82.6.1         1.00           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W)           407.3         LOS F         110.1         826.1         0.94	L	122.6	122.6	LOS F	57.6	420.8	0.99	1.34	13.4
North: Northbourne Avenue (N)           7         L         221         13.3         0.250         15.0         LOS B         4.7         36.3         0.46           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W) <t< td=""><td>L</td><td>102.9</td><td>102.9</td><td>LOS F</td><td>57.6</td><td>420.8</td><td>1.00</td><td>1.17</td><td>16.0</td></t<>	L	102.9	102.9	LOS F	57.6	420.8	1.00	1.17	16.0
7         L         221         13.3         0.250         15.0         LOS B         4.7         36.3         0.46           8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W)	L	82.8	82.8	LOS F	57.6	420.8	0.98	1.14	18.3
8         T         1738         8.4         1.453         472.5         LOS F         110.1         826.1         1.00           9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W)              97.8         1.00									
9         R         125         1.1         1.127         195.2         LOS F         13.8         97.8         1.00           Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W)	L	15.0	15.0	LOS B	4.7	36.3	0.46	0.71	42.9
Approach         2084         8.3         1.453         407.3         LOS F         110.1         826.1         0.94           West: Mouat Street (W)	L	472.5	472.5	LOS F	110.1	826.1	1.00	2.75	4.3
West: Mouat Street (W)	L	195.2	195.2	LOS F	13.8	97.8	1.00	1.37	9.6
	L	407.3	407.3	LOS F	110.1	826.1	0.94	2.45	4.9
10 L 61 0.0 1.397 397.8 LOS F 93.8 677.6 1.00	L	397.8	397.8	LOS F	93.8	677.6	1.00	1.77	5.1
11 T 707 4.2 1.397 399.6 LOS F 95.0 691.3 1.00	L	399.6	399.6	LOS F	95.0	691.3	1.00	2.00	5.0
12 R 693 4.9 1.397 239.9 LOS F 95.0 691.3 1.00	L	239.9	239.9	LOS F	95.0	691.3	1.00	1.73	8.1
Approach         1461         4.3         1.397         323.8         LOS F         95.0         691.3         1.00	L	323.8	323.8	LOS F	95.0	691.3	1.00	1.86	6.1
All Vehicles 6349 7.4 1.453 243.9 LOS F 110.1 826.1 0.96	L	243.9	243.9	LOS F	110.1	826.1	0.96	1.71	7.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Movem	Movement Performance - Pedestrians												
		Demand	Average	Level of	Average Bad	ck of Queue	Prop.	Effective					
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	m		per ped					
P1	Across S approach	72	17.5	LOS B	0.1	0.1	0.56	0.56					
P2	Across S approach	72	37.6	LOS D	0.2	0.2	0.83	0.83					
P3	Across E approach	107	44.6	LOS E	0.3	0.3	0.90	0.90					
P5	Across N approach	47	17.5	LOS B	0.1	0.1	0.56	0.56					
P6	Across N approach	47	39.3	LOS D	0.1	0.1	0.85	0.85					
P7	Across W approach	16	44.6	LOS E	0.0	0.0	0.90	0.90					
All Pede	All Pedestrians		33.6	LOS D			0.77	0.77					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### Northbourne Avenue Service Road - Antill Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back of Queue		Prop.	Effective Stop Rate	Average		
		Flow			Delay	Service	Vehicles	Distance	Queued		Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
North West: Northbourne Avenue Service Road													
27	L	434	0.5	1.397	391.6	LOS F	85.8	603.0	1.00	5.30	5.1		
Approac	ch	434	0.5	1.397	391.6	LOS F	85.8	603.0	1.00	5.30	5.1		
West: A	ntill Stre	et (W)											
10	L	1	0.0	0.348	9.1	LOS A	0.0	0.0	0.00	1.19	48.1		
11	Т	1240	5.9	0.348	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approac	ch	1241	5.9	0.348	0.0	NA	0.0	0.0	0.00	0.00	60.0		
All Vehic	cles	1675	4.5	1.397	101.4	NA	85.8	603.0	0.26	1.37	15.8		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Challis S	treet (S)									
1	L	497	9.3	2.478	1367.0	LOS F	194.6	1470.8	1.00	7.31	1.6
Approad	ch	497	9.3	2.478	1367.0	LOS F	194.6	1470.8	1.00	7.31	1.6
East: Ar	ntil Stree	t (W)									
4	L	93	20.5	0.147	9.8	LOS A	0.4	3.3	0.33	0.64	48.0
5	Т	1398	3.0	0.385	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	1491	4.1	0.385	0.6	NA	0.4	3.3	0.02	0.04	59.1
West: A	ntill Stre	et									
11	Т	1469	1.2	0.408	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	207	15.0	1.023	109.3	LOS F	12.4	97.8	1.00	1.97	15.0
Approach 16		1676	4.5	1.023	13.5	NA	12.4	97.8	0.12	0.24	43.8
All Vehi	cles	3663	5.0	2.478	191.8	NA	194.6	1470.8	0.20	1.12	9.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 AM Stop (Two-Way)

Movement Performance - Vehicles											
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Badham	Street (S)									
1	L	562	0.0	2.130	1050.2	LOS F	219.4	1553.2	1.00	8.89	2.0
3	R	69	12.1	2.130	1050.6	LOS F	219.4	1553.2	1.00	7.38	2.0
Approa	ch	632	1.3	2.130	1050.3	LOS F	219.4	1553.2	1.00	8.72	2.0
East: A	East: Antill Street (W)										
4	L	93	0.0	0.108	10.2	LOS B	0.4	2.7	0.43	0.70	47.0
5	Т	1006	6.1	0.283	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1099	5.6	0.283	0.9	NA	0.4	2.7	0.04	0.06	58.6
West: A	Antill Stre	et (W)									
11	Т	705	0.0	0.381	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	672	3.1	1.335	326.8	LOS F	115.0	826.8	1.00	6.46	6.0
Approa	ch	1377	1.5	1.335	159.4	NA	115.0	826.8	0.49	3.15	11.2
South West: Median (RT Stage 2)											
32	R	69	12.1	0.129	13.5	LOS B	0.5	3.1	0.60	0.86	28.0
Approach		69	12.1	0.129	13.5	LOS B	0.5	3.1	0.60	0.86	28.0
All Vehi	icles	3177	3.1	2.130	278.5	NA	219.4	1553.2	0.44	3.14	6.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Master Plan 2031 AM Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back of Queue		Prop.	Effective	Average	
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South: Dickson Shop Access Road												
1	L	162	0.0	0.369	16.8	LOS C	1.7	11.8	0.73	0.96	41.2	
Approa	ch	162	0.0	0.369	16.8	LOS C	1.7	11.8	0.73	0.96	41.2	
East: A	ntill Stree	et (W)										
4	L	1	0.0	0.265	8.2	LOS A	0.0	0.0	0.00	1.09	49.0	
5	Т	941	6.5	0.265	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Approa	ch	942	6.5	0.265	0.0	NA	0.0	0.0	0.00	0.00	60.0	
All Vehi	icles	1104	5.5	0.369	2.5	NA	1.7	11.8	0.11	0.14	56.2	

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Master Plan 2031 AM

Signals - Fixed Time Cycle Time = 70 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	366	0.0	0.381	17.6	LOS B	7.0	50.5	0.60	0.80	40.5
<mark>3</mark>	R	<mark>405</mark>	5.2	<mark>1.000</mark> 3	34.2	LOS C	13.4	97.9	1.00	0.86	31.1
Approa	ch	771	4.1	1.000	26.3	LOS C	13.4	97.9	0.81	0.83	34.9
East: A	ntill Stree	et (E)									
<mark>4</mark>	L	<mark>499</mark>	1.3	<mark>1.000</mark> 3	38.3	LOS D	18.2	128.9	1.00	0.89	29.2
5	Т	929	7.8	0.973	60.4	LOS E	24.9	185.9	1.00	1.34	21.6
Approa	ch	1427	4.9	1.000	52.6	LOS D	24.9	185.9	1.00	1.18	23.8
West: A	ntill Stre	et (W)									
11	Т	562	1.5	0.565	24.3	LOS C	8.5	60.2	0.91	0.77	34.2
12	R	219	0.0	0.669	38.8	LOS D	7.5	52.2	0.98	0.85	29.1
Approa	ch	781	1.1	0.669	28.3	LOS C	8.5	60.2	0.93	0.79	32.6
All Vehi	cles	2979	3.7	1.000	39.5	LOS D	24.9	185.9	0.93	0.99	28.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movem	nent Performance -	Pedestrian	าร					
Mov	Description	Demand	Average	Level of			Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	29.3	LOS C	0.1	0.1	0.91	0.91
P3	Across E approach	107	29.3	LOS C	0.2	0.2	0.91	0.91
P4	Across E approach	33	26.6	LOS C	0.1	0.1	0.87	0.87
All Pede	estrians	212	28.8	LOS C			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Master Plan 2031 AM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
			%	v/c					Queucu		
	-	veh/h	70	V/C	sec		veh	m		per veh	km/h
South: F	Rosevea	r Place (S)									
1	L	74	0.0	0.443	35.8	LOS E	1.6	11.0	0.92	1.04	30.2
3	R	1	0.0	0.443	35.4	LOS E	1.6	11.0	0.92	1.03	30.3
Approac	ch	75	0.0	0.443	35.8	LOS E	1.6	11.0	0.92	1.04	30.2
East: Ar	ntill Stree	et (E)									
4	L	51	0.0	0.381	8.2	LOS A	0.0	0.0	0.00	1.04	49.0
5	Т	1320	4.1	0.381	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	1371	4.0	0.381	0.3	NA	0.0	0.0	0.00	0.04	59.5
West: A	ntill Stre	et									
11	Т	882	3.1	0.394	4.6	LOS A	3.5	25.2	0.19	0.00	52.1
12	R	72	0.0	0.394	32.5	LOS D	3.5	25.2	1.00	1.06	33.1
Approac	ch	954	2.9	0.394	6.7	NA	3.5	25.2	0.25	0.08	49.9
South W	Vest: Me	dian (RT Sta	ge 2)								
32	R	1	0.0	0.005	21.3	LOS C	0.0	0.1	0.75	0.78	20.9
Approac	ch	1	0.0	0.005	21.3	LOS C	0.0	0.1	0.75	0.78	20.9
All Vehi	cles	2400	3.4	0.443	4.0	NA	3.5	25.2	0.13	0.09	53.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Master Plan 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	280	0.0	0.448	9.5	LOS A	5.1	35.4	1.00	0.00	42.5
3	R	168	0.0	0.448	17.9	LOS C	5.1	35.4	1.00	1.12	42.3
Approad	ch	448	0.0	0.448	12.7	NA	5.1	35.4	1.00	0.42	42.4
East: Di	ickson S	hop Access I	Road (E)								
4	L	316	0.0	3.790	2555.6	LOS F	304.2	2129.6	1.00	8.35	0.8
6	R	316	0.0	3.790	2555.8	LOS F	304.2	2129.6	1.00	6.97	0.8
Approad	ch	632	0.0	3.790	2555.7	LOS F	304.2	2129.6	1.00	7.66	0.8
North: E	Badham	Street (N)									
7	L	122	0.0	0.408	8.2	LOS A	0.0	0.0	0.00	0.98	49.0
8	Т	613	3.4	0.408	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	735	2.9	0.408	1.4	NA	0.0	0.0	0.00	0.16	57.8
All Vehi	cles	1815	1.2	3.790	893.1	NA	304.2	2129.6	0.60	2.84	2.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Master Plan 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	101	0.0	0.377	8.2	LOS A	0.0	0.0	0.00	0.99	49.0
2	Т	566	6.7	0.377	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	667	5.7	0.377	1.2	NA	0.0	0.0	0.00	0.15	58.0
North: C	Cowper S	Street (N)									
8	Т	731	1.2	0.588	11.4	LOS B	10.8	76.3	1.00	0.00	41.8
9	R	133	0.0	0.588	19.8	LOS C	10.8	76.3	1.00	1.18	41.7
Approad	ch	863	1.0	0.588	12.7	NA	10.8	76.3	1.00	0.18	41.8
West: D	ickson S	Shop Access	Road (W	)							
10	L	221	0.0	0.939	71.8	LOS F	13.2	92.2	0.88	2.00	20.2
12	R	42	0.0	0.939	72.3	LOS F	13.2	92.2	0.88	1.72	20.1
Approad	ch	263	0.0	0.939	71.8	LOS F	13.2	92.2	0.88	1.96	20.2
All Vehi	cles	1794	2.6	0.939	17.1	NA	13.2	92.2	0.61	0.43	39.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	38	0.0	0.229	8.2	LOS A	0.0	0.0	0.00	1.03	49.0
2	Т	383	0.0	0.229	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	421	0.0	0.229	0.7	NA	0.0	0.0	0.00	0.09	58.8
North: B	adham	Street (N)									
8	Т	855	2.5	0.552	6.0	LOS A	10.8	76.8	0.91	0.00	45.7
9	R	76	0.0	0.552	14.4	LOS B	10.8	76.8	0.91	1.13	46.6
Approac	h	931	2.3	0.552	6.7	NA	10.8	76.8	0.91	0.09	45.7
West: W	oolley S	Street									
10	L	82	0.0	0.868	96.6	LOS F	6.6	46.4	0.87	1.63	16.4
12	R	42	0.0	0.868	96.9	LOS F	6.6	46.4	0.87	1.38	16.3
Approac	h	124	0.0	0.868	96.7	LOS F	6.6	46.4	0.87	1.55	16.3
All Vehic	cles	1476	1.4	0.868	12.5	NA	10.8	76.8	0.65	0.21	42.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Master Plan 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	1	0.0	0.002	1.6	LOS A	0.0	0.1	0.41	0.00	51.4
3	R	1	0.0	0.002	10.1	LOS B	0.0	0.1	0.41	0.73	47.9
Approac	h	2	0.0	0.002	5.8	NA	0.0	0.1	0.41	0.36	49.6
East: Di	ckson P	lace									
4	L	1	0.0	0.204	10.4	LOS B	0.9	6.1	0.41	0.65	46.4
6	R	137	0.0	0.204	10.7	LOS B	0.9	6.1	0.41	0.72	46.3
Approac	h	138	0.0	0.204	10.7	LOS B	0.9	6.1	0.41	0.72	46.3
North: B	adham	Street (N)									
7	L	383	5.5	0.227	8.4	LOS A	0.0	0.0	0.00	0.67	49.0
8	Т	1	0.0	0.227	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	384	5.5	0.227	8.4	NA	0.0	0.0	0.00	0.67	49.0
All Vehic	cles	524	4.0	0.227	9.0	NA	0.9	6.1	0.11	0.68	48.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Master Plan 2031 AM

Signals - Fixed Time Cycle Time = 130 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	e - Vehio	cles							
Mov ID	Turn	Demand	HV C	Deg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street (S)									
1	L	251	0.0	1.144	214.3	LOS F	86.9	643.6	1.00	1.66	8.8
2	Т	408	10.8	1.144	206.2	LOS F	86.9	643.6	1.00	1.66	8.8
Approad	ch	659	6.7	1.144	209.3	LOS F	86.9	643.6	1.00	1.66	8.8
North: C	Cowper S	Street (N)									
8	Т	735	2.6	0.984	82.6	LOS F	65.0	475.1	1.00	1.19	17.6
<mark>9</mark>	R	<mark>131</mark>	13.2	<mark>1.000</mark> <sup>3</sup>	50.4	LOS D	5.7	44.5	1.00	0.80	25.2
Approad	ch	865	6.3	1.000	77.8	LOS E	65.0	475.1	1.00	1.13	18.5
West: D	ickson F	Place (W)									
10	L	328	14.7	1.138	206.9	LOS F	103.2	760.8	1.00	1.35	9.0
12	R	455	0.0	1.138	206.4	LOS F	103.2	760.8	1.00	1.35	8.9
Approad	ch	783	6.2	1.138	206.6	LOS F	103.2	760.8	1.00	1.35	9.0
All Vehi	cles	2307	6.4	1.144	159.1	LOS F	103.2	760.8	1.00	1.36	11.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movem	ent Performance -	Pedestriar	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service			Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	72	12.5	LOS B	0.1	0.1	0.44	0.44
P2	Across S approach	72	59.1	LOS E	0.3	0.3	0.95	0.95
P7	Across W approach	16	15.3	LOS B	0.0	0.0	0.48	0.48
P8	Across W approach	38	59.1	LOS E	0.1	0.1	0.95	0.95
All Pede	estrians	198	38.6	LOS D			0.73	0.73

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Movor	nont Po	rformance	- Vohi	eloc							
					Average		OE% Deels	of Output	Drog	Effective	Average
Mov ID	Turn	Demand Flow	HVI	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec	0011100	venicies		Quouou		km/h
Coutbu	Challia C		70	V/C	Sec		ven	m		per veh	KIII/11
	Challis S	. ,		0.054	10.0	100 5	10.0		0.00	0.40	10.0
1	L	40	0.0	0.651	13.2	LOS B	13.8	97.9	0.89	0.10	46.3
2	Т	701	2.4	0.651	5.0	LOS A	13.8	97.9	0.89	0.00	44.9
3	R	267	0.0	0.651	13.4	LOS B	13.8	97.9	0.89	0.99	46.4
Approa	ch	1008	1.7	0.651	7.5	NA	13.8	97.9	0.89	0.27	45.4
East: C	ape Stre	et									
4	L	269	0.0	10.183	8329.8	LOS F	607.5	4387.9	1.00	9.96	0.3
5	Т	699	0.0	10.183	8328.5	LOS F	607.5	4387.9	1.00	5.61	0.3
6	R	51	75.0	10.183	8333.1	LOS F	607.5	4387.9	1.00	5.13	0.3
Approa	ch	1019	3.7	10.183	8329.1	LOS F	607.5	4387.9	1.00	6.74	0.3
North: 0	Challis St	treet									
7	L	42	100.0	0.444	30.3	LOS D	5.6	47.2	1.00	0.00	36.8
8	Т	194	1.1	0.444	18.4	LOS C	5.6	47.2	1.00	0.00	36.1
9	R	99	31.9	0.444	28.1	LOS D	5.6	47.2	1.00	1.15	35.9
Approa	ch	335	22.6	0.444	22.8	NA	5.6	47.2	1.00	0.34	36.2
West: C	Cape Stre	eet									
10	L	55	0.0	4.488	3202.2	LOS F	164.4	1150.6	1.00	5.80	0.7
11	Т	1	0.0	4.488	3201.0	LOS F	164.4	1150.6	1.00	4.94	0.7
12	R	261	0.0	4.488	3202.5	LOS F	164.4	1150.6	1.00	4.45	0.7
Approa	ch	317	0.0	4.488	3202.4	LOS F	164.4	1150.6	1.00	4.69	0.7
All Vehi	icles	2679	4.9	10.183	3552.4	NA	607.5	4387.9	0.96	3.26	0.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	939	4.0	0.522	4.1	LOS A	9.0	65.1	0.76	0.00	47.9
6	R	1	0.0	0.522	12.5	LOS B	9.0	65.1	0.76	1.05	48.2
Approa	ch	940	4.0	0.522	4.1	NA	9.0	65.1	0.76	0.00	47.9
North: V	Voolley S	Street									
7	L	1	0.0	1.052	218.7	LOS F	8.6	60.4	1.00	1.85	8.5
9	R	76	0.0	1.052	218.9	LOS F	8.6	60.4	1.00	1.57	8.5
Approa	ch	77	0.0	1.052	218.9	LOS F	8.6	60.4	1.00	1.58	8.5
West: C	Cape Stre	eet (W)									
10	L	48	0.0	0.182	8.2	LOS A	0.0	0.0	0.00	0.99	49.0
11	Т	259	16.3	0.182	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	307	13.7	0.182	1.3	NA	0.0	0.0	0.00	0.16	57.9
All Vehi	cles	1324	6.0	1.052	15.9	NA	9.0	65.1	0.60	0.13	39.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street									
2	Т	564	7.8	0.527	21.1	LOS C	10.3	76.8	1.00	0.00	35.4
3	R	84	2.5	0.527	29.6	LOS D	10.3	76.8	1.00	1.22	35.2
Approad	ch	648	7.1	0.527	22.2	NA	10.3	76.8	1.00	0.16	35.4
East: Da	avenport	Street (E)									
4	L	17	0.0	1.696	728.5	LOS F	35.4	247.5	1.00	3.12	2.8
6	R	99	0.0	1.696	728.7	LOS F	35.4	247.5	1.00	2.68	2.8
Approad	ch	116	0.0	1.696	728.7	LOS F	35.4	247.5	1.00	2.75	2.8
North: C	Cowper S	Street									
7	L	152	0.0	0.559	8.2	LOS A	0.0	0.0	0.00	0.99	49.0
8	Т	865	1.7	0.559	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	1017	1.4	0.559	1.2	NA	0.0	0.0	0.00	0.15	58.1
All Vehi	cles	1781	3.4	1.696	56.1	NA	35.4	247.5	0.43	0.32	23.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	rne Avenue	(S)								
2	Т	1330	13.7	0.272	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	101	3.0	1.690	736.8	LOS F	31.1	223.0	1.00	2.52	2.9
Approa	ch	1432	9.4	1.690	52.2	NA	31.1	223.0	0.07	0.18	19.2
East: M	lorphett S	Street (E)									
4	L	162	3.9	2.702	1617.8	LOS F	70.9	513.0	1.00	3.38	1.3
Approa	ch	162	3.9	2.702	1617.8	LOS F	70.9	513.0	1.00	3.38	1.3
North: N	Northbou	rne Avenue	(N)								
7	L	99	2.1	0.576	8.3	LOS A	0.0	0.0	0.00	1.03	49.0
8	Т	2966	6.5	0.576	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	3065	6.3	0.576	0.3	NA	0.0	0.0	0.00	0.03	59.6
All Vehi	icles	4659	7.2	2.702	72.5	NA	70.9	513.0	0.06	0.19	18.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Mo	orphett S	Street (E)									
5	Т	141	4.5	0.780	12.8	LOS B	9.0	63.5	1.00	0.00	38.7
6	R	444	0.0	0.780	21.1	LOS C	9.0	63.5	1.00	1.39	38.5
Approac	h	585	1.1	0.780	19.1	NA	9.0	63.5	1.00	1.06	38.6
North: C	hallis S	treet									
7	L	764	0.3	1.075	98.4	LOS F	57.0	399.6	1.00	3.50	16.2
9	R	21	0.0	0.163	36.4	LOS E	0.5	3.6	0.88	0.97	29.8
Approac	h	785	0.3	1.075	96.7	LOS F	57.0	399.6	1.00	3.43	16.4
West: M	orphett	Street (W)									
10	L	461	3.7	0.383	8.3	LOS A	0.0	0.0	0.00	0.76	49.0
11	Т	211	1.0	0.383	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	672	2.8	0.383	5.7	NA	0.0	0.0	0.00	0.52	52.0
All Vehic	cles	2042	1.3	1.075	44.6	NA	57.0	399.6	0.67	1.79	26.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Mov ID Turn	Demand Flow	HV D	an Cala							
	Flow		eg. Sath	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
	1 1011			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
	veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Cowper St	reet (S)									
1 L	234	0.0	0.422	8.2	LOS A	0.0	0.0	0.00	0.90	49.0
2 T	503	10.0	0.422	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approach	737	6.9	0.422	2.6	NA	0.0	0.0	0.00	0.29	56.0
North: Cowper Str	reet (N)									
8 T	541	1.6	0.830	24.5	LOS C	14.8	105.0	1.00	0.00	32.9
9 R	322	2.0	0.830	33.0	LOS D	14.8	105.0	1.00	1.51	32.7
Approach	863	1.7	0.830	27.7	NA	14.8	105.0	1.00	0.56	32.8
West: Morphett St	treet									
10 L	160	1.3	5.233	3884.3	LOS F	247.4	1766.0	1.00	6.33	0.6
12 R	297	2.8	5.233	3884.7	LOS F	247.4	1766.0	1.00	4.82	0.6
Approach	457	2.3	5.233	3884.6	LOS F	247.4	1766.0	1.00	5.35	0.6
All Vehicles	2057	3.7	5.233	875.3	NA	247.4	1766.0	0.64	1.53	2.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northborne Avenue - Cape Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: C	ape Stre	et									
4	L	840	3.8	1.974	900.2	LOS F	270.0	1951.0	1.00	10.03	2.3
Approa	ch	840	3.8	1.974	900.2	LOS F	270.0	1951.0	1.00	10.03	2.3
North: I	Northbou	rn Avenue									
7	L	314	0.0	0.178	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
8	Т	724	14.1	0.427	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1038	9.8	0.427	2.5	NA	0.0	0.0	0.00	0.20	56.2
All Veh	icles	1878	7.1	1.974	404.0	NA	270.0	1951.0	0.45	4.60	4.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Badham Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et									
5	Т	497	7.6	0.375	8.3	LOS A	7.4	55.1	0.91	0.00	45.1
6	R	46	0.0	0.375	16.8	LOS C	7.4	55.1	0.91	1.07	44.3
Approac	:h	543	7.0	0.375	9.1	NA	7.4	55.1	0.91	0.09	45.1
North: B	adham	Street									
7	L	196	0.0	0.938	33.9	LOS D	21.8	152.5	0.77	1.73	30.9
9	R	484	0.0	0.938	34.1	LOS D	21.8	152.5	0.77	1.78	30.9
Approac	:h	680	0.0	0.938	34.1	LOS D	21.8	152.5	0.77	1.77	30.9
West: Ca	ape Stre	eet									
10	L	91	0.0	0.157	8.2	LOS A	0.0	0.0	0.00	0.89	49.0
11	Т	168	25.0	0.157	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	259	16.3	0.157	2.9	NA	0.0	0.0	0.00	0.31	55.6
All Vehic	cles	1482	5.4	0.938	19.5	NA	21.8	152.5	0.68	0.90	38.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Dickson Place - Cape Street Master Plan 2031 AM Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cape Str	eet									
1	L	1	0.0	2.866	1713.0	LOS F	287.7	2148.5	1.00	10.14	1.2
3	R	674	7.8	2.866	1713.6	LOS F	287.7	2148.5	1.00	8.19	1.2
Approa	ch	675	7.8	2.866	1713.6	LOS F	287.7	2148.5	1.00	8.19	1.2
East: Di	ickson P	lace									
4	L	421	9.5	0.329	8.5	LOS A	0.0	0.0	0.00	0.74	49.0
5	Т	137	0.0	0.329	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	558	7.2	0.329	6.4	NA	0.0	0.0	0.00	0.56	51.3
West: D	Dickson F	lace									
11	Т	133	0.0	0.835	31.8	LOS D	11.5	84.5	1.00	0.00	29.1
12	R	251	8.4	0.835	40.6	LOS E	11.5	84.5	1.00	1.53	28.9
Approa	ch	383	5.5	0.835	37.5	NA	11.5	84.5	1.00	1.00	29.0
All Vehi	cles	1616	7.0	2.866	726.7	NA	287.7	2148.5	0.65	3.85	2.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street Master Plan 2031 PM Signals - Fixed Time Cycle Time = 125 seconds (Optimum Cycle Time - Minimum Delay)

Mover	ment Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
1	L	509	4.1	0.643	17.0	LOS B	13.7	99.1	0.56	0.76	41.1
2	Т	1826	4.6	1.370	343.8	LOS F	140.5	1022.0	0.98	2.28	5.7
3	R	130	7.4	1.211	276.7	LOS F	18.4	137.1	1.00	1.49	7.1
Approa	ich	2465	4.7	1.370	272.7	LOS F	140.5	1022.0	0.89	1.92	7.0
East: A	ntill Stree	et									
4	L	333	0.0	0.567	18.4	LOS B	9.2	64.7	0.56	0.75	40.0
5	Т	855	4.7	1.366	243.8	LOS F	108.6	792.4	1.00	1.80	7.7
6	R	486	5.6	1.366	158.9	LOS F	108.6	792.4	1.00	1.33	11.4
Approa	ich	1674	4.0	1.366	174.3	LOS F	108.6	792.4	0.91	1.45	10.4
North: I	Northbou	rne Avenue	(N)								
7	L	225	7.5	0.234	13.3	LOS B	4.3	32.1	0.38	0.69	44.2
8	Т	890	5.1	0.532	38.2	LOS D	14.9	109.1	0.88	0.75	28.0
9	R	125	0.0	1.106	186.1	LOS F	14.0	97.9	1.00	1.30	10.0
Approa	ich	1240	4.8	1.106	48.5	LOS D	14.9	109.1	0.80	0.80	25.1
West: N	Mouat Str	reet (W)									
10	L	236	0.9	1.203	227.8	LOS F	54.3	390.3	1.00	1.44	8.4
11	Т	507	5.8	1.203	245.5	LOS F	54.3	390.3	1.00	1.66	7.6
12	R	282	1.5	1.065	123.7	LOS F	25.3	179.5	1.00	1.21	14.0
Approa	ich	1025	3.5	1.203	207.9	LOS F	54.3	390.3	1.00	1.48	8.9
All Veh	icles	6404	4.3	1.370	193.2	LOS F	140.5	1022.0	0.90	1.51	9.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Movem	nent Performance -	Pedestrian	IS					
		Demand	Average	Level of	Average Bag	ck of Queue	Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	45	23.7	LOS C	0.1	0.1	0.62	0.62
P2	Across S approach	45	40.8	LOS E	0.1	0.1	0.81	0.81
P3	Across E approach	39	40.0	LOS D	0.1	0.1	0.80	0.80
P5	Across N approach	35	23.7	LOS C	0.1	0.1	0.62	0.62
P6	Across N approach	35	51.1	LOS E	0.1	0.1	0.90	0.90
P7	Across W approach	5	40.0	LOS D	0.0	0.0	0.80	0.80
All Pede	estrians	204	35.7	LOS D			0.75	0.75

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### Northbourne Avenue Service Road - Antill Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North V	Vest: Nor	thbourne Ave	enue Serv	/ice Road							
27	L	23	0.0	0.049	13.8	LOS B	0.2	1.2	0.62	0.84	43.4
Approa	ch	23	0.0	0.049	13.8	LOS B	0.2	1.2	0.62	0.84	43.4
West: A	Antill Stre	et (W)									
10	L	25	33.3	0.266	10.2	LOS B	0.0	0.0	0.00	1.19	48.1
11	Т	918	5.7	0.266	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	943	6.5	0.266	0.3	NA	0.0	0.0	0.00	0.03	59.6
All Vehi	icles	966	6.3	0.266	0.6	NA	0.2	1.2	0.01	0.05	59.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Challis Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Mov ID         Turn         Demand         HV Deg. Satn         Average         Level of         95% Back of Queue           Flow         Delay         Service         Vehicles         Distance           veh/h         %         v/c         sec         veh         m           South:         Challis Street (S)         Sec         Sec         Sec         Sec	Queued Sto	fective Averag p Rate Spee
	p	orvob km
South: Challis Street (S)		ber veh km/
1 L 280 24.8 2.031 979.7 LOS F 96.5 819.2	1.00	5.09 2.
Approach 280 24.8 2.031 979.7 LOS F 96.5 819.2	1.00	5.09 2.
East: Antil Street (W)		
4 L 21 80.0 0.058 11.5 LOS B 0.1 1.5	0.27	0.58 48.
5 T 1413 0.0 0.382 0.0 LOS A 0.0 0.0	0.00	0.00 60.
Approach 1434 1.2 0.382 0.2 NA 0.1 1.5	0.00	0.01 59.
West: Antill Street		
11 T 857 1.2 0.233 0.0 LOS A 0.0 0.0	0.00	0.00 60.
12 R 86 48.8 1.068 208.2 LOS F 8.8 87.4	1.00	1.70 9.
Approach 943 5.6 1.068 19.1 NA 8.8 87.4	0.09	0.16 39.
All Vehicles 2657 5.2 2.031 110.1 NA 96.5 819.2	0.14	0.60 14.

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 PM Stop (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	653	0.0	1.752	705.0	LOS F	206.3	1458.1	1.00	8.93	2.9
3	R	76	11.1	1.752	705.2	LOS F	206.3	1458.1	1.00	7.51	2.9
Approa	ch	728	1.2	1.752	705.0	LOS F	206.3	1458.1	1.00	8.78	2.9
East: Ar	ntill Stree	et (W)									
4	L	76	0.0	0.083	9.8	LOS A	0.3	2.1	0.41	0.67	47.4
5	Т	861	2.0	0.236	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	937	1.8	0.236	0.8	NA	0.3	2.1	0.03	0.05	58.7
West: A	ntill Stre	et (W)									
11	Т	537	1.6	0.293	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	455	0.5	0.741	19.9	LOS C	6.0	42.2	0.84	1.24	38.8
Approa	ch	992	1.1	0.741	9.1	NA	6.0	42.2	0.38	0.57	48.0
South V	Vest: Me	dian (RT Sta	ige 2)								
32	R	76	11.1	0.108	11.5	LOS B	0.4	2.7	0.52	0.78	30.5
Approa	ch	76	11.1	0.108	11.5	LOS B	0.4	2.7	0.52	0.78	30.5
All Vehi	cles	2733	1.6	1.752	191.8	NA	206.3	1458.1	0.43	2.59	9.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Master Plan 2031 PM Giveway / Yield (Two-Way)

							les	e - Vehio	erformance	nent Pe	Moven
Average	Effective	Prop.	of Queue	95% Back	Level of	Average	eg. Satn	HV C	Demand	Turn	Mov ID
Speed	Stop Rate	Queued	Distance	Vehicles	Service	Delay			Flow		
km/h	per veh		m	veh		sec	v/c	%	veh/h		
								s Road	Shop Acces	Dickson	South: I
43.8	0.90	0.63	11.7	1.7	LOS B	13.6	0.347	0.0	200	L	1
43.8	0.90	0.63	11.7	1.7	LOS B	13.6	0.347	0.0	200	ch	Approa
									et (W)	ntill Stree	East: A
49.0	1.09	0.00	0.0	0.0	LOS A	8.2	0.202	0.0	1	L	4
60.0	0.00	0.00	0.0	0.0	LOS A	0.0	0.202	2.3	737	Т	5
60.0	0.00	0.00	0.0	0.0	NA	0.0	0.202	2.3	738	ch	Approa
55.6	0.19	0.13	11.7	1.7	NA	2.9	0.347	1.8	938	icles	All Vehi
	0.00	0.00	0.0	0.0	LOS A NA	0.0	0.202	2.3 2.3	1 737 738	L T ch	4 5 Approad

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Master Plan 2031 PM Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper S	Street (S)									
1	L	322	0.0	0.549	26.3	LOS C	7.9	55.5	0.87	0.83	34.9
<mark>3</mark>	R	<mark>493</mark>	1.1	<mark>1.000</mark> 3	28.6	LOS C	13.9	97.9	0.99	0.87	33.7
Approac	ch	815	0.8	1.000	27.7	LOS C	13.9	97.9	0.94	0.85	34.1
East: Ar	ntill Stree	et (E)									
4	L	192	1.1	0.329	24.7	LOS C	4.3	30.4	0.79	0.79	35.7
5	Т	493	3.4	0.907	39.7	LOS D	9.2	66.5	1.00	1.12	27.4
Approac	ch	684	2.8	0.907	35.5	LOS D	9.2	66.5	0.94	1.03	29.3
West: A	ntill Stre	et (W)									
11	Т	448	2.8	0.823	32.8	LOS C	7.5	53.6	1.00	0.98	30.0
12	R	141	3.0	0.377	30.7	LOS C	3.7	26.4	0.90	0.79	32.6
Approac	ch	589	2.9	0.823	32.3	LOS C	7.5	53.6	0.98	0.94	30.6
All Vehi	cles	2088	2.0	1.000	31.6	LOS C	13.9	97.9	0.95	0.93	31.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movem	nent Performance -	Pedestrian	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bao Pedestrian		Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	5	16.9	LOS B	0.0	0.0	0.75	0.75
P3	Across E approach	13	24.3	LOS C	0.0	0.0	0.90	0.90
P4	Across E approach	13	21.7	LOS C	0.0	0.0	0.85	0.85
All Pede	estrians	31	22.0	LOS C			0.85	0.85

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Master Plan 2031 PM Giveway / Yield (Two-Way)

Movem	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: F	Rosevea	r Place (S)									
1	L	78	0.0	0.278	15.4	LOS C	1.2	8.1	0.64	0.91	42.1
3	R	48	0.0	0.278	15.0	LOS C	1.2	8.1	0.64	0.90	42.3
Approac	h	126	0.0	0.278	15.3	LOS C	1.2	8.1	0.64	0.91	42.2
East: Ar	ntill Stree	et (E)									
4	L	25	0.0	0.163	8.2	LOS A	0.0	0.0	0.00	1.03	49.0
5	Т	566	3.0	0.163	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	592	2.8	0.163	0.3	NA	0.0	0.0	0.00	0.04	59.4
West: A	ntill Stre	et									
11	Т	678	2.5	0.241	1.4	LOS A	1.7	12.3	0.21	0.00	55.6
12	R	84	0.0	0.241	12.1	LOS B	1.7	12.3	0.61	0.95	46.8
Approac	:h	762	2.2	0.241	2.6	NA	1.7	12.3	0.26	0.10	54.5
South W	/est: Me	dian (RT Sta	ge 2)								
32	R	48	0.0	0.145	17.1	LOS C	0.5	3.2	0.67	0.89	24.0
Approac	:h	48	0.0	0.145	17.1	LOS C	0.5	3.2	0.67	0.89	24.0
All Vehic	cles	1528	2.2	0.278	3.2	NA	1.7	12.3	0.20	0.17	54.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Master Plan 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	488	0.0	0.616	8.7	LOS A	9.9	69.1	1.00	0.00	43.2
3	R	278	0.0	0.616	17.1	LOS C	9.9	69.1	1.00	1.21	43.1
Approad	ch	766	0.0	0.616	11.7	NA	9.9	69.1	1.00	0.44	43.1
East: Di	East: Dickson Shop Acc		Road (E)								
4	L	120	0.0	1.853	838.7	LOS F	78.9	552.5	1.00	5.60	2.5
6	R	126	0.0	1.853	839.0	LOS F	78.9	552.5	1.00	4.09	2.5
Approad	ch	246	0.0	1.853	838.9	LOS F	78.9	552.5	1.00	4.83	2.5
North: E	Badham	Street (N)									
7	L	133	0.0	0.290	8.2	LOS A	0.0	0.0	0.00	0.93	49.0
8	Т	396	0.5	0.290	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	528	0.4	0.290	2.1	NA	0.0	0.0	0.00	0.23	56.8
All Vehi	cles	1541	0.1	1.853	140.6	NA	78.9	552.5	0.66	1.07	12.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Master Plan 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street (S)									
1	L	135	0.0	0.404	8.2	LOS A	0.0	0.0	0.00	0.97	49.0
2	Т	602	1.0	0.404	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	737	0.9	0.404	1.5	NA	0.0	0.0	0.00	0.18	57.6
North: C	Cowper S	Street (N)									
8	Т	246	2.6	0.228	6.1	LOS A	2.0	14.3	0.72	0.00	47.6
9	R	59	0.0	0.228	14.5	LOS B	2.0	14.3	0.72	1.02	45.2
Approa	ch	305	2.1	0.228	7.7	NA	2.0	14.3	0.72	0.20	47.1
West: D	ickson S	Shop Access	Road (W	)							
10	L	213	0.0	0.565	20.2	LOS C	3.6	24.9	0.76	1.08	38.6
12	R	38	0.0	0.565	20.8	LOS C	3.6	24.9	0.76	1.09	38.5
Approa	ch	251	0.0	0.565	20.3	LOS C	3.6	24.9	0.76	1.09	38.5
All Vehi	cles	1293	1.0	0.565	6.6	NA	3.6	24.9	0.32	0.36	50.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	143	0.0	0.381	8.2	LOS A	0.0	0.0	0.00	0.96	49.0
2	Т	554	0.0	0.381	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	h	697	0.0	0.381	1.7	NA	0.0	0.0	0.00	0.20	57.3
North: B	adham	Street (N)									
8	Т	335	0.6	0.580	11.0	LOS B	7.0	49.1	1.00	0.00	41.1
9	R	242	0.0	0.580	19.4	LOS C	7.0	49.1	1.00	1.19	40.9
Approac	h	577	0.4	0.580	14.5	NA	7.0	49.1	1.00	0.50	41.0
West: W	oolley S	Street									
10	L	215	0.0	1.263	295.0	LOS F	45.9	321.1	1.00	4.08	6.6
12	R	69	0.0	1.263	295.3	LOS F	45.9	321.1	1.00	3.38	6.5
Approac	h	284	0.0	1.263	295.1	LOS F	45.9	321.1	1.00	3.91	6.6
All Vehic	cles	1558	0.1	1.263	60.0	NA	45.9	321.1	0.55	0.99	22.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Master Plan 2031 PM Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	1	0.0	0.001	0.7	LOS A	0.0	0.0	0.29	0.00	53.8
3	R	1	0.0	0.001	9.1	LOS A	0.0	0.0	0.29	0.74	48.2
Approac	h	2	0.0	0.001	4.9	NA	0.0	0.0	0.29	0.37	50.9
East: Di	ckson P	lace									
4	L	1	0.0	0.373	9.5	LOS A	2.0	13.9	0.35	0.61	47.2
6	R	291	0.0	0.373	9.8	LOS A	2.0	13.9	0.35	0.67	47.2
Approac	h	292	0.0	0.373	9.8	LOS A	2.0	13.9	0.35	0.67	47.2
North: B	adham	Street (N)									
7	L	192	0.0	0.109	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
8	Т	1	0.0	0.109	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	:h	193	0.0	0.109	8.1	NA	0.0	0.0	0.00	0.66	49.0
All Vehic	cles	486	0.0	0.373	9.1	NA	2.0	13.9	0.21	0.67	47.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Master Plan 2031 PM Signals - Fixed Time Cycle Time = 130 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID		Demand Flow		eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	333	0.0	0.944	61.8	LOS E	69.2	490.4	1.00	1.06	22.8
2	Т	583	2.2	0.944	53.6	LOS D	69.2	490.4	1.00	1.06	22.9
Approa	ch	916	1.4	0.944	56.6	LOS E	69.2	490.4	1.00	1.06	22.9
North: C	Cowper S	Street (N)									
8	Т	223	4.7	0.197	10.6	LOS B	5.8	42.4	0.45	0.38	44.9
9	R	84	40.0	0.733	48.6	LOS D	3.7	35.2	1.00	0.85	26.1
Approa	ch	307	14.4	0.733	21.0	LOS C	5.8	42.4	0.60	0.51	37.5
West: D	Dickson F	Place (W)									
10	L	116	36.4	0.940	89.9	LOS F	23.8	186.7	1.00	1.02	17.4
12	R	185	0.0	0.940	88.6	LOS F	23.8	186.7	1.00	1.02	17.4
Approa	ch	301	14.0	0.940	89.1	LOS F	23.8	186.7	1.00	1.02	17.4
All Vehi	icles	1524	6.5	0.944	55.8	LOS E	69.2	490.4	0.92	0.95	23.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestriar	าร					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bac Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	160	59.1	LOS E	0.6	0.6	0.95	0.95
P2	Across S approach	160	59.1	LOS E	0.6	0.6	0.95	0.95
P7	Across W approach	40	59.1	LOS E	0.1	0.1	0.95	0.95
P8	Across W approach	40	59.1	LOS E	0.1	0.1	0.95	0.95
All Pede	estrians	400	59.1	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Moyor	nont Do	rformance	o - Vohia								
					A	ا میں ما		of 0	Duar	T ff a atime	A
Mov ID	Turn	Demand	HV L	0eg. Satn	Average	Level of	95% Back		Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	V/C	sec		veh	m		per veh	km/h
South: (	Challis S	treet (S)									
1	L	1	0.0	0.666	10.4	LOS B	9.6	69.6	0.53	0.34	47.0
2	Т	238	17.7	0.666	2.2	LOS A	9.6	69.6	0.53	0.00	48.8
3	R	667	0.0	0.666	10.6	LOS B	9.6	69.6	0.53	0.72	46.9
Approa	ch	906	4.6	0.666	8.4	NA	9.6	69.6	0.53	0.53	47.4
East: C	ape Stre	et									
4	L	137	0.0	1.185	290.6	LOS F	35.5	284.4	1.00	2.18	6.7
5	Т	1	0.0	1.185	289.3	LOS F	35.5	284.4	1.00	2.07	6.7
6	R	63	53.3	1.185	293.0	LOS F	35.5	284.4	1.00	1.97	6.6
Approa	ch	201	16.8	1.185	291.3	LOS F	35.5	284.4	1.00	2.12	6.6
North: C	Challis St	treet									
7	L	42	100.0	0.201	13.5	LOS B	1.0	8.6	0.46	0.39	44.8
8	Т	46	0.0	0.201	1.6	LOS A	1.0	8.6	0.46	0.00	50.0
9	R	135	12.5	0.201	10.6	LOS B	1.0	8.6	0.46	0.76	47.4
Approa	ch	223	26.4	0.201	9.3	NA	1.0	8.6	0.46	0.53	47.3
West: C	Cape Stre	eet									
10	Ľ	2	0.0	0.576	63.1	LOS F	2.2	15.7	0.94	1.00	21.8
11	Т	1	0.0	0.576	61.8	LOS F	2.2	15.7	0.94	1.08	21.9
12	R	57	0.0	0.576	63.3	LOS F	2.2	15.7	0.94	1.08	21.8
Approa	ch	60	0.0	0.576	63.3	LOS F	2.2	15.7	0.94	1.08	21.8
All Vehi		1391	9.7	1.185	51.8	NA	35.5	284.4	0.60	0.79	24.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	316	10.7	0.184	5.9	LOS A	2.2	16.8	0.73	0.00	48.2
6	R	1	0.0	0.184	14.4	LOS B	2.2	16.8	0.73	1.06	46.2
Approa	ch	317	10.6	0.184	5.9	NA	2.2	16.8	0.73	0.00	48.2
North: V	Voolley S	Street									
7	L	1	0.0	0.953	93.9	LOS F	9.3	66.3	0.99	1.82	16.7
9	R	160	2.6	0.953	94.3	LOS F	9.3	66.3	0.99	1.67	16.6
Approa	ch	161	2.6	0.953	94.3	LOS F	9.3	66.3	0.99	1.67	16.6
West: C	ape Stre	eet (W)									
10	L	284	0.0	0.406	8.2	LOS A	0.0	0.0	0.00	0.86	49.0
11	Т	425	9.9	0.406	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	709	5.9	0.406	3.3	NA	0.0	0.0	0.00	0.35	55.0
All Vehi	cles	1187	6.7	0.953	16.3	NA	9.3	66.3	0.33	0.43	40.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street									
2	Т	779	1.6	0.440	4.0	LOS A	7.0	49.8	0.75	0.00	47.9
3	R	15	0.0	0.440	12.4	LOS B	7.0	49.8	0.75	0.98	48.2
Approad	ch	794	1.6	0.440	4.1	NA	7.0	49.8	0.75	0.02	47.9
East: Da	avenport	t Street (E)									
4	L	15	0.0	1.219	291.0	LOS F	23.2	162.5	1.00	3.00	6.6
6	R	131	0.0	1.219	291.2	LOS F	23.2	162.5	1.00	2.37	6.6
Approad	ch	145	0.0	1.219	291.2	LOS F	23.2	162.5	1.00	2.44	6.6
North: C	Cowper S	Street									
7	L	84	0.0	0.227	8.2	LOS A	0.0	0.0	0.00	0.96	49.0
8	Т	324	3.2	0.227	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	408	2.6	0.227	1.7	NA	0.0	0.0	0.00	0.20	57.3
All Vehi	cles	1347	1.7	1.219	34.3	NA	23.2	162.5	0.55	0.33	29.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV Deg. Satn		Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
2	Т	2665	3.8	0.496	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	260	6.0	1.160	203.8	LOS F	30.4	223.7	1.00	3.00	9.3
Approa	Approach		4.2	1.160	18.1	NA	30.4	223.7	0.09	0.27	40.1
East: M	lorphett S	Street (E)									
4	L	57	0.0	0.344	33.6	LOS D	1.2	8.2	0.91	1.00	31.4
Approa	ch	57	0.0	0.344	33.6	LOS D	1.2	8.2	0.91	1.00	31.4
North: I	Northbou	rne Avenue	(N)								
7	L	36	0.0	0.309	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
8	Т	1634	3.9	0.309	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approach		1669	3.8	0.309	0.2	NA	0.0	0.0	0.00	0.02	59.7
All Veh	icles	4651	4.0	1.160	11.9	NA	30.4	223.7	0.07	0.19	45.3

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID Turn		Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
East: Morphett Street (E)													
5	Т	42	0.0	0.407	5.5	LOS A	2.4	17.3	0.67	0.00	46.2		
6	R	272	2.3	0.407	13.9	LOS B	2.4	17.3	0.67	0.99	43.8		
Approad	ch	314	2.0	0.407	12.7	NA	2.4	17.3	0.67	0.85	44.1		
North: C	Challis St	treet											
7	L	181	0.0	0.224	10.5	LOS B	0.9	6.1	0.47	0.76	46.5		
9	R	15	0.0	0.050	18.7	LOS C	0.2	1.2	0.69	0.90	39.5		
Approac	ch	196	0.0	0.224	11.1	LOS B	0.9	6.1	0.48	0.77	45.9		
West: M	1orphett	Street (W)											
10	L	499	6.8	0.350	8.4	LOS A	0.0	0.0	0.00	0.71	49.0		
11	Т	99	0.0	0.350	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approach		598	5.6	0.350	7.0	NA	0.0	0.0	0.00	0.60	50.5		
All Vehi	cles	1107	3.6	0.407	9.4	NA	2.4	17.3	0.28	0.70	47.7		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	v ID Turn Demand HV Deg		eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South: 0	Cowper S	Street (S)										
1	L	44	0.0	0.363	8.2	LOS A	0.0	0.0	0.00	1.04	49.0	
2	Т	619	1.7	0.363	0.0	LOS A	0.0	0.0	0.00	0.00	60.0	
Approad	ch	663	1.6	0.363	0.5	NA	0.0	0.0	0.00	0.07	59.1	
North: C	Cowper S	Street (N)										
8	Т	244	4.3	0.258	5.4	LOS A	2.1	15.4	0.69	0.00	47.7	
9	R	84	0.0	0.258	13.8	LOS B	2.1	15.4	0.69	1.02	45.5	
Approad	ch	328	3.2	0.258	7.6	NA	2.1	15.4	0.69	0.26	47.1	
West: N	1orphett	Street										
10	L	194	1.1	0.599	22.5	LOS C	3.8	26.5	0.78	1.13	37.0	
12	R	48	0.0	0.599	22.8	LOS C	3.8	26.5	0.78	1.11	37.0	
Approach		242	0.9	0.599	22.6	LOS C	3.8	26.5	0.78	1.12	37.0	
All Vehi	cles	1234	1.9	0.599	6.7	NA	3.8	26.5	0.34	0.33	49.9	

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Northborne Avenue - Cape Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back of Queue		Prop.	Effective	Average		
	Flow					Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
East: C	East: Cape Street												
4	L	440	4.8	0.617	15.1	LOS C	5.0	36.2	0.69	1.07	42.5		
Approa	ch	440	4.8	0.617	15.1	LOS C	5.0	36.2	0.69	1.07	42.5		
North: N	Northbou	rn Avenue											
7	L	59	0.0	0.033	8.2	LOS A	0.0	0.0	0.00	0.67	49.0		
8	Т	436	5.3	0.244	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approach		495	4.7	0.244	1.0	NA	0.0	0.0	0.00	0.08	58.4		
All Vehi	icles	935	4.7	0.617	7.6	NA	5.0	36.2	0.32	0.55	49.7		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Badham Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Movement Performance - Vehicles													
Mov ID	Turn	Demand	HV Deg. Satn		Average	Level of	95% Back of Queue		Prop.	Effective	Average		
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
East: Ca	ape Stre	et											
5	Т	84	40.0	0.129	7.3	LOS A	1.1	9.6	0.62	0.00	46.7		
6	R	29	0.0	0.129	15.7	LOS C	1.1	9.6	0.62	0.99	43.6		
Approad	ch	114	29.6	0.129	9.5	NA	1.1	9.6	0.62	0.26	45.8		
North: E	Badham	Street											
7	L	69	3.0	0.266	9.6	LOS A	1.2	8.7	0.38	0.62	47.1		
9	R	229	0.0	0.266	9.8	LOS A	1.2	8.7	0.38	0.72	47.2		
Approad	ch	299	0.7	0.266	9.7	LOS A	1.2	8.7	0.38	0.70	47.2		
West: C	West: Cape Street												
10	L	322	0.0	0.257	8.2	LOS A	0.0	0.0	0.00	0.74	49.0		
11	Т	109	38.5	0.257	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approach		432	9.8	0.257	6.1	NA	0.0	0.0	0.00	0.55	51.4		
All Vehi	cles	844	9.2	0.266	7.8	NA	1.2	9.6	0.22	0.56	49.0		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Dickson Place - Cape Street Master Plan 2031 PM Giveway / Yield (Two-Way)

Moveme	lovement Performance - Vehicles lov ID Turn Demand HV Deg. Satn Average Level of 95% Back of Queue Prop. Effective Average												
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average		
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed		
		veh/h	%	v/c	sec		veh	m		per veh	km/h		
South: Ca	ape Stre	eet											
1	L	19	0.0	0.721	32.8	LOS D	5.6	45.8	0.85	1.28	31.3		
3	R	192	22.0	0.721	34.0	LOS D	5.6	45.8	0.85	1.27	31.3		
Approach	I	211	20.0	0.721	33.9	LOS D	5.6	45.8	0.85	1.27	31.3		
East: Dick	kson Pl	ace											
4	L	145	23.2	0.243	9.0	LOS A	0.0	0.0	0.00	0.91	49.0		
5	Т	272	0.0	0.243	0.0	LOS A	0.0	0.0	0.00	0.00	60.0		
Approach		417	8.1	0.243	3.2	NA	0.0	0.0	0.00	0.32	55.6		
West: Dic	kson P	lace											
11	Т	116	0.0	0.231	7.2	LOS A	2.0	13.7	0.66	0.00	46.1		
12	R	76	0.0	0.231	15.7	LOS C	2.0	13.7	0.66	0.98	43.3		
Approach	l	192	0.0	0.231	10.6	NA	2.0	13.7	0.66	0.39	45.0		
All Vehicle	es	819	9.3	0.721	12.8	NA	5.6	45.8	0.37	0.58	44.4		

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Project: X:\PROJECT\3002303 Dickson Precinct Traffic and Parking Study\015 Traffic - Intersection Flow Modelling\Dickson Intersections\_WD\_PM\_MasterPlan2031.sip 8000072, SMEC AUSTRALIA PTY. LTD., FLOATING

Site: 21 PM

1- Northbourne Avenue - Antill Street - Mouat Street Master Plan 2031 MD Signals - Fixed Time Cycle Time = 95 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back		Prop. Queued	Effective Stop Bate	Average Speed
						Service	Vehicles	Distance	Queueu	Stop Rate	
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
1	L	88	9.5	0.102	13.5	LOS B	1.4	10.3	0.43	0.69	44.1
2	Т	642	4.6	0.712	38.8	LOS D	12.2	88.7	0.97	0.84	27.8
3	R	156	2.7	0.856	61.5	LOS E	8.2	58.8	1.00	0.98	22.7
Approa	ich	886	4.8	0.856	40.3	LOS D	12.2	88.7	0.92	0.85	27.7
East: A	ntill Stree	et									
4	L	326	0.6	0.413	13.5	LOS B	5.6	39.6	0.49	0.73	43.8
5	Т	549	3.8	0.885	44.6	LOS D	18.9	136.5	0.98	0.94	25.8
6	R	368	3.4	0.885	58.2	LOS E	18.9	136.5	1.00	1.04	23.5
Approa	ich	1244	2.9	0.885	40.5	LOS D	18.9	136.5	0.86	0.92	28.0
North:	Northbou	rne Avenue (	(N)								
7	L	379	3.3	0.367	14.0	LOS B	7.3	52.3	0.51	0.73	43.4
8	Т	714	2.4	0.620	37.5	LOS D	10.3	73.3	0.96	0.80	28.2
9	R	143	1.5	0.780	57.8	LOS E	7.2	50.9	1.00	0.91	23.4
Approa	ich	1236	2.6	0.780	32.7	LOS C	10.3	73.3	0.83	0.79	30.8
West: I	Vouat Str	reet (W)									
10	L	120	1.8	0.863	58.8	LOS E	14.6	105.8	1.00	1.10	23.6
11	Т	442	6.2	0.863	50.9	LOS D	14.6	105.8	1.00	1.05	23.8
12	R	202	2.1	0.691	49.6	LOS D	9.3	66.5	0.99	0.86	25.8
Approa	ich	764	4.4	0.863	51.8	LOS D	14.6	105.8	1.00	1.01	24.3
All Veh	icles	4131	3.5	0.885	40.2	LOS D	18.9	136.5	0.89	0.88	27.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movem	nent Performance -	Pedestrian	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bao Pedestrian	ck of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	3	18.9	LOS B	0.0	0.0	0.63	0.63
P2	Across S approach	3	34.5	LOS D	0.0	0.0	0.85	0.85
P3	Across E approach	18	41.7	LOS E	0.0	0.0	0.94	0.94
P5	Across N approach	12	18.9	LOS B	0.0	0.0	0.63	0.63
P6	Across N approach	12	38.9	LOS D	0.0	0.0	0.91	0.91
P7	Across W approach	3	41.7	LOS E	0.0	0.0	0.94	0.94
All Pede	estrians	51	33.9	LOS D			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### Northbourne Avenue Service Road - Antill Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North W	/est: Nor	thbourne Av	enue Ser	vice Road							
27	L	29	21.4	0.089	18.2	LOS C	0.3	2.5	0.70	0.90	40.5
Approad	ch	29	21.4	0.089	18.2	LOS C	0.3	2.5	0.70	0.90	40.5
West: A	ntill Stre	et (W)									
10	L	6	33.3	0.272	10.2	LOS B	0.0	0.0	0.00	1.23	48.1
11	Т	971	4.3	0.272	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	977	4.5	0.272	0.1	NA	0.0	0.0	0.00	0.01	59.9
All Vehi	cles	1006	5.0	0.272	0.6	NA	0.3	2.5	0.02	0.03	59.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	e - Vehio	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	173	19.5	0.650	31.2	LOS D	3.5	28.3	0.89	1.16	32.8
Approa	ch	173	19.5	0.650	31.2	LOS D	3.5	28.3	0.89	1.16	32.8
East: A	East: Antil Street (W)										
4	L	17	100.0	0.054	11.8	LOS B	0.1	1.5	0.24	0.56	46.4
5	Т	1088	0.4	0.295	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1105	1.9	0.295	0.2	NA	0.1	1.5	0.00	0.01	59.7
West: A	Antill Stre	et									
11	Т	939	1.3	0.256	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	63	56.7	0.427	42.0	LOS E	1.4	14.5	0.90	1.03	28.6
Approa	ch	1002	4.8	0.427	2.6	NA	1.4	14.5	0.06	0.07	56.1
All Vehi	icles	2280	4.5	0.650	3.6	NA	3.5	28.3	0.09	0.12	54.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 MD Stop (Two-Way)

Moven	nent Pe	erformance	- Vehic	cles							
Mov ID	Turn	Demand Flow	HV [	0eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
1	L	642	0.0	1.440	424.4	LOS F	154.2	1087.4	1.00	7.59	4.7
3	R	95	6.7	1.440	424.4	LOS F	154.2	1087.4	1.00	5.76	4.7
Approa	ch	737	0.9	1.440	424.4	LOS F	154.2	1087.4	1.00	7.36	4.7
East: A	ntill Stree	et (W)									
4	L	109	0.0	0.180	12.6	LOS B	0.6	4.3	0.62	0.85	44.7
5	Т	564	3.0	0.155	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	674	2.5	0.180	2.1	NA	0.6	4.3	0.10	0.14	56.8
West: A	Antill Stre	et (W)									
11	Т	303	2.8	0.167	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	745	0.3	0.869	20.8	LOS C	14.8	104.2	0.84	1.50	38.2
Approa	ch	1048	1.0	0.869	14.8	NA	14.8	104.2	0.60	1.07	42.7
South V	Vest: Me	dian (RT Sta	ge 2)								
32	R	95	6.7	0.096	9.4	LOS A	0.4	2.4	0.38	0.67	33.0
Approa	ch	95	6.7	0.096	9.4	LOS A	0.4	2.4	0.38	0.67	33.0
All Vehi	icles	2554	1.6	1.440	129.4	NA	154.2	1087.4	0.57	2.62	12.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	South: Dickson Shop Access Road										
1	L	189	0.0	0.248	10.4	LOS B	1.0	7.3	0.51	0.75	46.8
Approa	ch	189	0.0	0.248	10.4	LOS B	1.0	7.3	0.51	0.75	46.8
East: A	ntill Stree	et (W)									
4	L	1	0.0	0.134	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	484	3.5	0.134	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	485	3.5	0.134	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	675	2.5	0.248	2.9	NA	1.0	7.3	0.14	0.21	55.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Cowper Street Master Plan 2031 MD Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	99	0.0	0.177	24.5	LOS C	2.1	15.0	0.76	0.76	35.8
3	R	404	0.5	0.831	34.7	LOS C	12.8	90.0	0.95	0.98	30.8
Approa	ch	503	0.4	0.831	32.7	LOS C	12.8	90.0	0.91	0.94	31.7
East: Ar	ntill Stree	et (E)									
4	L	196	1.1	0.354	25.7	LOS C	4.5	32.0	0.81	0.80	35.2
5	Т	385	4.4	0.643	27.3	LOS C	5.7	41.3	0.98	0.83	32.5
Approa	ch	581	3.3	0.643	26.7	LOS C	5.7	41.3	0.93	0.82	33.4
West: A	ntill Stre	et (W)									
11	Т	291	2.9	0.480	25.8	LOS C	4.1	29.2	0.95	0.76	33.3
12	R	107	5.9	0.293	30.3	LOS C	2.7	20.2	0.88	0.77	32.8
Approa	ch	398	3.7	0.480	27.0	LOS C	4.1	29.2	0.93	0.76	33.2
All Vehi	cles	1482	2.4	0.831	28.8	LOS C	12.8	90.0	0.92	0.85	32.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestriar	IS					
Mov	Description	Demand	Average	Level of	Average Bac		Prop.	Effective
INIOV	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	3	24.3	LOS C	0.0	0.0	0.90	0.90
P2	Across S approach	3	22.5	LOS C	0.0	0.0	0.87	0.87
P3	Across E approach	12	24.3	LOS C	0.0	0.0	0.90	0.90
P4	Across E approach	12	21.7	LOS C	0.0	0.0	0.85	0.85
All Pede	estrians	30	23.1	LOS C			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Master Plan 2031 MD Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Rosevea	r Place (S)									
1	L	34	0.0	0.054	11.6	LOS B	0.2	1.3	0.51	0.76	45.5
3	R	1	0.0	0.054	11.2	LOS B	0.2	1.3	0.51	0.79	45.8
Approa	ch	35	0.0	0.054	11.6	LOS B	0.2	1.3	0.51	0.76	45.5
East: A	ntill Stree	et (E)									
4	L	1	0.0	0.149	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	539	3.1	0.149	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	540	3.1	0.149	0.0	NA	0.0	0.0	0.00	0.00	60.0
West: A	Antill Stre	et									
11	Т	493	1.7	0.135	1.5	LOS A	1.1	7.7	0.26	0.00	55.3
12	R	1	0.0	0.135	11.1	LOS B	1.1	7.7	0.51	0.96	48.1
Approa	ch	494	1.7	0.135	1.5	NA	1.1	7.7	0.26	0.00	55.3
South V	Vest: Me	dian (RT Stag	ge 2)								
32	R	1	0.0	0.002	12.7	LOS B	0.0	0.1	0.53	0.64	28.4
Approa	ch	1	0.0	0.002	12.7	LOS B	0.0	0.1	0.53	0.64	28.4
All Vehi	icles	1069	2.4	0.149	1.1	NA	1.1	7.7	0.14	0.03	57.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Project: X:\PROJECT\3002303 Dickson Precinct Traffic and Parking Study\015 Traffic - Intersection Flow Modelling\Dickson Intersections\_WE\_MD\_MasterPlan2031.sip 8000072, SMEC AUSTRALIA PTY. LTD., FLOATING

Site: 7 MD

Badham Street - Dickson Shop Access Road Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	459	0.0	0.863	33.0	LOS D	14.2	99.5	1.00	0.00	29.1
3	R	280	0.0	0.863	41.4	LOS E	14.2	99.5	1.00	1.61	29.0
Approa	ch	739	0.0	0.863	36.2	NA	14.2	99.5	1.00	0.61	29.1
East: Di	East: Dickson Shop A		Road (E)								
4	L	107	0.0	2.729	1622.9	LOS F	109.0	763.1	1.00	5.24	1.3
6	R	147	0.0	2.729	1623.1	LOS F	109.0	763.1	1.00	4.35	1.3
Approa	ch	255	0.0	2.729	1623.0	LOS F	109.0	763.1	1.00	4.73	1.3
North: E	Badham	Street (N)									
7	L	141	0.0	0.464	8.2	LOS A	0.0	0.0	0.00	0.98	49.0
8	Т	709	0.3	0.464	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	851	0.2	0.464	1.4	NA	0.0	0.0	0.00	0.16	57.8
All Vehi	cles	1844	0.1	2.729	239.3	NA	109.0	763.1	0.54	0.97	7.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper \$	Street (S)									
1	L	183	0.0	0.272	8.2	LOS A	0.0	0.0	0.00	0.88	49.0
2	Т	309	0.7	0.272	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	493	0.4	0.272	3.0	NA	0.0	0.0	0.00	0.33	55.4
North: C	Cowper S	Street (N)									
8	Т	194	3.3	0.231	3.1	LOS A	1.5	10.6	0.50	0.00	50.3
9	R	109	0.0	0.231	11.6	LOS B	1.5	10.6	0.50	0.91	46.8
Approad	ch	303	2.1	0.231	6.2	NA	1.5	10.6	0.50	0.33	49.0
West: D	ickson S	Shop Access	Road (W	)							
10	L	189	0.0	0.269	11.0	LOS B	1.2	8.2	0.52	0.75	46.1
12	R	11	0.0	0.269	11.6	LOS B	1.2	8.2	0.52	0.89	45.9
Approad	ch	200	0.0	0.269	11.0	LOS B	1.2	8.2	0.52	0.76	46.1
All Vehi	cles	996	0.8	0.272	5.6	NA	1.5	10.6	0.26	0.41	51.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	South: Badham Street (S)										
1	<u>1 L 1</u> 2 T 5		0.0	0.353	8.2	LOS A	0.0	0.0	0.00	0.96	49.0
2	Т	518	0.0	0.353	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	646	0.0	0.353	1.6	NA	0.0	0.0	0.00	0.19	57.4
North: B	Badham	Street (N)									
8	Т	560	0.4	0.798	19.3	LOS C	14.4	100.9	1.00	0.00	35.7
9	R	326	0.0	0.798	27.7	LOS D	14.4	100.9	1.00	1.48	35.5
Approac	ch	886	0.2	0.798	22.4	NA	14.4	100.9	1.00	0.54	35.6
West: W	loolley S	Street									
10	L	225	0.0	1.588	584.5	LOS F	76.4	534.6	1.00	5.69	3.5
12	R	72	0.0	1.588	584.8	LOS F	76.4	534.6	1.00	4.30	3.5
Approac	ch	297	0.0	1.588	584.6	LOS F	76.4	534.6	1.00	5.35	3.5
All Vehi	cles	1829	0.1	1.588	106.3	NA	76.4	534.6	0.65	1.20	15.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Master Plan 2031 MD Giveway / Yield (Two-Way)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	1	0.0	0.001	0.9	LOS A	0.0	0.0	0.32	0.00	53.1
3	R	1	0.0	0.001	9.3	LOS A	0.0	0.0	0.32	0.73	48.2
Approac	ch	2	0.0	0.001	5.1	NA	0.0	0.0	0.32	0.36	50.5
East: Di	ckson P	lace									
4	L	1	0.0	0.335	9.7	LOS A	1.7	11.8	0.37	0.62	47.1
6	R	253	0.0	0.335	10.0	LOS B	1.7	11.8	0.37	0.68	47.0
Approac	ch	254	0.0	0.335	10.0	LOS B	1.7	11.8	0.37	0.68	47.0
North: B	Badham	Street (N)									
7	L	236	0.9	0.135	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
8	Т	1	0.0	0.135	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	237	0.9	0.135	8.2	NA	0.0	0.0	0.00	0.67	49.0
All Vehic	cles	493	0.4	0.335	9.1	NA	1.7	11.8	0.19	0.67	47.9

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Master Plan 2031 MD

Signals - Fixed Time Cycle Time = 120 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV C	)eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper \$	Street (S)									
1	L	413	0.0	1.093	165.7	LOS F	80.5	570.5	1.00	1.47	10.9
2	Т	301	3.5	1.093	157.5	LOS F	80.5	570.5	1.00	1.47	10.9
Approa	ch	714	1.5	1.093	162.3	LOS F	80.5	570.5	1.00	1.47	10.9
North: 0	Cowper S	Street (N)									
8	Т	118	10.7	0.146	19.4	LOS B	3.9	29.7	0.60	0.49	37.7
9	R	107	27.5	0.842	48.7	LOS D	4.4	38.1	1.00	0.90	25.9
Approa	ch	225	18.7	0.842	33.4	LOS C	4.4	38.1	0.79	0.69	31.0
West: D	Dickson F	Place (W)									
10	L	219	18.3	1.100	175.6	LOS F	67.1	497.0	1.00	1.31	10.3
12	R	366	0.0	1.100	175.0	LOS F	67.1	497.0	1.00	1.31	10.3
Approa	ch	585	6.8	1.100	175.2	LOS F	67.1	497.0	1.00	1.31	10.3
All Vehi	cles	1524	6.1	1.100	148.2	LOS F	80.5	570.5	0.97	1.29	11.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bac Pedestrian	ck of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	71	54.2	LOS E	0.2	0.2	0.95	0.95
P2	Across S approach	71	54.2	LOS E	0.2	0.2	0.95	0.95
P7	Across W approach	12	54.2	LOS E	0.0	0.0	0.95	0.95
P8	Across W approach	12	54.2	LOS E	0.0	0.0	0.95	0.95
All Pede	estrians	166	54.2	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Movement Performance - Vehicles												
	) Turn	Demand Flow		Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	v/c	sec		veh	m		per veh	km/h	
South:	Challis S	treet (S)										
1	L	1	0.0	0.376	8.7	LOS A	2.3	16.3	0.26	0.52	48.0	
2	Т	118	10.7	0.376	0.5	LOS A	2.3	16.3	0.26	0.00	53.9	
3	R	408	0.0	0.376	8.9	LOS A	2.3	16.3	0.26	0.69	47.9	
Approa	ach	527	2.4	0.376	7.1	NA	2.3	16.3	0.26	0.54	49.1	
East: C	Cape Stre	et										
4	L	88	0.0	1.572	547.3	LOS F	150.1	1096.6	1.00	5.82	3.7	
5	Т	423	0.0	1.572	546.0	LOS F	150.1	1096.6	1.00	4.61	3.7	
6	R	69	42.4	1.572	549.3	LOS F	150.1	1096.6	1.00	4.20	3.7	
Approa	ach	581	5.1	1.572	546.6	LOS F	150.1	1096.6	1.00	4.75	3.7	
North:	Challis St	treet										
7	L	40	100.0	0.099	12.6	LOS B	0.4	4.1	0.32	0.46	45.0	
8	Т	6	0.0	0.099	0.7	LOS A	0.4	4.1	0.32	0.00	52.5	
9	R	61	20.7	0.099	10.0	LOS A	0.4	4.1	0.32	0.67	47.5	
Approa	ach	107	49.0	0.099	10.4	NA	0.4	4.1	0.32	0.55	46.7	
West:	Cape Stre	eet										
10	L	1	0.0	0.470	45.6	LOS E	1.8	12.8	0.90	0.73	26.4	
11	Т	1	0.0	0.470	44.4	LOS E	1.8	12.8	0.90	1.05	26.6	
12	R	61	0.0	0.470	45.9	LOS E	1.8	12.8	0.90	1.05	26.4	
Approa	ach	63	0.0	0.470	45.9	LOS E	1.8	12.8	0.90	1.04	26.4	
All Veh	icles	1279	7.4	1.572	254.4	NA	150.1	1096.6	0.63	2.48	7.5	

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: C	ape Stre	et (E)									
5	Т	335	8.8	0.193	2.9	LOS A	1.7	12.7	0.60	0.00	50.0
6	R	1	0.0	0.193	11.4	LOS B	1.7	12.7	0.60	1.02	48.6
Approa	ch	336	8.8	0.193	3.0	NA	1.7	12.7	0.60	0.00	49.9
North: V	Voolley \$	Street									
7	L	1	0.0	0.792	40.6	LOS E	6.1	42.5	0.92	1.45	28.1
9	R	206	0.0	0.792	40.9	LOS E	6.1	42.5	0.92	1.35	28.1
Approa	ch	207	0.0	0.792	40.9	LOS E	6.1	42.5	0.92	1.35	28.1
West: C	Cape Stre	eet (W)									
10	L	206	0.0	0.263	8.2	LOS A	0.0	0.0	0.00	0.84	49.0
11	Т	244	16.4	0.263	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	451	8.9	0.263	3.7	NA	0.0	0.0	0.00	0.38	54.4
All Vehi	cles	994	7.0	0.792	11.2	NA	6.1	42.5	0.40	0.46	44.4

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehi	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Cowper \$	Street									
2	Т	632	1.7	0.354	3.9	LOS A	4.6	33.0	0.74	0.00	48.1
3	R	8	0.0	0.354	12.3	LOS B	4.6	33.0	0.74	0.98	48.2
Approac	ch	640	1.6	0.354	4.0	NA	4.6	33.0	0.74	0.01	48.1
East: Da	avenport	t Street (E)									
4	L	8	0.0	0.628	49.7	LOS E	2.9	20.4	0.91	1.13	25.2
6	R	86	0.0	0.628	49.9	LOS E	2.9	20.4	0.91	1.12	25.2
Approac	ch	95	0.0	0.628	49.9	LOS E	2.9	20.4	0.91	1.13	25.2
North: C	Cowper S	Street									
7	L	101	0.0	0.269	8.2	LOS A	0.0	0.0	0.00	0.96	49.0
8	Т	383	3.3	0.269	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	484	2.6	0.269	1.7	NA	0.0	0.0	0.00	0.20	57.3
All Vehic	cles	1219	1.9	0.628	6.7	NA	4.6	33.0	0.46	0.17	47.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Northbourne Avenue (S)											
2	Т	1144	4.0	0.213	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	282	3.0	1.148	189.9	LOS F	31.2	223.7	1.00	3.04	9.9
Approa	Approach		3.7	1.148	37.5	NA	31.2	223.7	0.20	0.60	29.6
East: R	oadNam	е									
4	L	11	0.0	0.061	27.1	LOS D	0.2	1.3	0.86	0.95	34.6
Approa	ch	11	0.0	0.061	27.1	LOS D	0.2	1.3	0.86	0.95	34.6
North: N	Vorthbou	rne Avenue	(N)								
7	L	13	16.7	0.297	8.8	LOS A	0.0	0.0	0.00	1.11	49.0
8	Т	1615	2.0	0.297	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1627	2.1	0.297	0.1	NA	0.0	0.0	0.00	0.01	59.9
All Vehi	cles	3063	2.8	1.148	17.6	NA	31.2	223.7	0.09	0.29	40.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	11	0.0	0.128	2.3	LOS A	0.5	3.7	0.48	0.00	49.3
6	R	109	0.0	0.128	10.6	LOS B	0.5	3.7	0.48	0.77	46.6
Approad	ch	120	0.0	0.128	9.8	NA	0.5	3.7	0.48	0.70	46.8
North: C	Challis St	treet									
7	L	95	0.0	0.103	9.5	LOS A	0.4	2.7	0.36	0.68	47.4
9	R	1	0.0	0.002	12.2	LOS B	0.0	0.1	0.49	0.65	44.8
Approad	ch	96	0.0	0.103	9.6	LOS A	0.4	2.7	0.36	0.68	47.4
West: N	1orphett	Street (W)									
10	L	377	3.4	0.252	8.3	LOS A	0.0	0.0	0.00	0.70	49.0
11	Т	59	3.6	0.252	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	436	3.4	0.252	7.2	NA	0.0	0.0	0.00	0.61	50.2
All Vehi	cles	652	2.3	0.252	8.0	NA	0.5	3.7	0.14	0.64	49.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	South: Cowper Street (S)										
1	L	13	0.0	0.289	8.2	LOS A	0.0	0.0	0.00	1.07	49.0
2	Т	514	2.0	0.289	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	526	2.0	0.289	0.2	NA	0.0	0.0	0.00	0.03	59.7
North: C	Cowper S	Street (N)									
8	Т	286	3.7	0.280	3.9	LOS A	2.2	15.8	0.58	0.00	49.3
9	R	101	2.1	0.280	12.4	LOS B	2.2	15.8	0.58	0.97	46.7
Approac	ch	387	3.3	0.280	6.1	NA	2.2	15.8	0.58	0.25	48.6
West: M	lorphett	Street									
10	L	122	0.0	0.337	16.2	LOS C	1.6	10.9	0.65	0.93	41.4
12	R	34	0.0	0.337	16.5	LOS C	1.6	10.9	0.65	0.95	41.3
Approac	ch	156	0.0	0.337	16.3	LOS C	1.6	10.9	0.65	0.94	41.4
All Vehi	cles	1069	2.2	0.337	4.7	NA	2.2	15.8	0.31	0.24	52.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northborne Avenue - Cape Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Average
Speed
km/h
43.5
43.5
49.0
60.0
58.0
49.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Badham Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delav	Level of Service	95% Back		Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	Sec	Service	Vehicles veh	Distance m	Queueu	per veh	km/h
East: Ca	ape Stre		/0		000		Volt				T T T T
5	Т	181	16.3	0.469	10.3	LOS B	5.7	43.0	0.83	0.00	42.2
6	R	173	0.0	0.469	18.7	LOS C	5.7	43.0	0.83	1.11	40.9
Approac	ch	354	8.3	0.469	14.4	NA	5.7	43.0	0.83	0.54	41.6
North: B	Badham	Street									
7	L	274	0.0	0.470	10.8	LOS B	3.4	23.7	0.36	0.65	46.2
9	R	227	0.0	0.470	11.0	LOS B	3.4	23.7	0.36	0.83	46.0
Approac	ch	501	0.0	0.470	10.9	LOS B	3.4	23.7	0.36	0.73	46.1
West: C	ape Stre	eet									
10	L	166	0.0	0.152	8.2	LOS A	0.0	0.0	0.00	0.76	49.0
11	Т	80	50.0	0.152	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	246	16.2	0.152	5.5	NA	0.0	0.0	0.00	0.51	52.1
All Vehic	cles	1101	6.3	0.470	10.8	NA	5.7	43.0	0.43	0.62	45.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Dickson Place - Cape Street Master Plan 2031 MD Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand	HV C	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cape Str	eet									
1	L	8	0.0	1.440	431.8	LOS F	91.5	692.1	1.00	6.00	4.6
3	R	415	9.6	1.440	432.5	LOS F	91.5	692.1	1.00	5.17	4.6
Approad	ch	423	9.5	1.440	432.4	LOS F	91.5	692.1	1.00	5.18	4.6
East: Di	ckson P	lace									
4	L	278	10.6	0.302	8.6	LOS A	0.0	0.0	0.00	0.82	49.0
5	Т	244	0.0	0.302	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	522	5.6	0.302	4.6	NA	0.0	0.0	0.00	0.43	53.6
West: D	ickson F	Place									
11	Т	173	0.0	0.260	10.5	LOS B	3.1	21.8	0.78	0.00	43.0
12	R	63	3.3	0.260	19.0	LOS C	3.1	21.8	0.78	1.03	41.4
Approad	ch	236	0.9	0.260	12.8	NA	3.1	21.8	0.78	0.28	42.6
All Vehi	cles	1181	6.1	1.440	159.5	NA	91.5	692.1	0.51	2.10	11.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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1- Northbourne Avenue - Antill Street - Mouat Street Master Plan 2031 EV Signals - Fixed Time Cycle Time = 85 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	erformance	- Vehic	les							
Mov IE	) Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue (	(S)								
1	L	59	3.6	0.054	12.1	LOS B	0.7	5.3	0.41	0.67	45.2
2	Т	368	2.9	0.362	29.5	LOS C	5.5	39.2	0.87	0.70	31.6
3	R	118	3.6	0.833	56.6	LOS E	5.6	40.2	1.00	0.95	23.9
Approa	ach	545	3.1	0.833	33.5	LOS C	5.6	40.2	0.85	0.75	30.4
East: A	ntill Stree	et									
4	L	284	0.0	0.351	13.2	LOS B	4.4	31.1	0.50	0.73	44.0
5	Т	516	3.3	0.862	40.4	LOS D	14.7	105.9	0.98	0.93	27.2
6	R	253	3.3	0.734	44.9	LOS D	10.6	76.3	0.99	0.90	27.2
Approa	ach	1053	2.4	0.862	34.1	LOS C	14.7	105.9	0.85	0.87	30.4
North:	Northbou	rne Avenue (	N)								
7	L	206	4.1	0.178	10.8	LOS B	2.3	16.8	0.37	0.69	46.4
8	Т	724	1.7	0.561	31.6	LOS C	9.0	64.2	0.93	0.78	30.6
9	R	116	0.0	0.798	55.2	LOS E	5.4	37.5	1.00	0.91	24.1
Approa	ach	1046	2.0	0.798	30.1	LOS C	9.0	64.2	0.83	0.78	31.8
West: I	Mouat Str	reet (W)									
10	L	53	0.0	0.820	54.0	LOS D	10.6	76.5	1.00	1.01	25.1
11	Т	362	4.7	0.820	45.1	LOS D	10.6	76.5	1.00	0.99	25.4
12	R	274	0.0	0.820	51.2	LOS D	10.2	73.6	1.00	0.96	25.5
Approa	ach	688	2.4	0.820	48.2	LOS D	10.6	76.5	1.00	0.98	25.4
All Veh	icles	3333	2.4	0.862	35.7	LOS D	14.7	105.9	0.88	0.84	29.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	IS					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Bac Pedestrian	k of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	1	19.1	LOS B	0.0	0.0	0.67	0.67
P2	Across S approach	1	33.1	LOS D	0.0	0.0	0.88	0.88
P3	Across E approach	2	36.7	LOS D	0.0	0.0	0.93	0.93
P5	Across N approach	3	19.1	LOS B	0.0	0.0	0.67	0.67
P6	Across N approach	3	36.7	LOS D	0.0	0.0	0.93	0.93
P7	Across W approach	1	36.7	LOS D	0.0	0.0	0.93	0.93
All Pede	estrians	11	30.0	LOS C			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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#### Northbourne Avenue Service Road - Antill Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
North W	est: Nor	thbourne Ave	enue Serv	vice Road							
27	L	42	5.0	0.067	11.7	LOS B	0.2	1.8	0.53	0.77	45.4
Approad	ch	42	5.0	0.067	11.7	LOS B	0.2	1.8	0.53	0.77	45.4
West: A	ntill Stre	et (W)									
10	L	6	66.7	0.190	11.3	LOS B	0.0	0.0	0.00	1.27	48.1
11	Т	678	3.7	0.190	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	684	4.3	0.190	0.1	NA	0.0	0.0	0.00	0.01	59.9
All Vehi	cles	726	4.3	0.190	0.8	NA	0.2	1.8	0.03	0.06	58.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Challis Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	e - Vehi	icles							
Mov ID	Turn	Demand Flow	ΗV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Challis S	treet (S)									
1	L	196	11.8	0.453	18.0	LOS C	2.3	17.5	0.75	1.00	40.5
Approa	ch	196	11.8	0.453	18.0	LOS C	2.3	17.5	0.75	1.00	40.5
East: Ar	ntil Stree	t (W)									
4	L	8	100.0	0.027	12.0	LOS B	0.1	0.7	0.26	0.56	46.3
5	Т	853	0.2	0.231	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	861	1.2	0.231	0.1	NA	0.1	0.7	0.00	0.01	59.8
West: A	ntill Stre	et									
11	Т	634	0.3	0.172	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	88	28.6	0.228	18.1	LOS C	0.7	6.5	0.72	0.92	40.8
Approa	ch	722	3.8	0.228	2.2	NA	0.7	6.5	0.09	0.11	56.7
All Vehi	cles	1779	3.4	0.453	2.9	NA	2.3	17.5	0.12	0.16	55.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 EV Stop (Two-Way)

Moven	nent Pe	erformance	- Vehi	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
1	L	493	0.0	1.259	264.3	LOS F	96.6	680.1	1.00	5.78	7.3
3	R	147	2.9	1.259	264.1	LOS F	96.6	680.1	1.00	4.53	7.3
Approa	ch	640	0.7	1.259	264.3	LOS F	96.6	680.1	1.00	5.49	7.3
East: A	ntill Stree	et (W)									
4	L	204	0.0	0.274	11.6	LOS B	1.1	7.7	0.56	0.83	45.6
5	Т	476	2.2	0.130	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	680	1.5	0.274	3.5	NA	1.1	7.7	0.17	0.25	54.8
West: A	Antill Stre	et (W)									
11	Т	234	0.9	0.127	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
12	R	608	0.0	0.648	13.5	LOS B	6.1	42.5	0.60	0.99	43.8
Approa	ch	842	0.3	0.648	9.8	NA	6.1	42.5	0.44	0.71	47.3
South V	Vest: Me	dian (RT Sta	ge 2)								
32	R	147	2.9	0.136	8.9	LOS A	0.6	3.5	0.33	0.65	33.3
Approa	ch	147	2.9	0.136	8.9	LOS A	0.6	3.5	0.33	0.65	33.3
All Vehi	icles	2309	0.9	1.259	78.4	NA	96.6	680.1	0.51	1.90	18.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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#### Antill Street - Dickson Shops Access Road Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Dickson	Shop Access	Road								
1	L	131	0.0	0.183	10.7	LOS B	0.7	5.1	0.52	0.76	46.5
Approa	ch	131	0.0	0.183	10.7	LOS B	0.7	5.1	0.52	0.76	46.5
East: A	ntill Stree	et (W)									
4	L	1	0.0	0.150	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
5	Т	547	1.9	0.150	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	548	1.9	0.150	0.0	NA	0.0	0.0	0.00	0.00	60.0
All Vehi	cles	679	1.6	0.183	2.1	NA	0.7	5.1	0.10	0.15	56.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Cowper S	Street (S)									
1	L	141	0.0	0.445	29.2	LOS C	3.3	22.9	0.94	0.79	33.3
3	R	152	0.0	0.478	29.0	LOS C	3.5	24.8	0.94	0.79	33.4
Approac	:h	293	0.0	0.478	29.1	LOS C	3.5	24.8	0.94	0.79	33.4
East: Ar	ntill Stree	et (E)									
4	L	194	0.0	0.611	30.3	LOS C	4.7	33.1	0.97	0.83	32.7
5	Т	408	2.6	0.561	20.6	LOS C	4.8	34.2	0.95	0.78	36.1
Approac	h	602	1.7	0.611	23.8	LOS C	4.8	34.2	0.96	0.80	35.0
West: A	ntill Stre	et (W)									
11	Т	312	2.0	0.427	19.9	LOS B	3.5	25.0	0.92	0.74	36.6
12	R	74	0.0	0.161	24.0	LOS C	1.4	10.1	0.81	0.75	36.2
Approac	h	385	1.6	0.427	20.7	LOS C	3.5	25.0	0.90	0.74	36.6
All Vehic	cles	1280	1.3	0.611	24.1	LOS C	4.8	34.2	0.93	0.78	35.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestriar	าร					
Mov	Description	Demand	Average	Level of			Prop.	Effective
IVIOV	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	5	19.4	LOS B	0.0	0.0	0.88	0.88
P2	Across S approach	5	17.6	LOS B	0.0	0.0	0.84	0.84
P3	Across E approach	7	19.4	LOS B	0.0	0.0	0.88	0.88
P4	Across E approach	7	16.8	LOS B	0.0	0.0	0.82	0.82
All Pede	estrians	24	18.3	LOS B			0.85	0.85

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Rosevear Place Master Plan 2031 EV Giveway / Yield (Two-Way)

Flow         Delay         Service         Vehicles         Distance         Queued         Stop Rate         Speed           veh/h         %         v/c         sec         veh         m         per veh         km/h           South: Rosevear Place (S)         1         L         53         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           3         R         1         0.0         0.074         10.4         LOS B         0.3         1.8         0.47         0.74         46.2           Approach         54         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           East: Antill Street (E)	Moven	nent Pe	erformance	- Vehic	les							
South: Rosevear Place (S)           1         L         53         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           3         R         1         0.0         0.074         10.4         LOS B         0.3         1.8         0.47         0.74         46.2           Approach         54         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           East: Antill Street (E)	Mov ID	Turn		HV D	eg. Satn							Average Speed
1         L         53         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           3         R         1         0.0         0.074         10.4         LOS B         0.3         1.8         0.47         0.74         46.2           Approach         54         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           East: Antill Street (E)			veh/h	%	v/c	sec		veh	m		per veh	km/h
3       R       1       0.0       0.074       10.4       LOS B       0.3       1.8       0.47       0.77       46.6         Approach       54       0.0       0.074       10.8       LOS B       0.3       1.8       0.47       0.74       46.2         East: Antill Street (E)	South: F	Rosevea	r Place (S)									
Approach         54         0.0         0.074         10.8         LOS B         0.3         1.8         0.47         0.74         46.2           East: Antill Street (E)         4         1         0.0         0.122         8.2         LOS A         0.0         0.0         1.09         49.0           5         T         444         2.4         0.122         0.0         LOS A         0.0         0.0         0.00         1.09         49.0           5         T         444         2.4         0.122         0.0         LOS A         0.0         0.0         0.00         0.00         60.0           Approach         445         2.4         0.122         0.0         NA         0.0         0.0         0.00         0.00         60.0           West: Antill Street         U         U         1         LOS A         0.7         4.8         0.22         0.00         55.8           12         R         1         0.0         0.097         1.1         LOS A         0.7         4.8         0.45         0.93         48.8           Approach         353         1.8         0.097         1.1         NA         0.7         4.8	1	L	53	0.0	0.074	10.8	LOS B	0.3	1.8	0.47	0.74	46.2
East: Antill Street (E)         4       1       0.0       0.122       8.2       LOS A       0.0       0.0       1.09       49.0         5       T       444       2.4       0.122       0.0       LOS A       0.0       0.0       0.00       0.00       60.0         Approach       445       2.4       0.122       0.0       NA       0.0       0.0       0.00       60.0         West: Antill Street	3	R	1	0.0	0.074	10.4	LOS B	0.3	1.8	0.47	0.77	46.6
4       L       1       0.0       0.122       8.2       LOS A       0.0       0.0       0.00       1.09       49.0         5       T       444       2.4       0.122       0.0       LOS A       0.0       0.0       0.00       0.00       60.0         Approach       445       2.4       0.122       0.0       NA       0.0       0.0       0.00       0.00       60.0         West: Antill Street	Approa	ch	54	0.0	0.074	10.8	LOS B	0.3	1.8	0.47	0.74	46.2
5         T         444         2.4         0.122         0.0         LOS A         0.0         0.0         0.00         0.00         60.0           Approach         445         2.4         0.122         0.0         NA         0.0         0.0         0.00         0.00         60.0           West: Antill Street   <	East: Ar	ntill Stree	et (E)									
Approach         445         2.4         0.122         0.0         NA         0.0         0.0         0.00         0.00         60.0           West: Antill Street	4	L	1	0.0	0.122	8.2	LOS A	0.0	0.0	0.00	1.09	49.0
West: Antill Street         11         T         352         1.8         0.097         1.1         LOS A         0.7         4.8         0.22         0.00         55.8           12         R         1         0.0         0.097         10.3         LOS B         0.7         4.8         0.45         0.93         48.8           Approach         353         1.8         0.097         1.1         NA         0.7         4.8         0.45         0.93         48.8           South West: Median (RT Stage 2)         32         R         1         0.0         0.002         10.8         LOS B         0.0         0.0         0.46         0.61         30.8	5	Т	444	2.4	0.122	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
11         T         352         1.8         0.097         1.1         LOS A         0.7         4.8         0.22         0.00         55.8           12         R         1         0.0         0.097         10.3         LOS B         0.7         4.8         0.45         0.93         48.8           Approach         353         1.8         0.097         1.1         NA         0.7         4.8         0.45         0.93         48.8           South West: Median (RT Stage 2)         32         R         1         0.0         0.002         10.8         LOS B         0.0         0.0         0.46         0.61         30.8	Approa	ch	445	2.4	0.122	0.0	NA	0.0	0.0	0.00	0.00	60.0
12         R         1         0.0         0.097         10.3         LOS R         0.7         4.8         0.45         0.93         48.8           Approach         353         1.8         0.097         1.1         NA         0.7         4.8         0.45         0.93         48.8           South West: Median (RT Stage 2)         32         R         1         0.0         0.002         10.8         LOS B         0.0         0.0         0.46         0.61         30.8	West: A	ntill Stre	et									
Approach         353         1.8         0.097         1.1         NA         0.7         4.8         0.22         0.00         55.8           South West: Median (RT Stage 2)         32         R         1         0.0         0.002         10.8         LOS B         0.0         0.46         0.61         30.8	11	Т	352	1.8	0.097	1.1	LOS A	0.7	4.8	0.22	0.00	55.8
South West: Median (RT Stage 2)         32         R         1         0.0         0.002         10.8         LOS B         0.0         0.46         0.61         30.8	12	R	1	0.0	0.097	10.3	LOS B	0.7	4.8	0.45	0.93	48.8
32 R 1 0.0 0.002 10.8 LOS B 0.0 0.0 0.46 0.61 30.8	Approa	ch	353	1.8	0.097	1.1	NA	0.7	4.8	0.22	0.00	55.8
	South V	Vest: Me	dian (RT Stag	ge 2)								
Approach 1 0.0 0.002 10.8 LOS B 0.0 0.0 0.46 0.61 30.8	32	R	1	0.0	0.002	10.8	LOS B	0.0	0.0	0.46	0.61	30.8
	Approa	ch	1	0.0	0.002	10.8	LOS B	0.0	0.0	0.46	0.61	30.8
All Vehicles 853 2.0 0.122 1.2 NA 0.7 4.8 0.12 0.05 57.1	All Vehi	cles	853	2.0	0.122	1.2	NA	0.7	4.8	0.12	0.05	57.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

 $\label{eq:model} \ensuremath{\mathsf{Minor}}\xspace \ensuremath{\mathsf{Road}}\xspace \ensuremath{\mathsf{Approach}}\xspace \ensuremath{\mathsf{LOS}}\xspace \ensuremath{\mathsf{values}}\xspace \ensuremath{\mathsf{aproach}}\xspace \ensuremath{\mathsf{aproach$ 

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Shop Access Road Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
2	Т	385	0.0	0.443	12.0	LOS B	5.9	41.6	1.00	0.00	41.0
3	R	116	0.0	0.443	20.4	LOS C	5.9	41.6	1.00	1.13	40.9
Approa	ch	501	0.0	0.443	13.9	NA	5.9	41.6	1.00	0.26	41.0
East: Di	ickson S	hop Access I	Road (E)								
4	L	91	0.0	1.855	839.6	LOS F	67.8	474.3	1.00	4.45	2.5
6	R	120	0.0	1.855	839.9	LOS F	67.8	474.3	1.00	3.83	2.5
Approa	ch	211	0.0	1.855	839.8	LOS F	67.8	474.3	1.00	4.10	2.5
North: E	Badham	Street (N)									
7	L	145	0.0	0.442	8.2	LOS A	0.0	0.0	0.00	0.97	49.0
8	Т	665	0.0	0.442	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	811	0.0	0.442	1.5	NA	0.0	0.0	0.00	0.17	57.7
All Vehi	cles	1522	0.0	1.855	121.5	NA	67.8	474.3	0.47	0.75	13.7

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Shop Access Road Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	99	0.0	0.167	8.2	LOS A	0.0	0.0	0.00	0.90	49.0
2	Т	204	0.0	0.167	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	303	0.0	0.167	2.7	NA	0.0	0.0	0.00	0.29	55.9
North: C	Cowper S	Street (N)									
8	Т	225	0.0	0.160	1.5	LOS A	1.0	7.2	0.33	0.00	53.6
9	R	42	0.0	0.160	10.0	LOS A	1.0	7.2	0.33	0.94	48.4
Approac	ch	267	0.0	0.160	2.9	NA	1.0	7.2	0.33	0.15	52.7
West: D	ickson S	Shop Access	Road (W	)							
10	L	88	0.0	0.108	9.5	LOS A	0.4	3.1	0.37	0.64	47.5
12	R	6	0.0	0.108	10.1	LOS B	0.4	3.1	0.37	0.82	47.2
Approad	ch	95	0.0	0.108	9.5	LOS A	0.4	3.1	0.37	0.65	47.5
All Vehi	cles	665	0.0	0.167	3.7	NA	1.0	7.2	0.19	0.29	53.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Woolley Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	286	0.0	0.333	8.2	LOS A	0.0	0.0	0.00	0.83	49.0
2	Т	316	0.0	0.333	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	602	0.0	0.333	3.9	NA	0.0	0.0	0.00	0.40	54.2
North: B	North: Badham Street (N)										
8	Т	446	0.0	0.818	16.9	LOS C	14.5	101.2	1.00	0.00	36.8
9	R	408	0.0	0.818	25.4	LOS D	14.5	101.2	1.00	1.51	36.6
Approac	ch	855	0.0	0.818	20.9	NA	14.5	101.2	1.00	0.72	36.7
West: W	/oolley S	Street									
10	L	183	0.0	1.282	332.2	LOS F	43.0	300.8	1.00	4.21	5.9
12	R	61	0.0	1.282	332.5	LOS F	43.0	300.8	1.00	3.11	5.9
Approac	Approach		0.0	1.282	332.3	LOS F	43.0	300.8	1.00	3.94	5.9
All Vehic	cles	1701	0.0	1.282	59.6	NA	43.0	300.8	0.65	1.07	22.5

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Badham Street - Dickson Place Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
2	Т	1	0.0	0.001	0.6	LOS A	0.0	0.0	0.27	0.00	54.2
3	R	1	0.0	0.001	9.0	LOS A	0.0	0.0	0.27	0.74	48.3
Approad	ch	2	0.0	0.001	4.8	NA	0.0	0.0	0.27	0.37	51.1
East: Di	East: Dickson Place										
4	L	1	0.0	0.371	9.3	LOS A	2.0	14.0	0.32	0.60	47.4
6	R	295	0.0	0.371	9.6	LOS A	2.0	14.0	0.32	0.66	47.4
Approad	ch	296	0.0	0.371	9.6	LOS A	2.0	14.0	0.32	0.66	47.4
North: E	Badham	Street (N)									
7	L	166	0.0	0.095	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
8	Т	1	0.0	0.095	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	167	0.0	0.095	8.1	NA	0.0	0.0	0.00	0.66	49.0
All Vehi	cles	465	0.0	0.371	9.1	NA	2.0	14.0	0.21	0.66	48.0

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Dickson Place Master Plan 2031 EV

Signals - Fixed Time Cycle Time = 120 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	427	0.0	0.807	42.8	LOS D	32.4	228.3	0.94	0.91	27.9
2	Т	183	2.3	0.807	34.7	LOS C	32.4	228.3	0.94	0.87	28.2
Approa	ch	611	0.7	0.807	40.4	LOS D	32.4	228.3	0.94	0.90	28.0
North: C	Cowper S	Street (N)									
8	Т	122	3.4	0.129	15.3	LOS B	3.6	25.7	0.54	0.44	40.7
9	R	126	15.0	0.753	40.6	LOS D	4.6	36.6	0.97	0.87	28.5
Approa	ch	248	9.3	0.753	28.2	LOS C	4.6	36.6	0.76	0.66	33.4
West: D	ickson F	Place (W)									
10	L	78	29.7	0.804	58.9	LOS E	20.6	152.6	1.00	0.91	23.1
12	R	272	0.0	0.804	57.9	LOS E	20.6	152.6	1.00	0.91	23.1
Approa	ch	349	6.6	0.804	58.2	LOS E	20.6	152.6	1.00	0.91	23.1
All Vehi	cles	1208	4.2	0.807	43.0	LOS D	32.4	228.3	0.92	0.85	27.2

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians											
Mov ID	Description	Demand         Average         Level of         Average Back of Queue           Description         Flow         Delay         Service         Pedestrian         Distance				Prop. Queued	Effective Stop Rate				
		ped/h	sec		ped	m		per ped			
P1	Across S approach	47	54.2	LOS E	0.2	0.2	0.95	0.95			
P2	Across S approach	47	54.2	LOS E	0.2	0.2	0.95	0.95			
P7	Across W approach	12	54.2	LOS E	0.0	0.0	0.95	0.95			
P8	Across W approach	12	54.2	LOS E	0.0	0.0	0.95	0.95			
All Pede	estrians	118	54.2	LOS E			0.95	0.95			

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Challis Street - Cape Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Move	nent Pe	erformance	e - Vehi	icles							
Mov ID		Demand Flow		Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	4	0.0	0.440	8.9	LOS A	2.9	20.6	0.33	0.47	47.7
2	Т	131	6.5	0.440	0.7	LOS A	2.9	20.6	0.33	0.00	52.5
3	R	474	0.0	0.440	9.2	LOS A	2.9	20.6	0.33	0.69	47.7
Approa	ich	608	1.4	0.440	7.3	NA	2.9	20.6	0.33	0.54	48.7
East: C	ape Stre	et									
4	L	177	0.0	1.776	732.5	LOS F	188.5	1353.5	1.00	7.14	2.8
5	Т	381	0.0	1.776	731.2	LOS F	188.5	1353.5	1.00	5.18	2.8
6	R	67	28.1	1.776	733.9	LOS F	188.5	1353.5	1.00	4.86	2.8
Approa	ich	625	3.0	1.776	731.9	LOS F	188.5	1353.5	1.00	5.70	2.8
North:	Challis St	treet									
7	L	23	100.0	0.101	12.6	LOS B	0.5	4.3	0.30	0.58	46.0
8	Т	63	3.3	0.101	0.7	LOS A	0.5	4.3	0.30	0.00	53.5
9	R	48	17.4	0.101	9.9	LOS A	0.5	4.3	0.30	0.78	48.2
Approa	ich	135	25.0	0.101	6.0	NA	0.5	4.3	0.30	0.38	50.0
West: 0	Cape Stre	eet									
10	L	1	0.0	1.775	777.1	LOS F	50.9	356.6	1.00	4.11	2.7
11	Т	1	0.0	1.775	775.8	LOS F	50.9	356.6	1.00	3.70	2.7
12	R	160	0.0	1.775	777.3	LOS F	50.9	356.6	1.00	3.24	2.7
Approa	ich	162	0.0	1.775	777.3	LOS F	50.9	356.6	1.00	3.25	2.7
All Veh	icles	1531	4.0	1.776	384.8	NA	188.5	1353.5	0.67	2.92	5.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Cape Street - Woolley Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et (E)									
5	Т	318	6.0	0.180	3.2	LOS A	1.6	12.1	0.62	0.00	49.7
6	R	1	0.0	0.180	11.6	LOS B	1.6	12.1	0.62	1.03	48.4
Approac	Approach		5.9	0.180	3.2	NA	1.6	12.1	0.62	0.00	49.7
North: Woolley Street											
7	L	1	0.0	0.883	47.8	LOS E	9.6	67.3	0.95	1.73	25.7
9	R	259	0.0	0.883	48.1	LOS E	9.6	67.3	0.95	1.60	25.7
Approac	ch	260	0.0	0.883	48.1	LOS E	9.6	67.3	0.95	1.60	25.7
West: C	ape Stre	eet (W)									
10	L	337	0.0	0.288	8.2	LOS A	0.0	0.0	0.00	0.76	49.0
11	Т	164	14.1	0.288	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	501	4.6	0.288	5.5	NA	0.0	0.0	0.00	0.51	52.1
All Vehic	cles	1080	3.9	0.883	15.1	NA	9.6	67.3	0.41	0.62	41.3

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Davenport Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	cles							
Mov ID	Turn	Demand Flow	HV C	0eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street									
2	Т	516	0.8	0.287	2.5	LOS A	3.0	20.8	0.61	0.00	49.8
3	R	6	0.0	0.287	11.0	LOS B	3.0	20.8	0.61	0.95	49.1
Approac	ch	522	0.8	0.287	2.6	NA	3.0	20.8	0.61	0.01	49.8
East: Davenport		t Street (E)									
4	L	13	0.0	0.460	29.4	LOS D	2.1	14.8	0.82	0.94	33.0
6	R	95	0.0	0.460	29.6	LOS D	2.1	14.8	0.82	1.04	33.0
Approac	ch	107	0.0	0.460	29.6	LOS D	2.1	14.8	0.82	1.03	33.0
North: C	Cowper S	Street									
7	L	63	0.0	0.215	8.2	LOS A	0.0	0.0	0.00	0.98	49.0
8	Т	328	1.3	0.215	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	392	1.1	0.215	1.3	NA	0.0	0.0	0.00	0.16	57.9
All Vehi	cles	1021	0.8	0.460	5.0	NA	3.0	20.8	0.40	0.17	49.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Mover	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
2	Т	851	2.7	0.157	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
3	R	324	1.7	1.119	161.3	LOS F	31.5	223.6	1.00	3.06	11.3
Approach		1175	2.3	1.119	44.4	NA	31.5	223.6	0.28	0.84	27.1
East: Morphett St		Street (E)									
4	L	6	0.0	0.030	23.2	LOS C	0.1	0.6	0.83	0.93	36.9
Approa	ch	6	0.0	0.030	23.2	LOS C	0.1	0.6	0.83	0.93	36.9
North: N	Northbou	rne Avenue (	(N)								
7	L	27	0.0	0.274	8.2	LOS A	0.0	0.0	0.00	1.05	49.0
8	Т	1480	1.4	0.274	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	1507	1.4	0.274	0.1	NA	0.0	0.0	0.00	0.02	59.8
All Vehi	cles	2688	1.8	1.119	19.6	NA	31.5	223.6	0.12	0.38	39.1

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	6	0.0	0.153	2.9	LOS A	0.6	4.3	0.52	0.00	48.5
6	R	122	0.0	0.153	11.2	LOS B	0.6	4.3	0.52	0.81	46.0
Approad	ch	128	0.0	0.153	10.7	NA	0.6	4.3	0.52	0.77	46.1
North: Challis Street		treet									
7	L	185	1.1	0.213	10.1	LOS B	0.8	5.9	0.42	0.72	47.0
9	R	1	0.0	0.002	13.0	LOS B	0.0	0.1	0.52	0.66	44.1
Approad	ch	186	1.1	0.213	10.1	LOS B	0.8	5.9	0.42	0.72	47.0
West: N	1orphett	Street (W)									
10	L	429	2.0	0.288	8.3	LOS A	0.0	0.0	0.00	0.71	49.0
11	Т	76	0.0	0.288	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	ch	505	1.7	0.288	7.0	NA	0.0	0.0	0.00	0.60	50.3
All Vehi	cles	820	1.3	0.288	8.3	NA	0.8	5.9	0.18	0.66	48.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cowper Street - Morphett Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: 0	Cowper \$	Street (S)									
1	L	15	0.0	0.243	8.2	LOS A	0.0	0.0	0.00	1.07	49.0
2	Т	432	1.0	0.243	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approad	Approach 4		0.9	0.243	0.3	NA	0.0	0.0	0.00	0.04	59.6
North: Cowper Street (N		Street (N)									
8	Т	213	2.0	0.258	2.9	LOS A	1.7	11.7	0.48	0.00	50.5
9	R	131	0.0	0.258	11.3	LOS B	1.7	11.7	0.48	0.90	47.0
Approad	ch	343	1.2	0.258	6.1	NA	1.7	11.7	0.48	0.34	49.2
West: N	/lorphett	Street									
10	L	93	0.0	0.254	14.1	LOS B	1.0	7.2	0.59	0.82	43.1
12	R	36	0.0	0.254	14.4	LOS B	1.0	7.2	0.59	0.90	43.0
Approad	ch	128	0.0	0.254	14.2	LOS B	1.0	7.2	0.59	0.84	43.1
All Vehi	cles	918	0.9	0.258	4.4	NA	1.7	11.7	0.26	0.26	52.6

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Northborne Avenue - Cape Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand	HV D	eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et									
4	L	436	1.9	0.528	12.5	LOS B	3.9	27.8	0.59	0.91	44.7
Approa	ch	436	1.9	0.528	12.5	LOS B	3.9	27.8	0.59	0.91	44.7
North: N	Northbou	rn Avenue									
7	L	160	0.0	0.091	8.2	LOS A	0.0	0.0	0.00	0.67	49.0
8	Т	269	2.3	0.148	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	429	1.5	0.148	3.0	NA	0.0	0.0	0.00	0.25	55.4
All Vehi	icles	865	1.7	0.528	7.8	NA	3.9	27.8	0.30	0.58	49.4

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Cape Street - Badham Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Movem	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: Ca	ape Stre	et									
5	Т	202	9.4	0.379	5.5	LOS A	4.1	30.0	0.67	0.00	47.4
6	R	156	0.0	0.379	13.9	LOS B	4.1	30.0	0.67	0.95	44.8
Approac	ch	358	5.3	0.379	9.1	NA	4.1	30.0	0.67	0.41	46.2
North: Badham		Street									
7	L	206	0.0	0.382	9.9	LOS A	2.0	14.1	0.26	0.60	47.1
9	R	213	0.0	0.382	10.1	LOS B	2.0	14.1	0.26	0.77	46.9
Approac	ch	419	0.0	0.382	10.0	LOS A	2.0	14.1	0.26	0.69	47.0
West: C	ape Stre	eet									
10	L	109	0.0	0.100	8.2	LOS A	0.0	0.0	0.00	0.76	49.0
11	Т	55	42.3	0.100	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approac	ch	164	14.1	0.100	5.5	NA	0.0	0.0	0.00	0.51	52.2
All Vehic	cles	941	4.5	0.382	8.9	NA	4.1	30.0	0.37	0.55	47.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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Dickson Place - Cape Street Master Plan 2031 EV Giveway / Yield (Two-Way)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand	HV C	)eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cape Str	reet									
1	L	1	0.0	0.665	29.5	LOS D	4.5	34.6	0.84	1.23	32.9
3	R	198	11.7	0.665	30.2	LOS D	4.5	34.6	0.84	1.20	32.9
Approa	ch	199	11.6	0.665	30.2	LOS D	4.5	34.6	0.84	1.20	32.9
East: Di	ickson P	lace									
4	L	251	7.6	0.309	8.5	LOS A	0.0	0.0	0.00	0.84	49.0
5	Т	295	0.0	0.309	0.0	LOS A	0.0	0.0	0.00	0.00	60.0
Approa	ch	545	3.5	0.309	3.9	NA	0.0	0.0	0.00	0.39	54.4
West: D	ickson F	Place									
11	Т	149	0.0	0.126	8.8	LOS A	1.8	12.4	0.75	0.00	45.2
12	R	17	0.0	0.126	17.3	LOS C	1.8	12.4	0.75	1.03	43.2
Approa	ch	166	0.0	0.126	9.7	NA	1.8	12.4	0.75	0.10	45.0
All Vehi	cles	911	4.6	0.665	10.7	NA	4.5	34.6	0.32	0.51	46.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements. SIDRA Standard Delay Model used.

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# APPENDIX D 2012 (RECOMMENDED) SIDRA RESULTS

#### **MOVEMENT SUMMARY**

#### Site: 16 AM - Conversion

Northbourne Avenue - Morphett Street 2012 AM

Signals - Fixed Time Cycle Time = 60 seconds (Practical Cycle Time)

Moven	nent Pe	erformance	- Vehi	icles							
Mov ID	Turn	Demand Flow	ΗV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Northbou	Irne Avenue	(S)								
2	Т	755	9.3	0.144	0.0	Х	Х	Х	Х	0.00	60.0
3	R	378	1.7	0.868	40.1	LOS D	13.3	94.4	1.00	1.07	29.0
Approad	ch	1133	6.8	0.868	13.4	LOS B	13.3	94.4	0.33	0.36	44.0
East: M	orphett S	Street (E)									
4	L	262	2.0	0.434	20.2	LOS C	5.6	39.7	0.81	0.79	38.7
Approad	ch	262	2.0	0.434	20.2	LOS C	5.6	39.7	0.81	0.79	38.7
North: N	lorthbou	rne Avenue	(N)								
7	L	115	9.2	0.881	32.4	LOS C	29.5	214.8	0.94	1.11	33.6
8	Т	2492	4.2	0.881	23.8	LOS C	29.7	215.6	0.94	1.05	34.3
Approad	ch	2606	4.4	0.881	24.1	LOS C	29.7	215.6	0.94	1.06	34.3
All Vehi	cles	4001	4.9	0.881	20.8	LOS C	29.7	215.6	0.76	0.84	36.9

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 PM

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbou	Irne Avenue	(S)								
2	Т	2455	2.8	0.450	0.1	Х	Х	Х	Х	0.00	59.9
3	R	423	0.0	0.554	21.3	LOS C	9.4	65.9	0.80	0.81	38.2
Approach		2878	2.4	0.554	3.2	LOS A	9.4	65.9	0.12	0.12	55.2
East: M	orphett S	Street (E)									
4	L	358	0.6	0.359	11.6	LOS B	4.4	30.9	0.54	0.74	45.4
Approa	ch	358	0.6	0.359	11.6	LOS B	4.4	30.9	0.54	0.74	45.4
North: N	Vorthbou	rne Avenue (	(N)								
7	L	41	0.0	0.573	25.0	LOS C	9.1	65.5	0.85	0.88	37.5
8	Т	1095	3.8	0.573	16.8	LOS B	9.1	65.9	0.85	0.73	38.7
Approa	ch	1136	3.7	0.573	17.1	LOS B	9.1	65.9	0.85	0.74	38.7
All Vehi	cles	4372	2.6	0.573	7.5	LOS A	9.4	65.9	0.34	0.33	48.9

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 AM

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Northbou	Irne Avenue	(S)								
2	Т	1177	2.6	0.216	0.0	Х	Х	Х	Х	0.00	60.0
3	R	293	0.7	0.556	27.0	LOS C	7.4	52.4	0.89	0.82	34.9
Approa	ch	1469	2.2	0.556	5.4	LOS A	7.4	52.4	0.18	0.16	52.3
East: RoadNam		е									
4	L	222	0.9	0.292	12.4	LOS B	3.0	21.0	0.57	0.74	44.7
Approa	ch	222	0.9	0.292	12.4	LOS B	3.0	21.0	0.57	0.74	44.7
North: N	Vorthbou	rne Avenue (	(N)								
7	L	25	4.2	0.579	19.8	LOS B	11.0	78.3	0.75	0.93	41.2
8	Т	1562	1.7	0.579	11.5	LOS B	11.1	78.5	0.75	0.67	43.1
Approa	ch	1587	1.7	0.579	11.6	LOS B	11.1	78.5	0.75	0.67	43.1
All Vehi	icles	3279	1.9	0.579	8.9	LOS A	11.1	78.5	0.48	0.45	46.9

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (HCM 2000). Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street 2012 EV

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Northbou	Irne Avenue	(S)								
2	Т	722	1.5	0.131	0.0	Х	Х	Х	Х	0.00	60.0
3	R	315	0.3	0.448	21.8	LOS C	6.9	48.2	0.78	0.79	37.9
Approach		1037	1.1	0.448	6.6	LOS A	6.9	48.2	0.24	0.24	50.8
East: M	orphett S	Street (E)									
4	L	191	0.0	0.189	9.9	LOS A	1.6	11.0	0.40	0.70	47.1
Approa	ch	191	0.0	0.189	9.9	LOS A	1.6	11.0	0.40	0.70	47.1
North: N	Vorthbou	rne Avenue (	(N)								
7	L	18	0.0	0.434	22.5	LOS C	6.9	48.6	0.77	0.90	39.1
8	Т	938	1.0	0.434	14.3	LOS B	6.9	48.7	0.77	0.66	40.8
Approa	ch	956	1.0	0.434	14.5	LOS B	6.9	48.7	0.77	0.66	40.8
All Vehi	cles	2183	1.0	0.448	10.4	LOS B	6.9	48.7	0.48	0.46	45.6

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (HCM 2000). Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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## APPENDIX E 2031 MASTERPLAN (RECOMMENDED) SIDRA RESULTS

#### **MOVEMENT SUMMARY**

Site: 3 AM - Conversion

Antill Street - Challis Street Master Plan 2031 AM Signals - Fixed Time Cycle Time = 40 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Pe	erformance	- Vehic	cles							
Mov ID	Turn	Demand Flow	HV C	0eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: C	Challis S	treet (S)									
1	L	497	9.3	0.742	19.3	LOS B	7.1	53.6	0.90	0.93	39.5
Approac	ch	497	9.3	0.742	19.3	LOS B	7.1	53.6	0.90	0.93	39.5
East: Antil Street (W)		et (W)									
4	L	93	20.5	0.159	11.7	LOS B	0.7	6.0	0.54	0.69	45.8
5	Т	1398	3.0	0.906	25.4	LOS C	19.5	139.9	1.00	1.22	33.4
Approac	ch	1491	4.1	0.906	24.6	LOS C	19.5	139.9	0.97	1.19	34.0
West: A	ntill Stre	et									
11	Т	1269	1.2	0.494	1.6	LOS A	3.0	21.5	0.49	0.27	51.6
12	R	406	15.0	0.929	25.0	LOS C	8.3	65.7	1.00	0.86	35.9
Approac	ch	1676	4.5	0.929	7.2	LOS A	8.3	65.7	0.61	0.41	46.6
All Vehic	cles	3663	5.0	0.929	15.9	LOS B	19.5	139.9	0.80	0.80	39.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 AM Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	e - Vehio	cles							
Mov ID	Turn	Demand Flow	HV C	0eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
1	L	562	0.0	0.732	16.0	LOS B	8.0	56.0	0.62	0.83	41.9
3	R	69	12.1	0.428	38.2	LOS D	2.1	16.2	0.98	0.76	29.3
Approa	ch	632	1.3	0.732	18.4	LOS B	8.0	56.0	0.66	0.82	40.0
East: A	ntill Stree	et (W)									
4	L	93	0.0	0.138	12.8	LOS B	1.1	7.9	0.50	0.68	44.3
5	Т	1006	6.1	0.998	66.7	LOS E	26.8	197.4	1.00	1.48	20.5
Approa	ch	1099	5.6	0.998	62.2	LOS E	26.8	197.4	0.96	1.41	21.5
West: A	ntill Stre	et (W)									
11	Т	961	0.0	0.378	4.0	LOS A	5.8	40.9	0.44	0.39	51.5
<mark>12</mark>	R	<mark>416</mark>	3.1	<mark>1.000</mark> 3	28.7	LOS C	11.4	81.6	0.96	0.85	33.6
Approa	ch	1377	1.5	1.000	11.4	LOS B	11.4	81.6	0.60	0.53	44.4
All Vehi	icles	3107	2.9	1.000	30.8	LOS C	26.8	197.4	0.74	0.90	31.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

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Challis Street - Cape Street Master Plan 2031 AM

Signals - Fixed Time Cycle Time = 100 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	40	0.0	0.508	33.5	LOS C	12.6	89.9	0.81	0.92	25.9
2	Т	701	2.4	0.936	41.4	LOS D	40.3	285.8	0.91	0.95	20.1
3	R	267	0.0	0.936	63.1	LOS E	40.3	285.8	1.00	1.12	16.5
Approa	ich	1008	1.7	0.936	46.8	LOS D	40.3	285.8	0.93	0.99	19.2
East: C	ape Stre	et									
4	L	269	0.0	0.932	42.4	LOS D	23.3	163.2	0.97	1.07	28.5
5	Т	699	0.0	0.932	48.3	LOS D	26.3	198.1	0.99	1.11	24.4
6	R	51	75.0	0.932	72.0	LOS E	26.3	198.1	1.00	1.16	21.4
Approa	ich	1019	3.7	0.932	47.9	LOS D	26.3	198.1	0.98	1.10	25.2
North:	Challis S	treet									
7	L	42	100.0	0.860	58.3	LOS E	14.0	117.2	1.00	1.11	20.6
8	Т	194	1.1	0.860	47.2	LOS D	14.0	117.2	1.00	1.11	18.6
9	R	99	31.9	0.860	56.7	LOS E	14.0	117.2	1.00	1.11	18.5
Approa	ich	335	22.6	0.860	51.4	LOS D	14.0	117.2	1.00	1.11	18.9
West:	Cape Stre	eet									
10	L	55	0.0	0.779	22.2	LOS C	7.9	55.1	0.47	0.78	22.8
11	Т	1	0.0	0.779	14.4	LOS B	7.9	55.1	0.47	0.46	25.8
12	R	261	0.0	0.779	22.5	LOS C	7.9	55.1	0.47	0.81	22.5
Approa	ich	317	0.0	0.779	22.4	LOS C	7.9	55.1	0.47	0.81	22.6
All Veh	icles	2679	4.9	0.936	44.9	LOS D	40.3	285.8	0.91	1.03	22.0

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street

Master Plan 2031 AM

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbou	Irne Avenue	(S)								
2	Т	1090	13.7	0.213	0.0	Х	Х	Х	Х	0.00	60.0
<mark>3</mark>	R	<mark>342</mark>	3.0	<mark>1.000</mark> 3	56.8	LOS E	20.5	146.9	1.00	0.86	23.8
Approa	ch	1432	9.4	1.000	13.6	LOS B	20.5	146.9	0.24	0.21	43.8
East: M	orphett S	Street (E)									
4	L	162	3.9	0.278	46.7	LOS D	8.6	61.8	0.77	0.80	26.3
Approa	ch	162	3.9	0.278	46.7	LOS D	8.6	61.8	0.77	0.80	26.3
North: N	Vorthbou	rne Avenue	(N)								
7	L	99	2.1	0.993	87.4	LOS F	101.6	748.3	1.00	1.17	18.2
8	Т	2966	6.5	0.993	79.1	LOS E	101.8	752.2	1.00	1.18	18.3
Approa	ch	3065	6.3	0.993	79.4	LOS E	101.8	752.2	1.00	1.18	18.3
All Vehi	cles	4659	7.2	1.000	58.0	LOS E	101.8	752.2	0.76	0.86	22.6

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

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Morphett Street - Challis Street Master Plan 2031 AM Signals - Fixed Time Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	141	4.5	0.115	4.9	LOS A	1.9	13.9	0.37	0.31	51.3
6	R	444	0.0	0.798	31.2	LOS C	14.8	103.5	0.78	0.89	32.2
Approa	ch	585	1.1	0.798	24.9	LOS C	14.8	103.5	0.69	0.75	35.4
North: C	Challis St	treet									
7	L	764	0.3	0.529	9.0	LOS A	6.4	44.8	0.35	0.70	47.9
9	R	21	0.0	0.078	38.5	LOS D	0.7	4.9	0.86	0.71	29.1
Approa	ch	785	0.3	0.529	9.8	LOS A	6.4	44.8	0.37	0.70	47.1
West: N	/lorphett	Street (W)									
10	L	461	3.7	0.763	21.5	LOS C	16.3	117.2	0.86	0.95	38.5
11	Т	211	1.0	0.763	13.5	LOS B	16.3	117.2	0.86	0.85	39.5
Approa	ch	672	2.8	0.763	19.0	LOS B	16.3	117.2	0.86	0.92	38.8
All Vehi	icles	2042	1.3	0.798	17.1	LOS B	16.3	117.2	0.62	0.78	40.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestria	ns					
		Demand	Average			ck of Queue	Prop	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P3	Across E approach	53	34.2	LOS D	0.1	0.1	0.93	0.93
P5	Across N approach	53	33.3	LOS D	0.1	0.1	0.91	0.91
All Pede	estrians	106	33.8	LOS D			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Cowper Street - Morphett Street Master Plan 2031 AM Signals - Fixed Time Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID		Demand Flow		eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	234	0.0	0.896	44.6	LOS D	31.9	236.7	0.99	1.17	27.9
2	Т	503	10.0	0.896	36.7	LOS D	31.9	236.7	0.99	1.16	28.0
Approa	ch	737	6.9	0.896	39.2	LOS D	31.9	236.7	0.99	1.17	28.0
North: C	Cowper S	Street (N)									
8	Т	541	1.6	0.441	6.9	LOS A	10.0	70.9	0.51	0.46	48.4
9	R	322	2.0	0.917	58.9	LOS E	16.0	113.7	1.00	1.03	22.8
Approa	ch	863	1.7	0.917	26.3	LOS C	16.0	113.7	0.69	0.67	34.2
West: N	/lorphett	Street									
10	L	160	1.3	0.880	48.6	LOS D	17.7	126.4	1.00	1.12	25.6
12	R	297	2.8	0.880	49.1	LOS D	17.7	126.4	1.00	1.12	25.6
Approa	ch	457	2.3	0.880	48.9	LOS D	17.7	126.4	1.00	1.12	25.6
All Vehi	cles	2057	3.7	0.917	35.9	LOS D	31.9	236.7	0.87	0.95	29.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestria	ns					
		Demand	Average			ck of Queue	Prop	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P5	Across N approach	53	33.3	LOS D	0.1	0.1	0.91	0.91
P7	Across W approach	53	18.2	LOS B	0.1	0.1	0.68	0.68
All Pede	estrians	106	25.8	LOS C			0.79	0.79

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Challis Street Master Plan 2031 PM

Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Pe	rformance	e - Vehi	cles							
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	280	24.8	0.621	15.9	LOS B	5.0	42.6	0.83	0.84	42.3
Approa	ch	280	24.8	0.621	15.9	LOS B	5.0	42.6	0.83	0.84	42.3
East: A	ntil Stree	t (W)									
4	L	21	80.0	0.042	11.2	LOS B	0.1	1.2	0.32	0.63	48.0
5	Т	1413	0.0	0.616	6.4	LOS A	10.7	75.2	0.66	0.59	48.4
Approa	ch	1434	1.2	0.616	6.4	LOS A	10.7	75.2	0.66	0.59	48.4
West: A	Antill Stre	et									
11	Т	857	1.2	0.307	1.1	LOS A	1.8	12.7	0.33	0.19	54.0
12	R	86	48.8	0.472	33.4	LOS C	2.1	21.3	0.96	0.77	32.1
Approa	ch	943	5.6	0.472	4.0	LOS A	2.1	21.3	0.39	0.24	50.8
All Vehi	icles	2657	5.2	0.621	6.6	LOS A	10.7	75.2	0.58	0.49	48.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 PM Signals - Fixed Time Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Badham	Street (S)									
1	L	653	0.0	0.910	18.2	LOS B	11.7	81.6	0.64	0.85	40.2
3	R	76	11.1	0.619	51.1	LOS D	3.2	24.4	1.00	0.81	25.0
Approa	ch	728	1.2	0.910	21.6	LOS C	11.7	81.6	0.67	0.85	37.8
East: A	ntill Stree	et (W)									
4	L	76	0.0	0.148	11.4	LOS B	0.9	6.3	0.39	0.67	45.7
5	Т	861	2.0	0.786	31.4	LOS C	16.9	120.1	0.98	0.93	30.6
Approa	ch	937	1.8	0.786	29.8	LOS C	16.9	120.1	0.93	0.91	31.5
West: A	Antill Stre	et (W)									
11	Т	537	1.6	0.378	3.0	LOS A	6.6	46.7	0.34	0.31	53.7
12	R	455	0.5	0.647	29.1	LOS C	14.5	102.1	0.87	0.85	33.3
Approa	ch	992	1.1	0.647	15.0	LOS B	14.5	102.1	0.58	0.55	42.0
All Vehi	icles	2657	1.3	0.910	22.0	LOS C	16.9	120.1	0.73	0.76	36.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Challis Street - Cape Street Master Plan 2031 PM Signals - Eived Time - Oueconde (C

Signals - Fixed Time Cycle Time = 90 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	street (S)									
1	L	1	0.0	0.325	24.9	LOS C	6.5	52.2	0.66	0.90	30.7
2	Т	238	17.7	0.325	17.1	LOS B	6.5	52.2	0.66	0.57	32.6
3	R	667	0.0	0.803	33.4	LOS C	26.9	188.0	0.92	0.91	24.5
Approa	ch	906	4.6	0.803	29.1	LOS C	26.9	188.0	0.85	0.82	26.2
East: C	ape Stre	et									
4	L	137	0.0	0.761	39.9	LOS D	11.2	79.3	0.98	1.00	29.5
5	Т	307	1.4	0.761	36.3	LOS D	11.2	79.3	0.99	0.96	28.0
6	R	63	53.3	0.761	56.2	LOS E	7.5	62.6	1.00	0.91	24.8
Approa	ch	507	7.5	0.761	39.8	LOS D	11.2	79.3	0.99	0.97	27.9
North:	Challis S	treet									
7	L	42	100.0	0.426	21.0	LOS C	5.0	42.6	0.65	0.78	34.7
8	Т	46	0.0	0.426	9.9	LOS A	5.0	42.6	0.65	0.55	36.8
9	R	135	12.5	0.426	18.7	LOS B	5.0	42.6	0.65	0.82	34.4
Approa	ch	223	26.4	0.426	17.3	LOS B	5.0	42.6	0.65	0.76	34.9
West: 0	Cape Stre	eet									
10	L	2	0.0	0.262	9.9	LOS A	0.5	3.6	0.28	0.65	34.6
11	Т	1	0.0	0.262	2.0	LOS A	0.5	3.6	0.28	0.21	44.9
12	R	57	0.0	0.262	10.2	LOS B	0.5	3.6	0.28	0.69	34.2
Approa	ch	60	0.0	0.262	10.0	LOS B	0.5	3.6	0.28	0.68	34.3
All Veh	icles	1697	8.2	0.803	30.1	LOS C	26.9	188.0	0.85	0.85	27.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 PM

Signals - Fixed Time Cycle Time = 50 seconds (Practical Cycle Time)

Movem	ent Pe	rformance	- Vehic	les							l.
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	lorthbou	rne Avenue	(S)								
2	Т	2362	3.8	0.436	0.1	Х	Х	Х	Х	0.00	59.9
3	R	562	6.0	0.832	28.6	LOS C	15.4	113.3	0.96	1.02	34.1
Approac	h	2924	4.2	0.832	5.6	LOS A	15.4	113.3	0.18	0.20	52.1
East: Mo	orphett S	Street (E)									
4	L	57	0.0	0.064	12.8	LOS B	0.6	4.3	0.55	0.68	44.3
Approac	h	57	0.0	0.064	12.8	LOS B	0.6	4.3	0.55	0.68	44.3
North: N	orthbou	rne Avenue (	N)								
7	L	36	0.0	0.857	32.4	LOS C	16.1	116.2	0.99	1.08	33.7
8	Т	1634	3.9	0.857	24.2	LOS C	16.1	116.6	0.99	1.07	34.0
Approac	h	1669	3.8	0.857	24.4	LOS C	16.1	116.6	0.99	1.07	34.0
All Vehic	les	4651	4.0	0.857	12.4	LOS B	16.1	116.6	0.48	0.52	43.7

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 PM Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	42	0.0	0.040	6.1	LOS A	0.5	3.8	0.46	0.35	49.4
6	R	272	2.3	0.547	28.7	LOS C	7.0	49.9	0.90	0.82	33.5
Approa	ch	314	2.0	0.547	25.7	LOS C	7.0	49.9	0.84	0.76	35.0
North: C	Challis St	reet									
7	L	181	0.0	0.127	8.3	LOS A	0.6	4.4	0.24	0.65	48.5
9	R	15	0.0	0.040	27.7	LOS C	0.3	2.4	0.80	0.70	34.0
Approa	ch	196	0.0	0.127	9.8	LOS A	0.6	4.4	0.28	0.66	47.0
West: N	lorphett	Street (W)									
10	L	499	6.8	0.536	11.3	LOS B	6.9	50.6	0.55	0.78	46.1
11	Т	99	0.0	0.536	3.3	LOS A	6.9	50.6	0.55	0.44	48.7
Approa	ch	598	5.6	0.536	10.0	LOS B	6.9	50.6	0.55	0.73	46.5
All Vehi	cles	1107	3.6	0.547	14.4	LOS B	7.0	50.6	0.58	0.72	42.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Movem	nent Performance -	Pedestria	ns					
		Demand	Average			ck of Queue	Prop	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P3	Across E approach	53	24.3	LOS C	0.1	0.1	0.90	0.90
P5	Across N approach	53	24.3	LOS C	0.1	0.1	0.90	0.90
All Pede	estrians	106	24.3	LOS C			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Cowper Street - Morphett Street Master Plan 2031 PM Signals - Fixed Time Cycle Time = 100 seconds (Optimum Cycle Time - Minimum Delay)

				_							
Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	44	0.0	0.586	25.6	LOS C	18.5	131.3	0.65	1.20	37.6
2	Т	619	1.7	0.586	14.7	LOS B	18.5	131.3	0.65	0.63	40.7
Approa	ch	663	1.6	0.586	15.4	LOS B	18.5	131.3	0.65	0.67	40.5
North: C	Cowper S	Street (N)									
8	Т	244	4.3	0.181	4.1	LOS A	3.5	25.4	0.32	0.27	52.4
9	R	84	0.0	0.591	59.7	LOS E	4.3	29.8	1.00	0.79	22.6
Approa	ch	328	3.2	0.591	18.4	LOS B	4.3	29.8	0.49	0.41	39.2
West: N	/lorphett	Street									
10	L	194	1.1	0.282	12.9	LOS B	4.1	28.7	0.46	0.73	44.2
12	R	48	0.0	0.282	13.3	LOS B	4.1	28.7	0.46	0.78	44.1
Approa	ch	242	0.9	0.282	13.0	LOS B	4.1	28.7	0.46	0.74	44.2
All Vehi	icles	1234	1.9	0.591	15.7	LOS B	18.5	131.3	0.57	0.61	40.8

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestria	ns					
		Demand	Average			ck of Queue	Prop	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P5	Across N approach	53	44.2	LOS E	0.1	0.1	0.94	0.94
P7	Across W approach	53	10.1	LOS B	0.1	0.1	0.45	0.45
All Pede	estrians	106	27.2	LOS C			0.70	0.70

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Challis Street Master Plan 2031 MD

Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Pe	erformance	e - Vehi	cles							
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	173	19.5	0.339	10.8	LOS B	1.7	13.6	0.45	0.71	46.7
Approa	ch	173	19.5	0.339	10.8	LOS B	1.7	13.6	0.45	0.71	46.7
East: A	ntil Stree	t (W)									
4	L	17	100.0	0.037	11.3	LOS B	0.1	1.0	0.25	0.62	46.4
5	Т	1088	0.4	0.432	4.6	LOS A	7.2	50.3	0.48	0.43	51.1
Approa	ch	1105	1.9	0.432	4.7	LOS A	7.2	50.3	0.48	0.43	51.0
West: A	Antill Stre	et									
11	Т	939	1.3	0.320	0.9	LOS A	2.0	14.4	0.29	0.16	54.8
12	R	63	56.7	0.432	39.5	LOS D	1.9	19.8	0.97	0.76	29.6
Approa	ch	1002	4.8	0.432	3.3	LOS A	2.0	19.8	0.33	0.20	52.0
All Vehi	icles	2280	4.5	0.432	4.6	LOS A	7.2	50.3	0.41	0.35	51.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 MD Signals - Fixed Time Cycle Time = 60 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	642	0.0	0.696	10.7	LOS B	5.7	40.2	0.47	0.74	46.3
3	R	95	6.7	0.563	38.6	LOS D	2.9	21.6	1.00	0.80	29.1
Approa	ch	737	0.9	0.696	14.3	LOS B	5.7	40.2	0.54	0.75	43.1
East: Ar	ntill Stree	et (W)									
4	L	109	0.0	0.170	11.6	LOS B	1.2	8.1	0.47	0.69	45.4
5	Т	564	3.0	0.933	43.7	LOS D	11.3	80.9	1.00	1.19	26.1
Approa	ch	674	2.5	0.933	38.5	LOS D	11.3	80.9	0.91	1.11	28.1
West: A	ntill Stre	et (W)									
11	Т	555	2.8	0.221	3.4	LOS A	2.9	20.9	0.38	0.32	52.2
<mark>12</mark>	R	<mark>494</mark>	0.3	<mark>1.000</mark> 3	25.0	LOS C	11.6	81.6	0.97	0.86	35.6
Approa	ch	1048	1.0	1.000	13.6	LOS B	11.6	81.6	0.66	0.58	42.8
All Vehi	cles	2459	1.4	1.000	20.6	LOS C	11.6	81.6	0.69	0.77	37.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

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Challis Street - Cape Street Master Plan 2031 MD

Signals - Fixed Time Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov ID	Turn	Demand	HV C	0eg. Satn	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
		Flow			Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	1	0.0	0.201	28.8	LOS C	3.3	24.9	0.75	0.86	28.4
2	Т	118	10.7	0.201	20.9	LOS C	3.3	24.9	0.75	0.61	29.7
3	R	408	0.0	0.680	33.3	LOS C	14.0	97.9	0.92	0.85	24.5
Approa	ch	527	2.4	0.680	30.5	LOS C	14.0	97.9	0.88	0.80	25.5
East: C	ape Stre	et									
4	L	88	0.0	0.663	33.6	LOS C	9.7	67.8	0.94	0.94	32.5
5	Т	423	0.0	0.663	28.6	LOS C	9.7	69.5	0.95	0.86	31.4
6	R	69	42.4	0.663	43.0	LOS D	9.0	69.5	0.97	0.87	29.2
Approa	ch	581	5.1	0.663	31.1	LOS C	9.7	69.5	0.95	0.88	31.3
North: (	Challis St	treet									
7	L	40	100.0	0.156	14.7	LOS B	1.2	12.4	0.40	0.69	39.2
8	Т	6	0.0	0.156	3.7	LOS A	1.2	12.4	0.40	0.32	46.2
9	R	61	20.7	0.156	12.7	LOS B	1.2	12.4	0.40	0.76	40.0
Approa	ch	107	49.0	0.156	12.9	LOS B	1.2	12.4	0.40	0.71	40.0
West: 0	Cape Stre	eet									
10	L	1	0.0	0.244	9.1	LOS A	0.4	2.7	0.25	0.64	35.8
11	Т	1	0.0	0.244	1.3	LOS A	0.4	2.7	0.25	0.18	46.3
12	R	61	0.0	0.244	9.4	LOS A	0.4	2.7	0.25	0.69	35.3
Approa	ch	63	0.0	0.244	9.3	LOS A	0.4	2.7	0.25	0.68	35.4
All Veh		1279	7.4	0.680	28.3	LOS C	14.0	97.9	0.84	0.82	29.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 MD

Signals - Fixed Time Cycle Time = 40 seconds (Practical Cycle Time)

Moven	oont Pe	erformance	- Vehic	los							
Mov ID		Demand Flow		eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: N	Northbou	Irne Avenue	(S)								
2	Т	1002	4.0	0.185	0.0	Х	Х	Х	Х	0.00	60.0
3	R	423	3.0	0.755	23.5	LOS C	8.6	62.0	0.95	0.95	36.9
Approad	ch	1425	3.7	0.755	7.0	LOS A	8.6	62.0	0.28	0.28	50.4
East: Ro	oadNam	е									
4	L	11	0.0	0.013	11.8	LOS B	0.1	0.6	0.57	0.64	45.2
Approad	ch	11	0.0	0.013	11.8	LOS B	0.1	0.6	0.57	0.64	45.2
North: N	lorthbou	rne Avenue	(N)								
7	L	13	16.7	0.793	25.0	LOS C	11.4	81.0	0.95	1.01	38.5
8	Т	1615	2.0	0.793	16.1	LOS B	11.4	81.0	0.95	0.96	39.0
Approad	ch	1627	2.1	0.793	16.2	LOS B	11.4	81.0	0.95	0.96	39.0
All Vehi	cles	3063	2.8	0.793	11.9	LOS B	11.4	81.0	0.64	0.64	43.7

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (HCM 2000). Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 MD Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	11	0.0	0.012	7.3	LOS A	0.1	0.9	0.54	0.37	47.9
6	R	109	0.0	0.439	31.2	LOS C	2.6	18.5	0.96	0.78	32.2
Approa	ch	120	0.0	0.439	29.1	LOS C	2.6	18.5	0.92	0.74	33.2
North: C	Challis St	reet									
7	L	95	0.0	0.070	8.4	LOS A	0.3	2.1	0.27	0.65	48.3
9	R	1	0.0	0.002	22.2	LOS C	0.0	0.1	0.73	0.62	37.2
Approa	ch	96	0.0	0.070	8.6	LOS A	0.3	2.1	0.27	0.65	48.2
West: N	lorphett	Street (W)									
10	L	377	3.4	0.345	9.5	LOS A	2.4	17.6	0.39	0.74	47.7
11	Т	59	3.6	0.345	1.5	LOS A	2.4	17.6	0.39	0.27	51.5
Approa	ch	436	3.4	0.345	8.4	LOS A	2.4	17.6	0.39	0.67	48.2
All Vehi	cles	652	2.3	0.439	12.2	LOS B	2.6	18.5	0.47	0.68	44.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	nent Performance -	Pedestria	ns					
		Demand	Average			ck of Queue	Prop	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P3	Across E approach	53	19.4	LOS B	0.1	0.1	0.88	0.88
P5	Across N approach	53	19.4	LOS B	0.1	0.1	0.88	0.88
All Pede	estrians	106	19.4	LOS B			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Cowper Street - Morphett Street Master Plan 2031 MD Signals - Fixed Time Cycle Time = 90 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	erformance	e - Vehio	cles							
Mov ID	Turn	Demand Flow	HV [	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Cowper \$	Street (S)									
1	L	13	0.0	0.531	25.8	LOS C	14.5	103.3	0.69	0.94	36.7
2	Т	514	2.0	0.531	18.0	LOS B	14.5	103.3	0.69	0.64	38.4
Approa	ch	526	2.0	0.531	18.2	LOS B	14.5	103.3	0.69	0.65	38.4
North: 0	Cowper S	Street (N)									
8	Т	286	3.7	0.221	4.7	LOS A	4.2	30.6	0.37	0.32	51.5
9	R	101	2.1	0.518	51.4	LOS D	4.4	31.5	0.99	0.78	24.8
Approa	ch	387	3.3	0.518	16.9	LOS B	4.4	31.5	0.53	0.44	40.2
West: N	/lorphett	Street									
10	L	122	0.0	0.165	11.7	LOS B	2.0	14.1	0.39	0.71	45.3
12	R	34	0.0	0.165	12.1	LOS B	2.0	14.1	0.39	0.76	45.1
Approa	ch	156	0.0	0.165	11.8	LOS B	2.0	14.1	0.39	0.72	45.3
All Vehi	icles	1069	2.2	0.531	16.8	LOS B	14.5	103.3	0.59	0.58	39.9

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Movem	nent Performance -	Pedestria	ns					
		Demand	Average			ck of Queue	Prop	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P5	Across N approach	53	39.2	LOS D	0.1	0.1	0.93	0.93
P7	Across W approach	53	12.3	LOS B	0.1	0.1	0.52	0.52
All Pede	estrians	106	25.7	LOS C			0.73	0.73

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Antill Street - Challis Street Master Plan 2031 EV Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

				-							1
Moven	nent Pe	erformance	e - Vehic	les							
Mov ID	Turn	Demand Flow	HV C	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Challis S	Street (S)									
1	L	196	11.8	0.284	10.5	LOS B	1.6	12.0	0.47	0.71	46.8
Approa	ch	196	11.8	0.284	10.5	LOS B	1.6	12.0	0.47	0.71	46.8
East: A	ntil Stree	et (W)									
4	L	8	100.0	0.019	11.5	LOS B	0.0	0.5	0.29	0.61	46.1
5	Т	853	0.2	0.385	5.7	LOS A	5.5	38.4	0.56	0.48	49.6
Approa	ch	861	1.2	0.385	5.7	LOS A	5.5	38.4	0.55	0.49	49.6
West: A	ntill Stre	et									
11	Т	634	0.3	0.226	1.0	LOS A	1.2	8.5	0.31	0.17	54.4
12	R	88	28.6	0.378	31.0	LOS C	2.1	18.2	0.93	0.77	32.9
Approa	ch	722	3.8	0.378	4.7	LOS A	2.1	18.2	0.38	0.24	50.4
All Vehi	icles	1779	3.4	0.385	5.8	LOS A	5.5	38.4	0.48	0.41	49.6

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Antill Street - Badham Street Master Plan 2031 EV Signals - Fixed Time Cycle Time = 40 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: E	Badham	Street (S)									
1	L	493	0.0	0.477	9.7	LOS A	2.9	20.4	0.51	0.72	47.1
3	R	147	2.9	0.569	26.9	LOS C	3.0	21.3	0.97	0.82	34.4
Approa	ch	640	0.7	0.569	13.7	LOS B	3.0	21.3	0.61	0.74	43.5
East: Ar	ntill Stree	et (W)									
4	L	204	0.0	0.241	10.8	LOS B	1.5	10.8	0.54	0.71	46.1
5	Т	476	2.2	0.870	25.6	LOS C	5.9	41.8	1.00	1.07	33.3
Approa	ch	680	1.5	0.870	21.2	LOS C	5.9	41.8	0.86	0.96	36.4
West: A	ntill Stre	et (W)									
11	Т	402	0.9	0.202	5.0	LOS A	2.1	14.5	0.54	0.44	49.4
<mark>12</mark>	R	<mark>440</mark>	0.0	<mark>1.000</mark> 3	36.1	LOS D	11.7	81.6	1.00	1.05	30.2
Approa	ch	842	0.3	1.000	21.2	LOS C	11.7	81.6	0.78	0.76	37.0
All Vehi	cles	2162	0.8	1.000	19.0	LOS B	11.7	81.6	0.76	0.82	38.5

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

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Challis Street - Cape Street Master Plan 2031 EV

Signals - Fixed Time Cycle Time = 80 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Pe	erformance	e - Vehic	les							
Mov II	D Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Challis S	treet (S)									
1	L	4	0.0	0.199	25.6	LOS C	3.3	24.5	0.71	0.87	30.3
2	Т	131	6.5	0.199	17.8	LOS B	3.3	24.5	0.71	0.59	31.9
3	R	474	0.0	0.710	32.0	LOS C	16.2	113.3	0.91	0.87	25.1
Approa	ach	608	1.4	0.710	28.9	LOS C	16.2	113.3	0.87	0.81	26.3
East: (	Cape Stre	et									
4	L	177	0.0	0.699	28.9	LOS C	10.1	70.9	0.92	0.94	34.5
5	Т	381	0.0	0.699	26.7	LOS C	10.1	70.9	0.95	0.88	32.0
6	R	67	28.1	0.699	45.7	LOS D	8.3	62.7	0.99	0.88	27.9
Approa	ach	625	3.0	0.699	29.4	LOS C	10.1	70.9	0.94	0.90	32.2
North:	Challis S	treet									
7	L	23	100.0	0.223	21.3	LOS C	2.2	18.4	0.73	0.78	35.8
8	Т	63	3.3	0.223	10.2	LOS B	2.2	18.4	0.73	0.59	37.2
9	R	48	17.4	0.223	19.2	LOS B	2.2	18.4	0.73	0.81	34.9
Approa	ach	135	25.0	0.223	15.4	LOS B	2.2	18.4	0.73	0.70	36.1
West:	Cape Stre	eet									
10	L	1	0.0	0.627	11.4	LOS B	1.4	9.7	0.27	0.68	32.7
11	Т	1	0.0	0.627	3.6	LOS A	1.4	9.7	0.27	0.24	42.4
12	R	160	0.0	0.627	11.7	LOS B	1.4	9.7	0.27	0.72	32.1
Approa	ach	162	0.0	0.627	11.7	LOS B	1.4	9.7	0.27	0.72	32.2
All Vel	nicles	1531	4.0	0.710	26.1	LOS C	16.2	113.3	0.82	0.83	30.2

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

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Northbourne Avenue - Morphett Street Master Plan 2031 EV

Signals - Fixed Time Cycle Time = 40 seconds (Practical Cycle Time)

Moven	nent Pe	erformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV Deg. Satn		Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: I	Northbou	Irne Avenue	(S)								
2	Т	691	2.7	0.127	0.0	Х	Х	Х	Х	0.00	60.0
3	R	484	1.7	0.795	24.3	LOS C	10.4	73.6	0.96	0.99	36.3
Approa	ch	1175	2.3	0.795	10.0	LOS B	10.4	73.6	0.40	0.41	47.1
East: M	orphett S	Street (E)									
4	L	6	0.0	0.007	11.3	LOS B	0.0	0.3	0.54	0.63	45.7
Approa	ch	6	0.0	0.007	11.3	LOS B	0.0	0.3	0.54	0.63	45.7
North: N	lorthbou	rne Avenue (	(N)								
7	L	27	0.0	0.784	24.6	LOS C	10.5	74.1	0.95	1.00	38.2
8	Т	1480	1.4	0.784	16.4	LOS B	10.5	74.3	0.95	0.95	38.8
Approa	ch	1507	1.4	0.784	16.5	LOS B	10.5	74.3	0.95	0.95	38.8
All Vehi	cles	2688	1.8	0.795	13.7	LOS B	10.5	74.3	0.71	0.71	42.1

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (HCM 2000). Vehicle movement LOS values are based on average delay per movement Intersection and Approach LOS values are based on average delay for all vehicle movements. SIDRA Standard Delay Model used.

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Morphett Street - Challis Street Master Plan 2031 EV Signals - Fixed Time Cycle Time = 50 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: M	orphett S	Street (E)									
5	Т	6	0.0	0.007	7.2	LOS A	0.1	0.6	0.54	0.36	48.0
6	R	122	0.0	0.490	31.4	LOS C	3.0	20.8	0.97	0.78	32.1
Approa	ch	128	0.0	0.490	30.2	LOS C	3.0	20.8	0.95	0.76	32.7
North: C	Challis St	reet									
7	L	185	1.1	0.137	8.5	LOS A	0.6	4.5	0.28	0.66	48.3
9	R	1	0.0	0.002	22.2	LOS C	0.0	0.1	0.73	0.62	37.2
Approa	ch	186	1.1	0.137	8.6	LOS A	0.6	4.5	0.29	0.66	48.2
West: N	/lorphett	Street (W)									
10	L	429	2.0	0.402	9.7	LOS A	3.2	22.7	0.42	0.75	47.4
11	Т	76	0.0	0.402	1.8	LOS A	3.2	22.7	0.42	0.31	50.8
Approa	ch	505	1.7	0.402	8.5	LOS A	3.2	22.7	0.42	0.68	47.9
All Vehi	icles	820	1.3	0.490	11.9	LOS B	3.2	22.7	0.47	0.69	44.7

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians												
		Demand	Average			ck of Queue	Prop	Effective					
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate					
		ped/h	sec		ped	ped m		per ped					
P3	Across E approach	53	19.4	LOS B	0.1	0.1	0.88	0.88					
P5	Across N approach	53	19.4	LOS B	0.1	0.1	0.88	0.88					
All Pedestrians		106	19.4	LOS B			0.88	0.88					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Cowper Street - Morphett Street Master Plan 2031 EV Signals - Fixed Time Cycle Time = 70 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance	- Vehic	les							
Mov ID	Turn	Demand Flow	HV D	eg. Satn	Average Delay	Level of Service	95% Back Vehicles	% Back of Queue hicles Distance		Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: (	Cowper S	Street (S)									
1	L	15	0.0	0.569	28.9	LOS C	11.3	79.9	0.80	0.93	35.1
2	Т	432	1.0	0.569	21.0	LOS C	11.3	79.9	0.80	0.74	36.2
Approa	ch	446	0.9	0.569	21.3	LOS C	11.3	79.9	0.80	0.75	36.1
North: C	Cowper S	Street (N)									
8	Т	213	2.0	0.183	5.8	LOS A	3.0	21.5	0.45	0.38	49.8
9	R	131	0.0	0.570	41.4	LOS D	4.5	31.4	0.99	0.80	27.9
Approa	ch	343	1.2	0.570	19.4	LOS B	4.5	31.4	0.65	0.54	38.4
West: N	/lorphett	Street									
10	L	93	0.0	0.128	11.1	LOS B	1.3	9.3	0.40	0.70	45.8
12	R	36	0.0	0.128	11.5	LOS B	1.3	9.3	0.40	0.75	45.6
Approa	ch	128	0.0	0.128	11.2	LOS B	1.3	9.3	0.40	0.71	45.8
All Vehi	icles	918	0.9	0.570	19.2	LOS B	11.3	79.9	0.69	0.67	38.1

Level of Service (LOS) Method: Delay & v/c (HCM 2010).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection). Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010). SIDRA Standard Delay Model used.

Mover	Movement Performance - Pedestrians													
		Demand	Average	Level of		ck of Queue	Prop	Effective						
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate						
		ped/h	sec		ped	ped m		per ped						
P5	Across N approach	53	29.3	LOS C	0.1	0.1	0.91	0.91						
P7	Across W approach	53	15.1	LOS B	0.1	0.1	0.66	0.66						
All Pedestrians		106	22.2	LOS C			0.79	0.79						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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### F.1 Short Term Cost Estimates

Item	Unit	Quantity	Rate	Amount
Partially Signalise Intersection				
Detector Loops	Each	4	\$700	\$2,800
Light columns and fittings	Each	5	\$5,500	\$27,500
Cabling	Item	1	\$15,000	\$15,000
Linemarking	Item	1	\$3,000	\$3,000
Signage	Each	4	\$200	\$800
Controller	Each	1	\$25,000	\$25,000
Adjust Intersetion Geometry	Item	1	\$37,500	\$37,500
Fees to ActewAGL	Item	1	\$8,500	\$8,500
Pedestrian Crossings (5 locations)				
Pram Ramps (incl demolition of existing kerb)	Each	8	\$250	\$2,000
Linemarking	m²	100	\$100	\$10,000
Signage	Each	16	\$200	\$3,200
New Path (1.2m wide, 100mm thick reinforced)	m²	500	\$140	\$70,000
Widen Existing Path				
Demolish Existing Path	m <sup>2</sup>	600	\$25	\$15,000
Install New Path (2m wide, 100mm thick reinforced)	m <sup>2</sup>	1,000	\$140	\$140,000
Walkway Lighting				
Cable and Conduit	m	65	\$40	\$2,600
Lighting column (incl fittings)	Each	2	\$2,000	\$4,000
Ramp regrading				
Demolish existing ramps	Item	2	\$10,000	\$20,000
Earthworks	ltem	2	\$10,000	\$20,000
Handrail	Item	2	\$10,000	\$20,000
New Path (1.2m wide, 100mm thick reinforced)	m <sup>2</sup>	50	\$140	\$7,000
		Su	b TOTAL	\$433,900
			ncy (40%)	\$173,560
		· · · ·	ST (10%)	\$43,390
			TOTAL	\$650,850

Item	Unit	Quantity	Rate	Amount
Signalise Intersection (Antill St / Challis St)				
Detector Loops	Each	6	\$700	\$4,200
Light columns and fittings	Each	8	\$4,000	\$32,000
Pedestrian Fittings	Item	1	\$5,000	\$5,000
Cabling	Item	1	\$15,000	\$15,000
Linemarking	Item	1	\$4,000	\$4,000
Signage	Each	8	\$200	\$1,600
Adjust Intersetion Geometry	Item	1	\$45,000	\$45,000
Control Box	Each	1	\$25,000	\$25,000
Fees to ActewAGL	Item	1	\$8,500	\$8,500
Signalise Intersection (Cape St / Challis St)				
Detector Loops	Each	6	\$700	\$4,200
Light columns and fittings	Each	10	\$4,000	\$40,000
Pedestrian Fittings	Item	1	\$7,000	\$7,000
Cabling	Item	1	\$20,000	\$20,000
Linemarking	Item	1	\$4,000	\$4,000
Signage	Each	12	\$200	\$2,400
Adjust Intersetion Geometry	Item	1	\$75,000	\$75,000
Control Box	Each	1	\$25,000	\$25,000
Fees to ActewAGL	Item	1	\$8,500	\$8,500
Signalise Intersection (Challis St / Morphett St)				
Detector Loops	Each	4	\$700	\$2,800
Light columns and fittings	Each	6	\$4,000	\$24,000
Pedestrian Fittings	Item	1	\$5,000	\$24,000
Cabling	Item	1	\$15,000	\$5,000
Linemarking	Item	1	\$3,000	\$3,000
Signage	Each	6	\$200	\$1,200
Adjust Intersetion Geometry	Item	1	\$45,000	\$45,000
Control Box	Each	1	\$25,000	\$25,000
Fees to ActewAGL	Item	1	\$8,500	\$8,500
Signalise Intersection (Morphett St / Cowper St)		· ·	<b>#=</b> 00	<u> </u>
Detector Loops	Each	4	\$700	\$2,800
Light columns and fittings	Each	6	\$4,000	\$24,000
Pedestrian Fittings	Item	1	\$5,000	\$5,000
Cabling	Item	1	\$15,000	\$15,000
Linemarking	Item	1	\$4,000	\$4,000
Signage	Each	6	\$200	\$1,200
Adjust Intersetion Geometry	Item	1	\$52,500	\$52,500
Control Box	Each	1	\$25,000	\$25,000
Fees to ActewAGL	Item	1	\$8,500	\$8,500

# F.2 Long Term Cost Estimates

Signalise Intersection (Antill St / Badham St)									
Detector Loops	Each	7	\$700	\$4,900					
Light columns and fittings	Each	8	\$4,000	\$32,000					
Pedestrian Fittings	Item	1	\$5,000	\$5,000					
Cabling	Item	1	\$15,000	\$15,000					
Linemarking	Item	1	\$4,000	\$4,000					
Signage	Each	8	\$200	\$1,600					
Adjust Intersetion Geometry	Item	1	\$52,500	\$52,500					
Control Box	Each	1	\$25,000	\$25,000					
Fees to ActewAGL	Item	1	\$8,500	\$8,500					
		Su	b TOTAL	\$742,400					
Contingency (40%)									
		G	ST (10%)	\$74,240					
	TOTAL								



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